

CHOLLA POWER PLANT BOTTOM ASH MONOFILL

Periodic Run-on And Run-off Control System Plan

October 2021
AECOM Project 60664605

Prepared for:

Arizona Public Service
400 North 5th Street
Phoenix, AZ 85004

Prepared by:

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Attachment

Attachment A: AECOM, 2016, *Cholla Power Plant, Bottom Ash Monofill, Run-on and Run-off Control System Plan*, CH_RunOO_001_20161017, October 17, 2016.

1. Introduction

This periodic update to the Run-On and Run-Off Control System Plan for the Bottom Ash Monofill at Cholla Power Plant, operated by Arizona Public Service (APS), has been prepared in accordance with the requirements of Title 40 of the Code of Federal Regulations Part 257 (40 CFR 257) (“the Coal Combustion Residuals [CCR] Rule”, or “the Rule”) and the specific requirement of 40 CFR § 257.81(c)(4) that “(t)he owner or operator of the CCR unit must prepare periodic run-on and run-off control system plans required by paragraph (c)(1) of this section every five years.”

2. Methodology

The methodology used to prepare this 2021 Update and Recertification of the Run-on and Run-off Control System Plan for the Bottom Ash Monofill at the Cholla Power Plant is for the certifying Qualified Professional Engineer (QPE) to:

1. Perform a documented review of the 5 years of annual inspection reports since 2016;
2. Perform a documented review of each major component of the contributing technical information from:
 - a. AECOM, 2016. *Cholla Power Plant, Bottom Ash Monofill, Run-on and Run-off Control System Plan*, CH_RunOO_001_20161017, October 17, 2016 (hereafter referred to as the “2016 Plan” and incorporated and referenced directly as Attachment A to this document).
3. Consider and document whether the 2016 Plan and its conclusions:
 - a. Meet the current reporting requirements of the Rule;
 - b. Reflect the current condition of the structure, as known to the QPE and documented in the annual inspections;
 - c. Are compromised by any identified issues of concern; and
 - d. Are consistent with the standard of care of professionals performing similar evaluations in this region of the country; and
4. Identify any additional analyses, investigations, inspections, and/or repairs that should be completed in order to complete this 2021 Recertification.

This report documents the results of these considerations, incorporates the 2016 Plan as an Appendix, identifies any additional technical investigation or evaluations (if needed), and presents an updated certification by the QPE.

3. 2017–2021 Annual Inspection Reports

Information relevant to the current adequacy and performance of the run-on and run-off control system were reviewed. No issues were identified during the review that would affect the performance of the system and its compliance, as described in the 2016 Plan, with the requirements of 40 CFR § 257.81(c)(5).

4. 2016 Plan – Review by Section

Other than as described in the remainder of this section, the details presented in this section of the 2016 Plan adequately represent current conditions and satisfy the requirements of the Rule.

4.1 “Overview”

The details presented in this section of the 2016 Plan adequately represent current conditions and satisfy the requirements of the Rule.

4.2 “§257.81 (a)(1)(2) Run-on and Run-off Controls for CCR Landfills”

The 2016 Plan presents the details of a control system to capture and convey the 24-hour, 100-year off-site, run-on design storm event. The design storm exceeds the minimum (24-hour, 25-year) event required by §257.81 (a)(1).

The review addressed the suitability of the hydrologic basis used for the 2016 Plan. The methods used to estimate the rainfall and losses were based on the Arizona Department of Transportation *Highway Drainage Design Manual* published 1993. A newer manual was released in 2007 that is similar to the original manual. In this application, the 1993 Manual is assessed to be conservative in that it did not account for a reduction in the C-value for smaller return events (such as the 25-year).

The details presented in this section of the 2016 Plan adequately represent current conditions and satisfy the requirements of the Rule.

4.3 “§257.81 (b) Run-on and run-off controls for CCR landfills”

The 2016 Plan presents the details of a control system to capture, convey, and store the 24-hour, 25-year on-site, run-off design storm event as required by §257.81 (a)(2). As described in the 2016 Plan, there will be no discharge from the on-site retention basin.

The details presented in this section of the 2016 Plan adequately represent current conditions and satisfy the requirements of the Rule.

4.4 “§257.81 (c)(1)(2)(3)(4)(5) Run-on and run-off controls for CCR landfills”

The owner or operator continues to acknowledge and will comply with these requirements.

A certification of this Periodic Run-On and Run-Off Control Plan by a QPE is included in this document per the requirement of §257.81 (c)(5).

4.5 “§257.81 (d) Run-on and run-off controls for CCR landfills”

The owner or operator continues to acknowledge and will comply with these requirements.

5. Recommended Additional Technical Investigations or Evaluations

None identified and none recommended.

6. Conclusion

The 2016 Plan and its conclusions meet the current reporting requirements of the Rule, reflect the current condition of the structure as known to the QPE and documented in the annual inspections, are not compromised by any identified issues of concern, and are consistent with the standard of care of professionals performing similar evaluations in this region of the country.

7. Limitations

This document is for the sole use of APS on this project only and is not to be used for other projects. In the event that conclusions based upon the data presented in this document are made by others, such conclusions are the responsibility of others.

The Periodic Run-on And Run-off Control System Plan presented in this document is based on the 2016 Plan and relies and incorporates any Limitations expressed in that document.

The Certification of Professional Opinion in this report is limited to the information available to AECOM at the time this Assessment was performed in accordance with current practice and the standard of care. Standard of care is defined as the ordinary diligence exercised by fellow practitioners in this area performing the same services under similar circumstances during the same period. Professional judgments presented herein are primarily based on information from previous reports that have been assumed to be accurate, knowledge of the site, and partly on our general experience with dam safety evaluations performed on other dams.

No warranty or guarantee, either written or implied, is applicable to this work. The use of the word “certification” and/or “certify” in this document shall be interpreted and construed as a

Cholla Power Plant
Bottom Ash Monofill
Periodic Run-on And Run-off Control System Plan

Statement of Professional Opinion and is not and shall not be interpreted or construed as a guarantee, warranty, or legal opinion.

8. Certification Statement

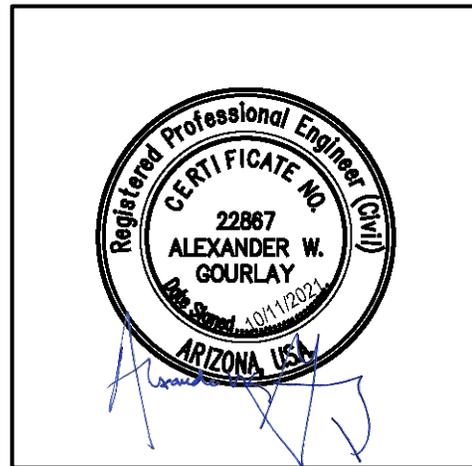
Certification Statement for:

- Certification Statement 40 CFR § 257.81(c)(5) – Periodic Run-on and Run-Off Control System Plan for an Existing CCR Landfill
- CCR Unit: Arizona Public Service; Cholla Power Plant; Bottom Ash Monofill

I, Alexander W. Gourlay, being a Registered Professional Engineer in good standing in the State of Arizona, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the information contained in this Periodic Run-On and Run-Off Control System Plan dated October 2021, including the technical content in Attachment A, meets the requirements of 40 CFR § 257.81.

Alexander W. Gourlay, P.E.
Printed Name

October 11, 2021
Date



Attachment A:

AECOM, 2016. *Cholla Power Plant, Bottom Ash Monofill, Run-on and Run-off Control System Plan*, CH_RunOO_001_20161017, October 17, 2016.

ATTACHMENT A

**AECOM, 2016. *Cholla Power Plant, Bottom Ash Monofill, Run-on and Run-off Control System Plan, CH_RunOO_001_20161017,*
October 17, 2016.**

**CHOLLA POWER PLANT
BOTTOM ASH MONOFILL
RUN-ON AND RUN-OFF CONTROL SYSTEM PLAN
CH_RunOO_001_20161017**

This *Run-on and Run-off Control System Plan* (Plan) document has been prepared specifically for the Bottom Ash Monofill (BAM) at the Cholla Power Plant in accordance with our understanding of the requirements prescribed in §257.81(3)(i) of the Federal Register, Volume 80, Number 74, dated April 17, 2015 (U. S. Government, 2015) for run-on and run-off controls associated with existing Coal Combustion Residual (CCR) landfills. Section §257.81 from the Federal Register is reproduced below for reference purposes. This document serves as the initial run-on and run-off control system plan described in §257.81(3)(i).

The BAM is an existing CCR landfill facility. The location of the BAM is illustrated on Exhibit 1. Calculations prepared previously in support of the facility operation have been referenced and reproduced herein to address the requirements listed.

§257.81 Run-on and run-off controls for CCR landfills

(a) The owner or operator of an existing or new CCR landfill or any lateral expansion of a CCR landfill must design, construct, operate, and maintain:

(1) A run-on control system to prevent flow onto the active portion of the CCR unit during the peak discharge from a 24-hour, 25-year storm; and

(2) A run-off control system from the active portion of the CCR unit to collect and control at least the water volume resulting from a 24-hour, 25-year storm.

(b) Run-off from the active portion of the CCR unit must be handled in accordance with the surface water requirements under §257.3-3.

(c) *Run-on and run-off control system plan –*

(1) *Content of the plan.* The owner or operator must prepare initial and periodic run-on and run-off control system plans for the CCR unit according to the timeframes specified in paragraphs (c)(3) and (4) of this new section. These plans must document how the run-on and run-off control systems have been designed and constructed to meet the applicable requirements of this section. Each plan must be supported by the appropriate engineering calculations. The owner or operator has completed the initial run-on and run-off control system plan when the plan has been placed in the facility's operating record as required by §257.105(g)(3).

(2) *Amendment of the plan.* The owner or operator may amend the written run-on and run-off control system plan at any time provided the revised plan is placed in the facility's operating record as required by §257.105(g)(3). The owner or operator must amend the written run-on and run-off control system plan whenever there is a change in conditions that would substantially affect the written plan in effect.

(3) *Timeframes for preparing the initial plan –*

(i) *Existing CCR landfills.* The owner or operator of the CCR unit must prepare the initial run-on and run-off control system plan no later than October 17, 2016.

(ii) *New CCR landfills and any lateral expansion of a CCR landfill.* The owner or operator must prepare the initial run-on and run-off control system plan no later than the date of initial receipt of CCR in the CCR unit.

(4) *Frequency for revising the plan.* The owner or operator of the CCR unit must prepare periodic run-on and run-off control system plans required by paragraph (c)(1) of this section every five years. The date of completing the initial plan is the basis for establishing the deadline to complete the first subsequent plan. The owner or operator may complete any required plan prior to the required deadline provided the owner or operator places the completed plan into the facility's operating record within a reasonable amount of time. In all cases, the deadline for completing a subsequent plan is based on the date of completing the previous plan. For purposes of this paragraph (c)(4), the owner or operator has completed a periodic run-on and run-off control system plan when the plan has been placed in the facility's operating record as required by §257.105(g)(3).

(5) The owner or operator must obtain a certification from a qualified professional engineer stating that the initial and periodic run-on and run-off control system plans meet the requirements of this section.

(d) The owner or operator of the CCR unit must comply with the record keeping requirements specified in §257.105(g), the notification requirements specified in §257.106(g), and the internet requirements specified in §257.107(g).

SITE INFORMATION	
Site Name / Address	Cholla Power Plant / 4801 Frontage Road, Joseph City, AZ 86032
Owner Name / Address	Arizona Public Service / 400 North 5 th Street, Phoenix, AZ 85004
CCR Unit	Bottom Ash Monofill (BAM)
OVERVIEW	
<p>The Bottom Ash Monofill (BAM) located at the Cholla Power Plant is an existing CCR landfill. Construction of the BAM began in the late 1990s. An offsite flow channel system that intercepts and conveys offsite storm water from a 98-acre contributing watershed with outfall to the south is located immediately upstream of the BAM.</p> <p>This run-on / run-off control plan describes the existing controls that preclude run-on of offsite storm flows from the landfill and the run-off of onsite storm flows from the landfill. Run-on / run-off control systems that prevent flow onto and from active CCR units from a 24-hour, 25-year storm are required. An existing diversion channel located on the perimeter of the BAM was designed and constructed to collect and convey contributing offsite flows in order to fulfill this requirement.</p>	

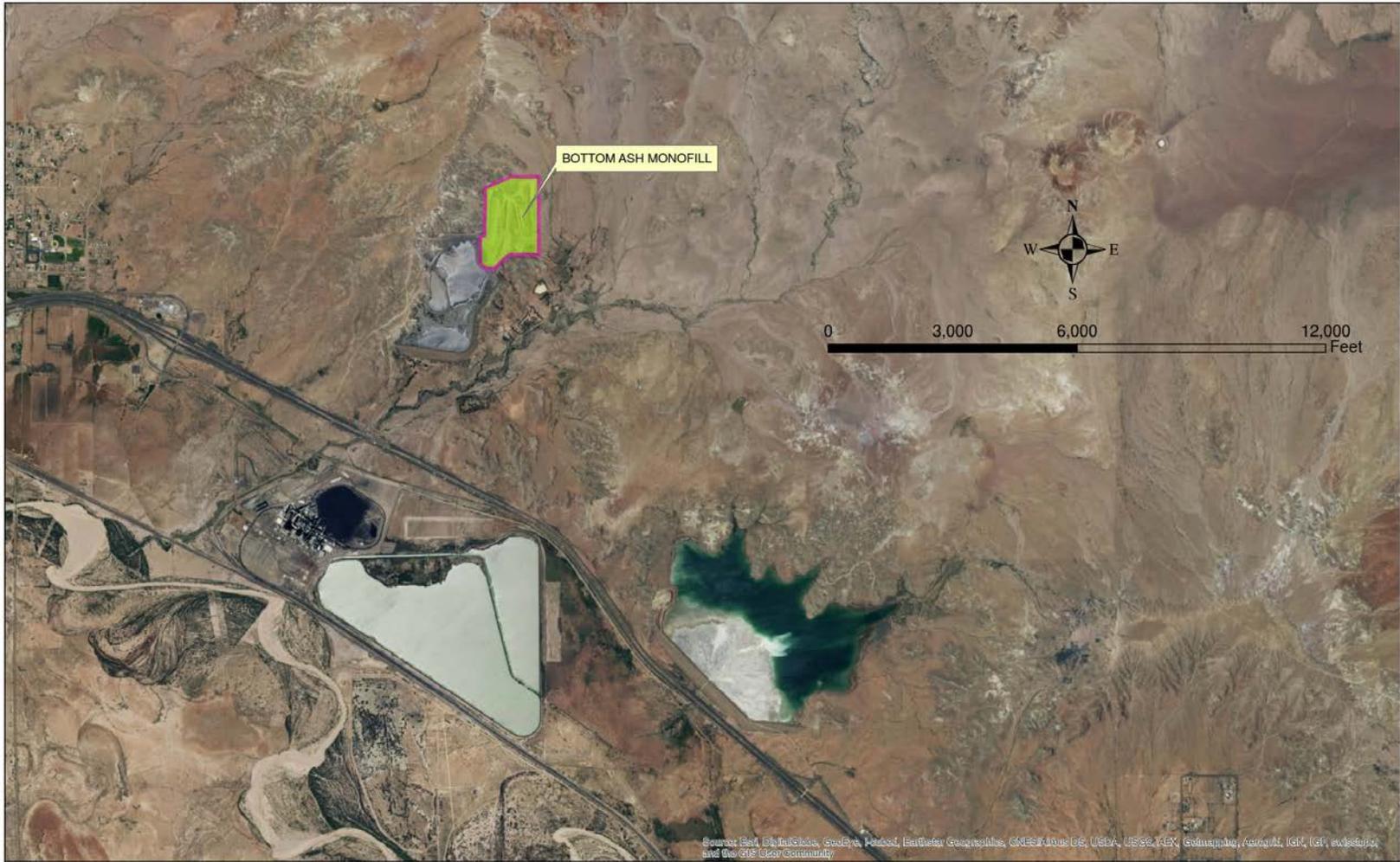


Exhibit 1 – Bottom Ash Monofill (BAM) at Cholla Power Plant Facility

§257.81 (a)(1)(2) Run-on and run-off controls for CCR landfills

(a) The owner or operator of an existing or new CCR landfill or any lateral expansion of a CCR landfill must design, construct, operate, and maintain:

(1) A run-on control system to prevent flow onto the active portion of the CCR unit during the peak discharge from a 24-hour, 25-year storm;

An offsite flow collection system is constructed on the upstream perimeter of the BAM. This system captures and conveys the 24-hour, 100-year offsite / run-on flows produced by the 98-acre offsite watershed around the project site and toward one of three historic discharge points. The historic outfall discharge points have been maintained with the use of undersized culverts placed within the offsite flow channel that allow excess flow to overtop the channel at historic discharge points as shown on the **Cholla Generating Station, Ash Monofill APP** Plan (URS 2009 A). This 100-year storm magnitude design exceeds the 24-hour, 25-year requirement shown in §257.81(a)(1). The **Cholla Generating Station, Ash Monofill APP** Plan is included in Appendix 1.

Estimates of the 24-hour, 100-year run-on peak flows captured and conveyed within the perimeter channel system are based on the **Cholla Power Plant, Ash Monofill Drainage Study, Preliminary Drainage Report** (URS 2009 B). The 24-hour, 100-year offsite peak flows were estimated with the use of the rational method of hydrology for three offsite drainage areas identified below:

- OFF1: 23.8 cfs
- OFF2: 50.7 cfs
- OFF3: 60.9 cfs

These 24-hour, 100-year run-on peak flows to the BAM Site are intercepted and conveyed in a trapezoidal channel system characterized by seven sections. The sections include a 10-foot bottom width, 2.5H: 1V side slopes, maximum channel depth of 5 feet, longitudinal slope ranging from 0.5% to 1.04%, and riprap erosion protection lining. The trapezoidal channel segment design and conveyance characteristics by Channel Section

	<p>include the following:</p> <ul style="list-style-type: none"> ▪ Peak Flows: <ul style="list-style-type: none"> ○ Channel Section 1: 25 cfs ○ Channel Section 2: 80 cfs ○ Channel Section 3: 140 cfs ○ Channel Section 4: 150 cfs ○ Channel Section 5: 80 cfs ○ Channel Section 6: 80 cfs ○ Channel Section 7: 25 cfs ▪ Longitudinal Slope: <ul style="list-style-type: none"> ○ Channel Section 1: 0.0089 ft/ft ○ Channel Section 2: 0.0104 ft/ft ○ Channel Section 3: 0.0050 ft/ft ○ Channel Section 4: 0.0100 ft/ft ○ Channel Section 5: 0.0100 ft/ft ○ Channel Section 6: 0.0100 ft/ft ○ Channel Section 7: 0.0050 ft/ft ▪ Normal Depth: <ul style="list-style-type: none"> ○ Channel Section 1: 0.73 feet ○ Channel Section 2: 1.35 feet ○ Channel Section 3: 2.22 feet ○ Channel Section 4: 1.91 feet ○ Channel Section 5: 1.36 feet ○ Channel Section 6: 1.36 feet ○ Channel Section 7: 0.86 feet <p>The normal depth calculations for the offsite flow channel system, developed as part of the <i>Cholla Power Plant, Ash Monofill Drainage Study, Preliminary Drainage Report</i> (URS 2009 B) are included in Appendix 2.</p>
<p>(a) The owner or operator of an existing or new CCR landfill or any lateral expansion of a CCR landfill must design, construct, operate, and maintain:</p> <p>(2) A run-off control system from the active portion of the CCR unit to collect and control at</p>	<p>An existing onsite storage basin located at the BAM landfill facility is designed to collect the onsite runoff volume generated by a 25-year, 24-hour storm which is estimated to be 5.2 acre-feet.</p> <p>The BAM yields a total 25-year, 24-hour storm water runoff volume of 5.23 acre-feet from the</p>

<p>least the water volume resulting from a 24-hour, 25-year storm.</p>	<p>onsite portion of the BAM based on the following parameters:</p> <ul style="list-style-type: none"> ▪ Surface Area: 47.2 acres ▪ Runoff Coefficient: 0.60 ▪ 25-year, 24-hour Precipitation Depth: 2.22 inches <p>The <i>Cholla Power Plant, Ash Monofill Drainage Study, Preliminary Drainage Report</i>, dated February 2009 indicates that the onsite storm water storage basin adjacent to the BAM provides a volume of 8.3 acre-feet at a depth of 12-feet and a surface area of 1.04 acres. This exceeds the 25-year, 24-hour storm water runoff volume of 5.23 acre-feet. The additional volume provided may accommodate sediment in addition to surface water resulting from dust control activities and compaction efforts.</p> <p>The storm water runoff and storage volume capacity calculations are included in Appendix 2.</p>
<p>§257.81 (b) Run-on and run-off controls for CCR landfills</p>	
<p>(b) Run-off from the active portion of the CCR unit must be handled in accordance with the surface water requirements under §257.3-3.</p>	<p>Onsite 25-year, 24-hour storm water runoff produced from the BAM Site is accommodated by a storm water storage basin contiguous to the BAM and does not discharge into waters of the United States.</p>
<p>§257.81 (c)(1)(2)(3)(4)(5) Run-on and run-off controls for CCR landfills</p>	
<p>(c)(1) <i>Content of the plan.</i> The owner or operator must prepare initial and periodic run-on and run-off control system plans for the CCR unit according to the timeframes specified in paragraphs (c)(3) and (4) of this section. These plans must document how the run-on and run-off control systems have been designed and constructed to meet the applicable requirements of this section. Each plan must be supported by appropriate</p>	<p>This <i>Run-on and Run-off Control System Plan</i> serves as the initial plan prescribed herein.</p>

<p>engineering calculations. The owner or operator has completed the initial run-on and run-off control system plan when the plan has been placed in the facility's operating record as required by §257.105(g)(3).</p>	
<p>(c)(2) <i>Amendment of the Plan.</i> The owner or operator may amend the written run-on and run-off control system plan at any time provided the revised plan is placed in the facility's operating record as required by §257.105(g)(3). The owner or operator must amend the written run-on and run-off control system plan whenever there is a change in conditions that would substantially affect the written plan in effect.</p>	<p>The owner or operator acknowledges and will comply with this requirement.</p>
<p>(c)(3) <i>Timeframes for preparing the initial plan –</i></p> <p>(i) <i>Existing CCR landfills.</i> The owner or operator of the CCR unit must prepare the initial run-on and run-off control system plan no later than October 17, 2016.</p> <p>(ii) <i>New CCR landfills and any lateral expansion of a CCR landfill.</i> The owner or operator must prepare the initial run-on and run-off control system plan no later than the date of initial receipt of CCR in the CCR Unit</p>	<p>The BAM is an existing CCR landfill at Cholla Power Plant. The run-on and run-off control system plan is described and included herein.</p> <p>The owner or operator acknowledges and will comply with this requirement.</p>
<p>(c)(4) <i>Frequency for revising the plan.</i> The owner or operator of the CCR unit must prepare periodic run-on and run-off control system plans required by paragraph (c)(1) of this section every five years. The date of completing the initial plan is the basis for establishing the deadline to complete the first subsequent plan. The owner or operator may complete any required plan prior to the required deadline provided the owner or operator places the completed plan into the facility's operating record within a reasonable amount of time. In all cases, the deadline for completing a subsequent</p>	<p>The owner or operator acknowledges and will comply with this requirement.</p>

<p>plan is based on the date of completing the previous plan. For purposes of this paragraph (c)(4), the owner or operator has completed a periodic run-on and run-off control system plan when the plan has been placed in the facility's operating record as required by §257.105(g)(3).</p>	
<p>(c)(5) The owner or operator must obtain a certification from a qualified professional engineer stating that the initial and periodic run-on and run-off control system plans meet the requirements of this section.</p>	<p>Certification by a professional engineer is included as an attachment to this document.</p>
<p>§257.81 (d) Run-on and run-off controls for CCR landfills</p>	
<p>(d) The owner or operator of the CCR unit must comply with the recordkeeping requirements specified in §257.105(g), the notification requirements specified in §257.106(g), and the internet requirements specified in §257.107(g).</p>	<p>The owner or operator acknowledges and will comply with this requirement.</p>

References

U. S. Government, April 17, 2015, *Federal Register, Volume 80, Number 74.*

URS Corporation, February 2009, *Cholla Generating Station, Ash Monofill APP.*

URS Corporation, February 2009, *Cholla Power Plant, Ash Monofill Drainage Study, Preliminary Drainage Report.*

Certification Statement 40 CFR § 257.81(c)(5) – Initial Run-on and Run-Off Control System Plan for an Existing CCR Landfill

CCR Unit: Arizona Public Service; Cholla Power Plant; Bottom Ash Monofill

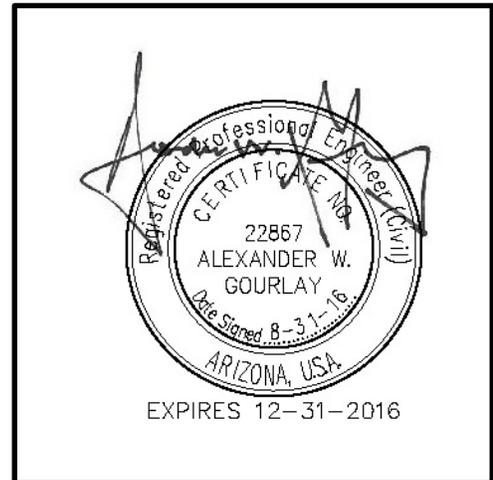
I, Alexander W. Gourlay, being a Registered Professional Engineer in good standing in the State of Arizona, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the information contained in the initial run-on and run-off control system plan dated August, 31, 2016 meets the requirements of 40 CFR § 257.81.

Alexander W. Gourlay, P.E.

Printed Name

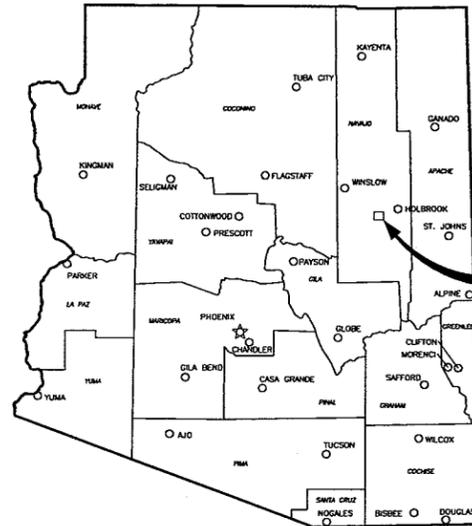
August 31, 2016

Date



APPENDIX 1 – CHOLLA GENERATING STATION, ASH MONOFILL APP PLAN

ARIZONA PUBLIC SERVICE CHOLLA GENERATING STATION CHOLLA ASH MONOFILL APP JOSEPH CITY, ARIZONA



LOCATION MAP

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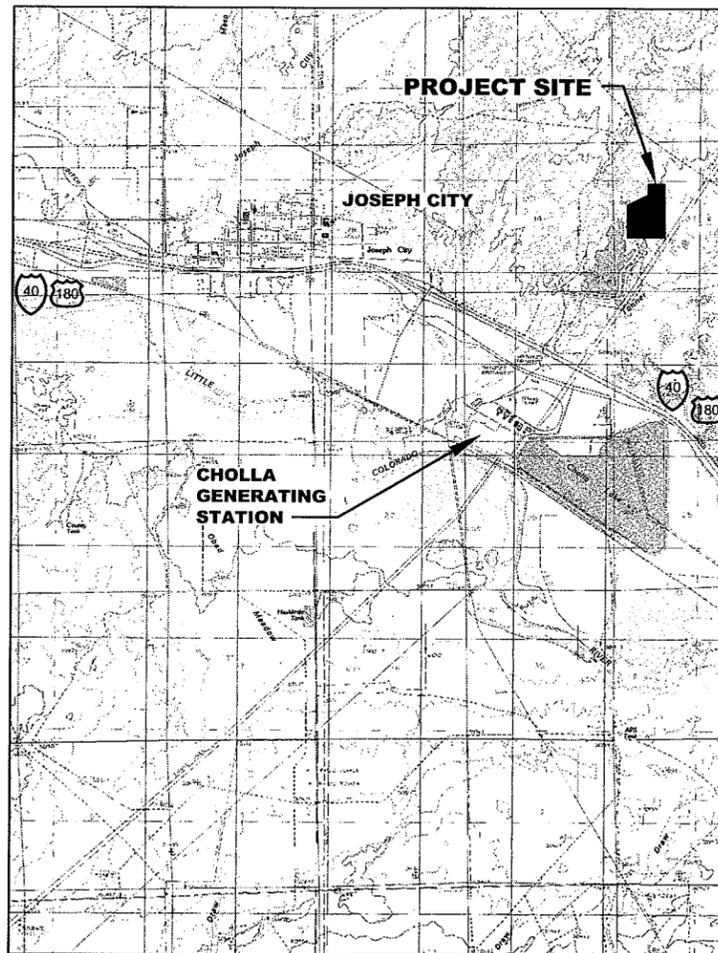


OWNER: ARIZONA PUBLIC SERVICE COMPANY
CHOLLA GENERATION STATION
JOSEPH CITY, AZ 85XXX

A.P.S. PROJECT MANAGER:
PROJECT MANAGER PERSON
(602) XXX-XXXX
(602) XXX-XXXX FAX

ENGINEER: URS CORPORATION
7720 N. 16TH STREET, SUITE 100
PHOENIX, AZ 85020

ENGINEER OF RECORD:
TODD RINGSMUTH, P.E.
(602) 371-1100
(602) 371-1615 FAX



VICINITY MAP

0 1500 3000 6000 9000
SCALE IN FEET



DRAWING INDEX

DRAWING NO.	REV.	DRAWING TITLE
5548-CHOLLA-ASH-XX01	A	COVER SHEET
5548-CHOLLA-ASH-XX02	A	GENERAL SITE PLAN
5548-CHOLLA-ASH-XX03	A	SECTION AND DETAILS
5548-CHOLLA-ASH-XX04	A	PLAN AND PROFILES
5548-CHOLLA-ASH-XX05	A	PLAN AND PROFILES
5548-CHOLLA-ASH-XX06	A	SECTION AND DETAILS

PRELIMINARY
NOT FOR
CONSTRUCTION

URS
7720 N. 16th Street Suite 100
Phoenix, Arizona 85020
(602) 371-1100

A	02-09	APP SUBMITTAL	RBZ	KLP	CDW	TER						
NO.	DATE	REVISION	DWN	CHD	EXD	RWVD	APVD	W.A.				

CHOLLA GENERATING STATION
ASH MONOFILL APP

COVER SHEET

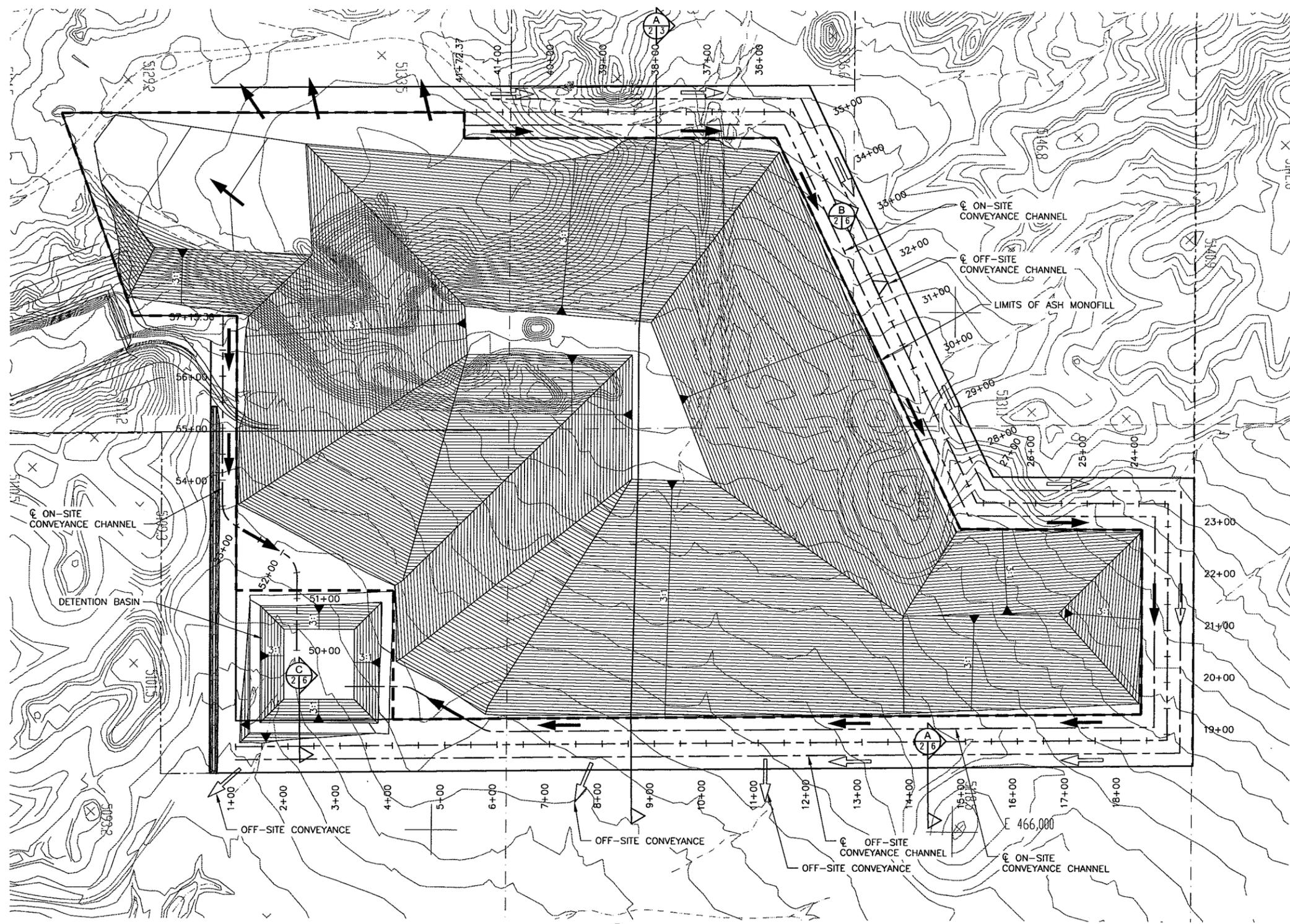


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EXD	CDW	UNIT	DISC	TYPE	SYS	NUMBER	SHEET				
RWVD	TER	XX	X	XX	XXX	XXXXXX	XX01				

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A	02-09	APP SUBMITTAL	RBZ	KLP	CDW	TER
NO.	DATE	REVISION	DWN	CHD	EXD	RVND

CHOLLA GENERATING STATION
 ASH MONOFILL APP
 GENERAL SITE PLAN



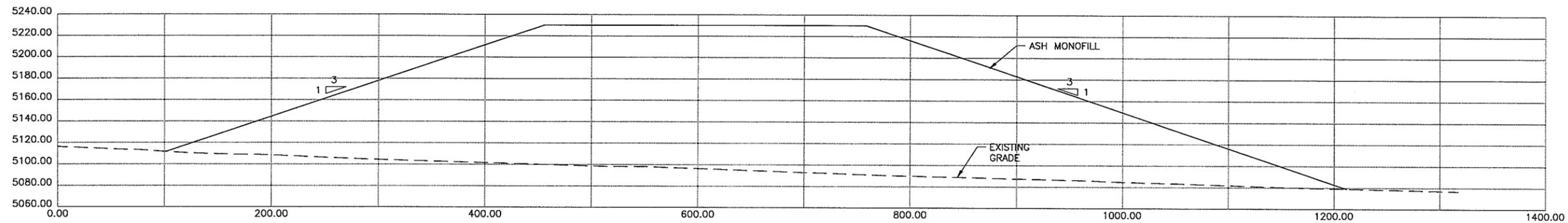
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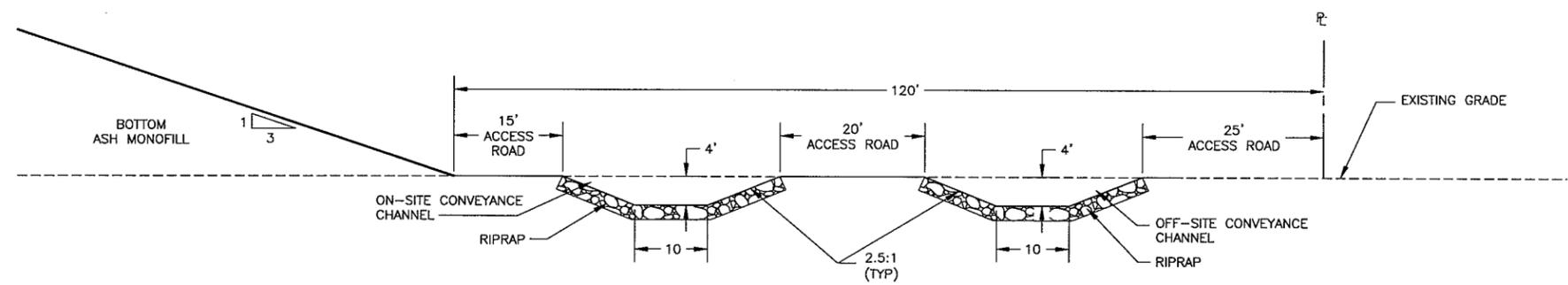
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 Blue State Center
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SECTION A
SCALE IN FEET



SECTION B
SCALE IN FEET

URS
7720 N. 16th Street Suite 100
Phoenix, Arizona 85020
(602) 371-1100

PRELIMINARY
NOT FOR
CONSTRUCTION

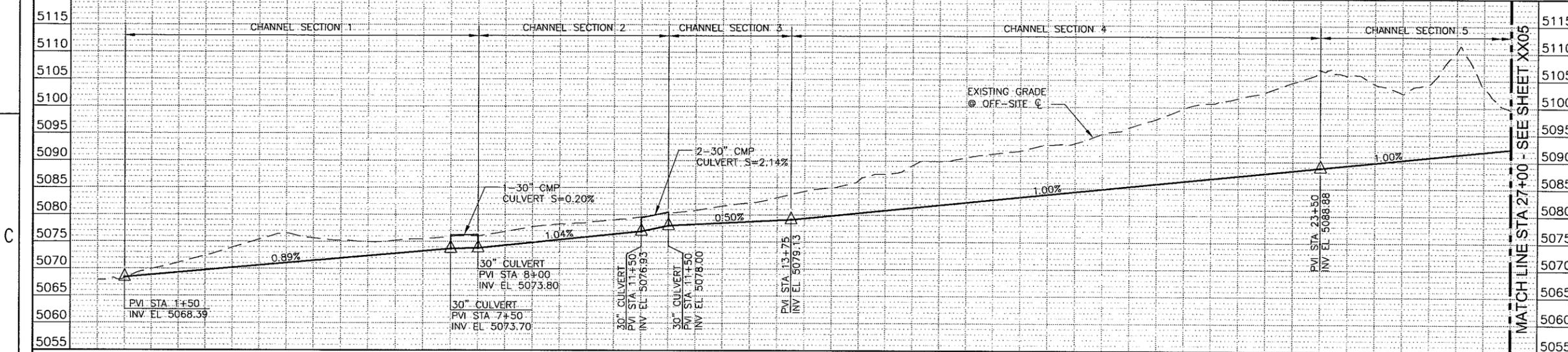
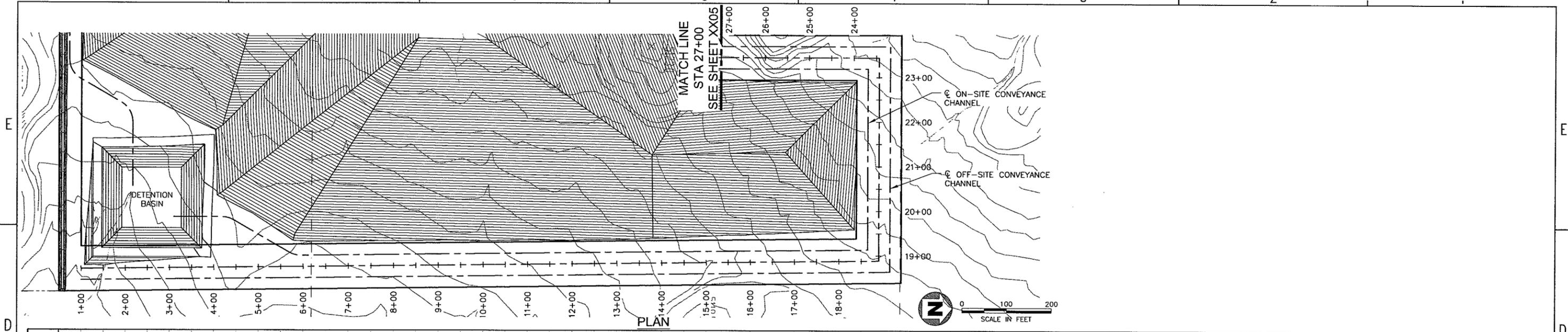
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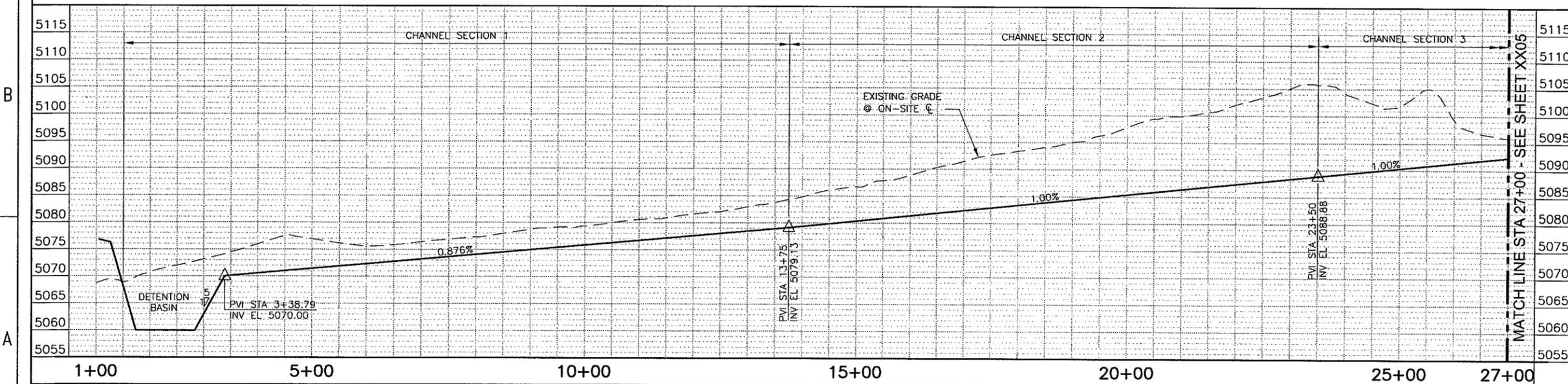
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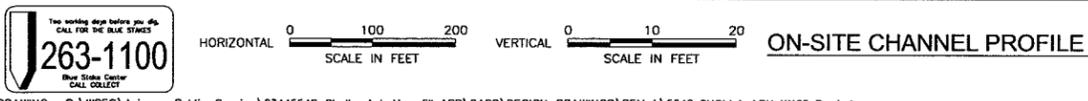
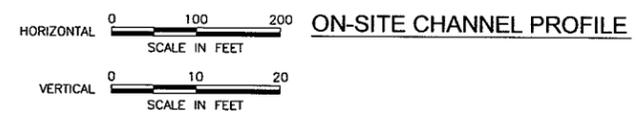
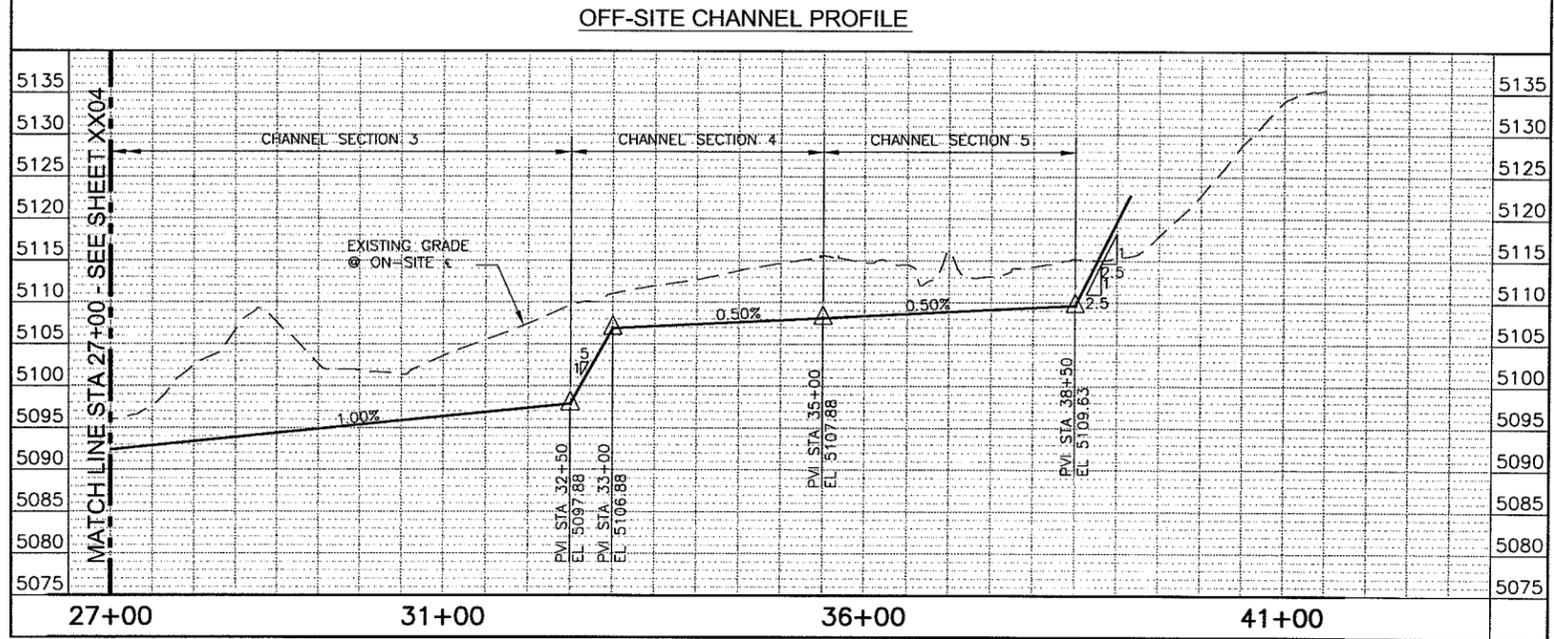
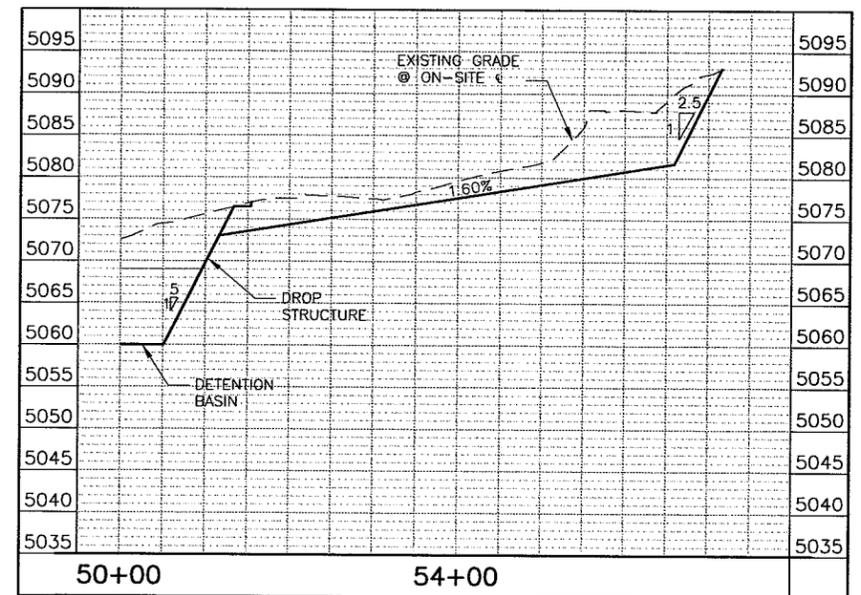
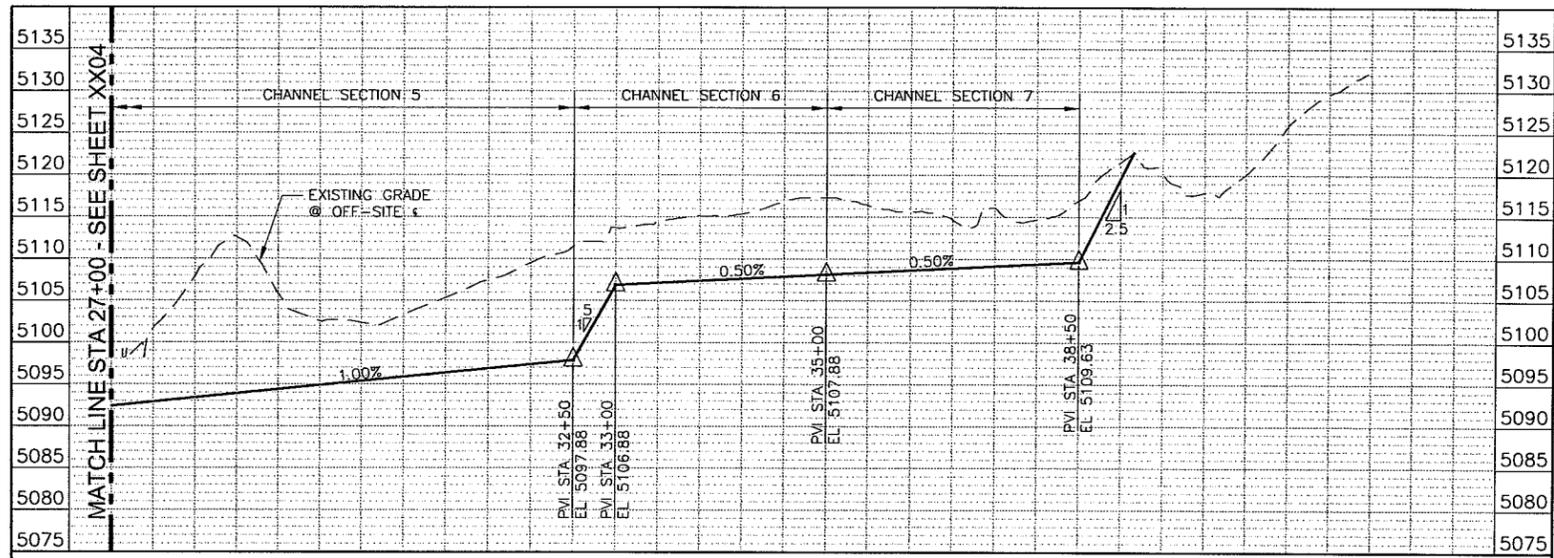
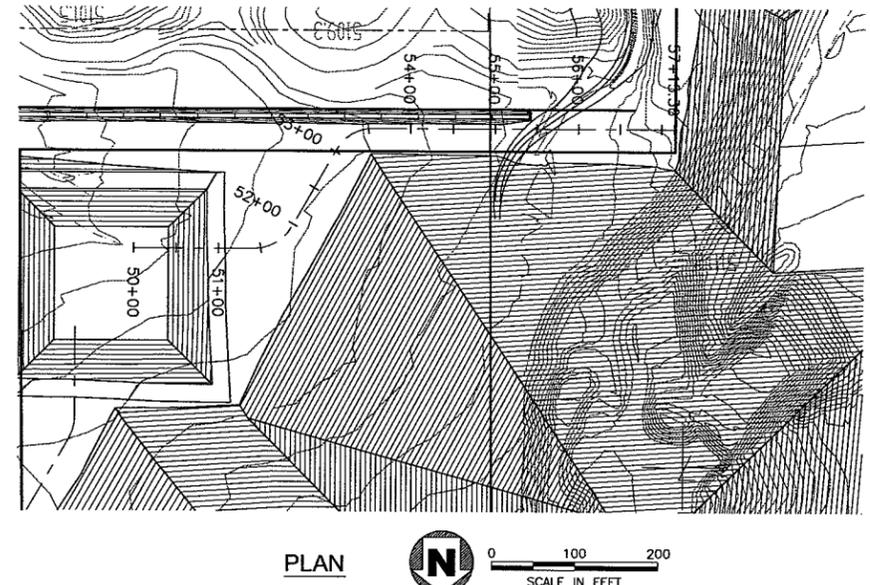
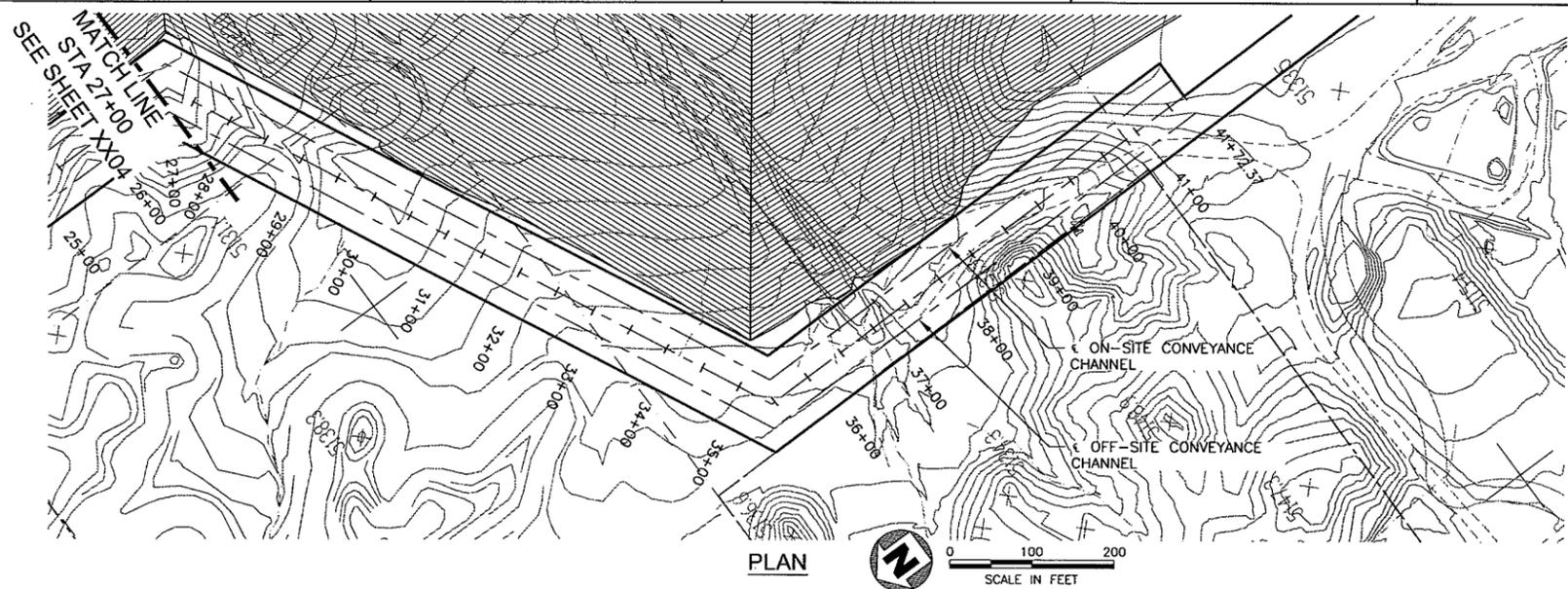
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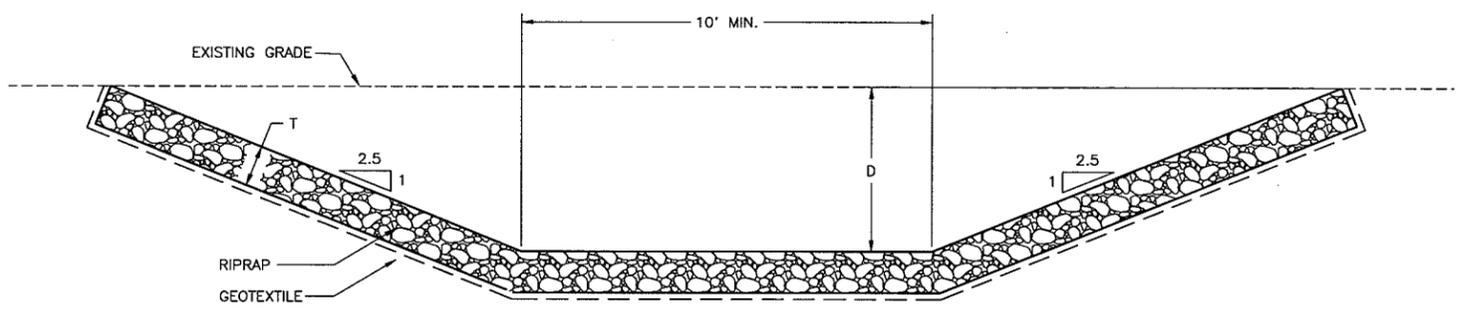
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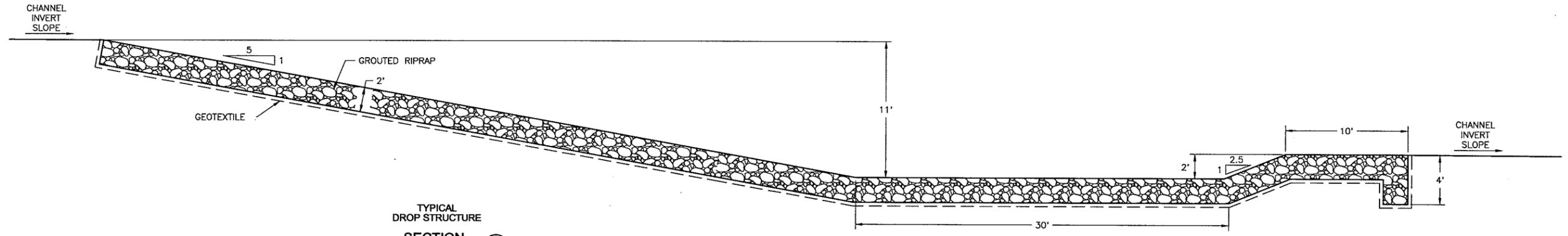
ON-SITE CHANNEL TABLE						
CHANNEL SECTION	STA	STA	RIPRAP D ₅₀ (IN)	T (FT)	CHANNEL DEPTH	CHANNEL SLOPE
1	1+50	13+75	4	1	3	0.876%
2	13+75	23+50	4	1	4	1.00%
3	23+50	32+50	4	1	4	1.00%
4	33+00	35+00	4	1	4	0.50%
5	35+00	38+50	4	1	4	0.50%
1	50+00	56+58.13	4	1	4	1.60%

OFF-SITE CHANNEL TABLE						
CHANNEL SECTION	STA	STA	RIPRAP D ₅₀ (IN)	T (FT)	CHANNEL DEPTH	CHANNEL SLOPE
1	1+50	7+50	4	1	3	0.885%
2	8+00	11+00	4	1	4	1.04%
3	11+50	13+75	4	1	4	0.50%
4	13+75	23+50	4/6	1	4	1.00%
5	23+50	32+50	4	1	4	1.00%
6	33+00	35+00	4	1	4	0.5%
7	35+00	38+50	4	1	3	0.5%

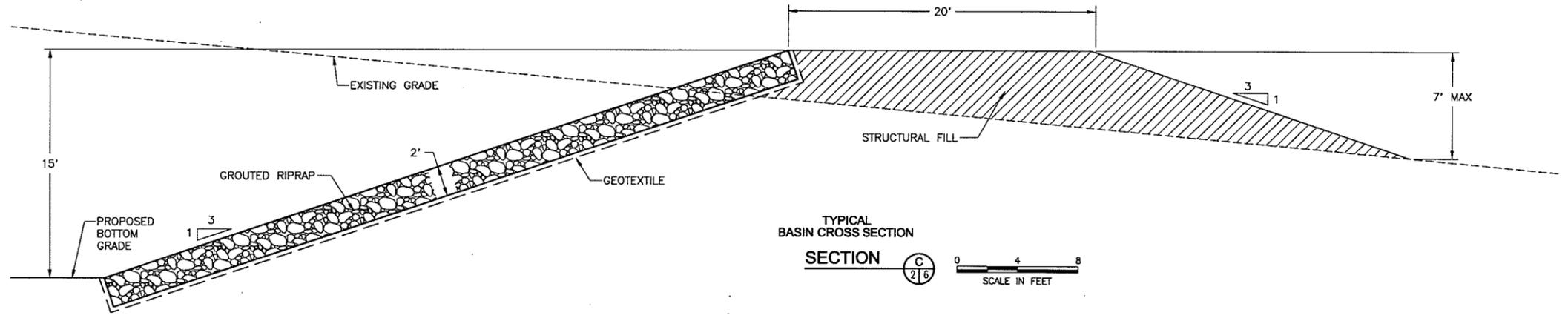
NOTE:
1. DROP STRUCTURE D₅₀=1FT, THICKNESS IS 2xD₅₀=2FT AND WILL BE GROUTED.



TYPICAL CHANNEL SECTION (A) 2/6
SCALE IN FEET



TYPICAL DROP STRUCTURE SECTION (B) 2/6
SCALE IN FEET



TYPICAL BASIN CROSS SECTION (C) 2/6
SCALE IN FEET

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**APPENDIX 2 - CHOLLA POWER PLANT, ASH MONOFILL DRAINAGE STUDY, PRELIMINARY DRAINAGE
REPORT**

**CHOLLA POWER PLANT
ASH MONOFILL DRAINAGE STUDY
PRELIMINARY DRAINAGE REPORT**

**Prepared for
NAVAJO COUNTY**

**URS Job No. 23445548
February 2009**

Cholla Power Plant - Ash Monofill

Drainage Study

Preliminary Drainage Report

Prepared for:

Navajo County

This report is based on data, site conditions and other information that are generally applicable as of 2009, and the conclusions and recommendations herein are therefore applicable only to that period.

This report is preliminary and is not to be used as the sole basis for final design or for construction or as a basis for major capital decisions. Further analysis of the study area should be performed prior to any designs or decisions.

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1.0 INTRODUCTION

This drainage report presents the hydrologic and hydraulic analysis completed for the proposed expansion to the Cholla Generating Station Ash Monofill for the Arizona Public Service (APS). This report will be included as a portion of the revised Aquifer Protection Permit that will be submitted to the Arizona Department of Environmental Quality (ADEQ). The purpose of this analysis was to calculate the pre-development and post-development flows to ensure the proposed design would not increase historic flow amounts and discharge points.

1.1 SITE LOCATION AND DESCRIPTION

The Ash Monofill area is in eastern Arizona located on the Arizona Public Service (APS) Cholla Generating Station property, approximately 2.5 miles east of the town of Joseph City, Arizona, as shown in Figure 1. The site covers approximately 50 acres and is north of Interstate 40. The study area encompasses all of the off-site drainage areas for the proposed Ash Monofill, which covers approximately 100 acres.

2.0 FEDERAL EMERGENCY MANAGEMENT AGENCY FLOODPLAIN CLASSIFICATION

The project site is located in the flood hazard area represented by Zone X of the National Flood Insurance Program, based on Flood Insurance Rate Map Number 04017C3308E and 04017C3310E, revised September 26, 2008 (see Figure 3). The Zone X classification indicates the following:

Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1-foot with drainage area less than one square mile.

3.0 HYDROLOGY

3.1 METHODOLOGY

The hydrologic analysis for the Ash Monofill site was computed using the Rational Method based on the *Arizona Department of Transportation Highway Drainage Design Manual, Hydrology (ADOT Drainage Manual)* (ADOT 1993). This methodology can be used to estimate peak discharges and runoff volumes for small, uniform drainage areas that are less than 160 acres

in size. The largest existing or developed drainage basin for this proposed expansion project is approximately 60 acres, which is much less than the 160 acre maximum threshold. The peak discharges and runoff volumes were used to design the offsite and onsite collection system. The topographic and soil data information were collected to assist in the analysis.

3.2 TOPOGRAPHY

Topography was provided by APS at 2-foot contour intervals and by United States Geological Survey's topographic maps at 5-foot contour intervals. All topographic data was generated in North American Vertical Datum (NAVD) 1988 for the vertical dimension and in North American Datum 1983, Arizona State Plane East, for the horizontal. The combined survey data were used as guidance for delineating drainage basins (see Figure 4).

3.3 SOILS

Soil data was obtained from the Natural Resources Conservation Service (NRCS) online soil survey site (NRCS 2008). There are two existing soil types in the vicinity of the proposed development. The first soil is Gypsiorthids-Torriothents, 5 to 60 percent slopes and the second soil is Brunswick Sandy Clay Loam, 1 to 5 percent slopes. The two soils belong to Hydrologic Soil Group B and this information was used to determine coefficient values for the Rational Method calculations. This information has been included in Appendix B.

3.4 HYDROLOGIC MODELING PARAMETERS

The *ADOT Drainage Manual* was used to determine the hydrologic parameters to be used in the Rational Method. A detailed discussion of the hydrologic parameter calculations is provided in the following sections.

3.4.1 Rainfall

The 100-year, 24-hour storm event was used as the design storm for the offsite collection system and the 25-year, 24-hour storm was used for the onsite collection system and retention basin. These are the design storms specified for use by ADEQ. The rainfall depths were obtained from the National Oceanic and Atmospheric Administration (NOAA) Atlas 14 (NOAA 2008). This information was used to create a site-specific intensity-duration-frequency (I-D-F) curve that was used in the Rational Method. The 100-year, 24-hour rainfall amount used in the analysis is 2.77 inches and the 25-year, 24-hour rainfall is 2.22 inches. The rainfall information and I-D-F calculation is included in Appendix A.

3.4.2 Drainage Areas

The existing drainage area basin boundaries for the study area are shown in Figure 4. The watershed delineations were performed using the 2-foot and 5-foot contours. The existing information was used to determine the pre-development discharges and historic outfall locations. The design of the proposed expansion will maintain the historic outfall locations and will ensure the post-development flows are below the pre-development flows.

The post-development drainage areas were delineated based on the proposed improvements within the APS property. The post-development drainage basins are shown on Figure 5. The onsite basins will be conveyed by channels to a retention basin and the offsite basins will be routed around the property to their historic outfall locations.

3.4.3 Rational Coefficient

The rational coefficients were selected based on the land use and vegetation cover using Figure 2-5 from the *ADOT Drainage Manual* (ADOT 1993). The study area was considered to be Upland Rangeland with a vegetation cover of approximately 10 percent. The soil in the vicinity of the project site belongs to the Hydrologic Soil Type B. Based on this information, the study area was considered to have a rational coefficient of 0.30 for existing conditions. The soil that will be used for the landfill cover will be a clay soil with little or no vegetation. This soil type was selected to minimize rainfall infiltration into the landfill and will have a rational coefficient of 0.60. This information has been included in Appendix B.

3.4.4 Time of Concentration

The time of concentrations for each of the sub-basins was calculated based on Equation 2-2, below, and following parameters from the Rational Method in the *ADOT Drainage Manual* (ADOT 1993):

$$T_c = 11.4L^{0.5} K_b^{0.52} S^{-0.31} i^{-0.38}$$

Where T_c = time of concentration in hours

L = length of the longest flow path in feet per mile

K_b = watershed resistance coefficient

S = slope of the longest flow path in feet per mile

i = average rainfall intensity, in inches per hour, for a duration of rainfall equal to T_c , unless T_c is less than 10 minutes, in which case (i) is for a 10-minute duration

A summary of the time of concentration is provided in Table 1 below and in Appendix B.

Table 1
Time of Concentration Summary¹

Sub-Basin	L (miles)	Area (ac)	H (feet) (change in elevation along L)	S (feet/mile)	i (inches/ hour)	T _c (min)
OFF-1 (Pre-Development)	0.56	39.6	129.0	229.9	4.60	16
OFF-2 (Pre-Development)	0.63	49.3	135.0	215.4	4.30	18
OFF-3 (Pre-Development)	0.85	59.7	166.0	194.9	3.80	22
OFF-1 (Post-Development)	0.26	14.1	71.0	276.3	5.73	10
OFF-2 (Post-Development)	0.39	34.4	92.0	234.0	5.00	13
OFF-3 (Post-Development)	0.66	49.2	126.0	189.8	4.20	19
ON-4	0.16	8.9	144.0	928.6	4.48	10
ON-5	0.12	13.7	127.0	1065.4	4.48	10
ON-6	0.16	19.9	153.0	967.5	4.48	10
ON-7	0.09	4.7	112.0	1246.0	4.48	10

SOURCE: Arizona Department of Transportation 1993

NOTE: ¹ Based on the *Arizona Department of Transportation Highway Drainage Design Manual, Hydrology*, Equation 2-2, Time of Concentration Estimation

3.5 HYDROLOGIC MODELING RESULTS

The Rational Method model provides the 100-year, 24-hour peak discharges for the offsite basins and the 25-year, 24-hour peak discharges for the onsite drainage basin. The hydrology calculations and Rational Method model are provided in Appendix B, along with the total volume of runoff calculated for each on-site drainage basin. The peak discharge summary is provided Table 2 and in Appendix B. This table confirms that the existing peak flow amounts for basin OFF-1, OFF-2, and OFF-3 will be decreased in the developed condition. The on-site runoff volume is shown in Table 3.

Table 2
Peak Discharge Summary Table

Sub-Basin	Rainfall	Area (acre)	Peak Discharge (cubic feet per second)
OFF-1 (Pre-Development)	100-yr, 24-hour	39.6	54
OFF-2 (Pre-Development)	100-yr, 24-hour	49.3	63
OFF-3 (Pre-Development)	100-yr, 24-hour	59.7	67
OFF-1 (Post-Development)	100-yr, 24-hour	14.1	24
OFF-2 (Post-Development)	100-yr, 24-hour	34.4	51
OFF-3 (Post-Development)	100-yr, 24-hour	49.2	61
ON-4	25-year, 24-hour	8.9	24
ON-5	25-year, 24-hour	13.7	37
ON-6	25-year, 24-hour	19.9	54
ON-7	25-year, 24-hour	4.7	13

Table 3
Volume Summary Table

Sub-Basin	P (inches)	Area (acres)	C	Volume (acre-feet)
ON-4	2.22	8.9	0.60	0.99
ON-5	2.22	13.7	0.60	1.52
ON-6	2.22	19.9	0.60	2.21
ON-7	2.22	4.7	0.60	0.52
Total Volume:				5.23

NOTES: P = 25-year, 24-hour rainfall event; C = rational coefficient

4.0 DRAINAGE DESIGN

4.1 OFFSITE COLLECTION SYSTEM

The offsite collection system was designed to capture the offsite flows and route them around the proposed improvements to their historic outlet points. There are three historic outfall points for the existing drainage basins, which are displayed on Figure 4. The offsite collection system consists of rip-rap lined channels, culverts, weirs, and a drop structure.

4.1.1 Offsite Perimeter Channel

The offsite perimeter channel is located within the APS property adjacent to the property line. It has a 10-foot bottom width with 2 ½ to 1 side slopes (H:V). URS field investigation of the proposed site revealed evidence of scour and sediment deposits in existing collection channels due to the nature of the contributing soils. The size and depth of the proposed perimeter channel is small enough that rip-rap can be used for the entire channel cross sectional area. The sizing calculations for the rip-rap channel are located in Appendix C.

The perimeter channel consists of seven sections based on the offsite inflow and historic outflow locations. The normal depth calculations for the channel sections are located in Appendix C. A drop structure along the channel alignment is necessary because the channel was designed to minimize the amount of excavation. The elevation drops nine feet in this structure over a length of fifty feet. The calculation for this drop structure is included in Appendix C.

4.1.2 Historic Outflow Points

Offsite flows in the channels will outlet at the historic outflow locations and the peak flow at these locations will be maintained. Due to the topography of the area, the proposed site layout, and the location of the historic outfalls, culverts were placed in the channel. These culverts reduce the flow in the downstream channel section by allowing excess flows to overtop the channel on the east side maintaining the historic outfall location. A lateral weir is located on the east bank of the channel which allows flow to overtop the channel section. The design of the culverts and weir is located in Appendix C.

4.2 ONSITE COLLECTION SYSTEM

The onsite collection system was designed based on the ultimate developed condition. This design parameter was selected because the peak flows and volumes would be maximized, resulting in a conservative design that eliminates the necessity of an interim design and

construction. The onsite collection system consists of channels, drop structures, and a storage basin. There is a portion of the project site within the proposed improvements that will not drain into the onsite collection system, but will be collected by an existing pond located south of this project.

4.2.1 Onsite Channels

The layout of the onsite channel system parallels the offsite collection system and is shown on Figure 5. The offsite and onsite channels were aligned based on the offset from landfill as well as the property line, which is shown on Figure 6. The onsite channel system will collect and convey the runoff from the 25-year, 24-hour storm to the storage basin located in the southeast corner of the project site. The channel alignments near the onsite storage basin were modified so that they would not flow into the basin at the corners. The channels will empty into the storage basin thru drop structures and a stilling basin. The stilling basin will protect the storage basin floor by dissipating the flow energy from the drop structure. The hydraulic design calculations are included in Appendix C.

4.2.2 Onsite Storage Basin

The onsite storage basin was designed to collect the runoff volume generated by a 25-year, 24-hour storm. That volume is shown in Table 3 as 5.2 acre-feet. The basin depth was increased by 2-feet to account for sediment and the basin will have 2-feet of freeboard. The overall proposed depth of the basin is 12-feet with 3 to 1 side slopes (H:V). An overflow spillway is located on the east side of the basin that will allow the basin to overtop during storms greater than the 25-year, 24-hour storm. The actual storage volume provided by the basin is 8.2 acre-feet. That calculation is provided in Appendix C. Any runoff volume collected in this storage basin will be pumped within 36-hours to the existing pond located south of the project site.

4.3 CONCLUSIONS

The offsite drainage system was design to collect and convey the 100-year, 24-hour peak flow around the project site to the historic outlet points. The post-development peak flow at these discharge points is less than the pre-development flows. The onsite collection system was designed to collect and convey the 25-year, 24-hour peak flow to a storage basin that has the volume capacity for that design storm. The drainage design drawings for the proposed improvements are included in this report.

5.0 REFERENCES

Arizona Department of Transportation (ADOT). 1993. *Highway Drainage Design Manual, Hydrology*. Revision 8-11-94. Report FHWA-AZ93-281. Prepared for the Arizona Department of Transportation by NBS/Lowry Engineers & Planners, Phoenix, Arizona, and George V. Sabol Consulting Engineers, Brighton, Colorado.

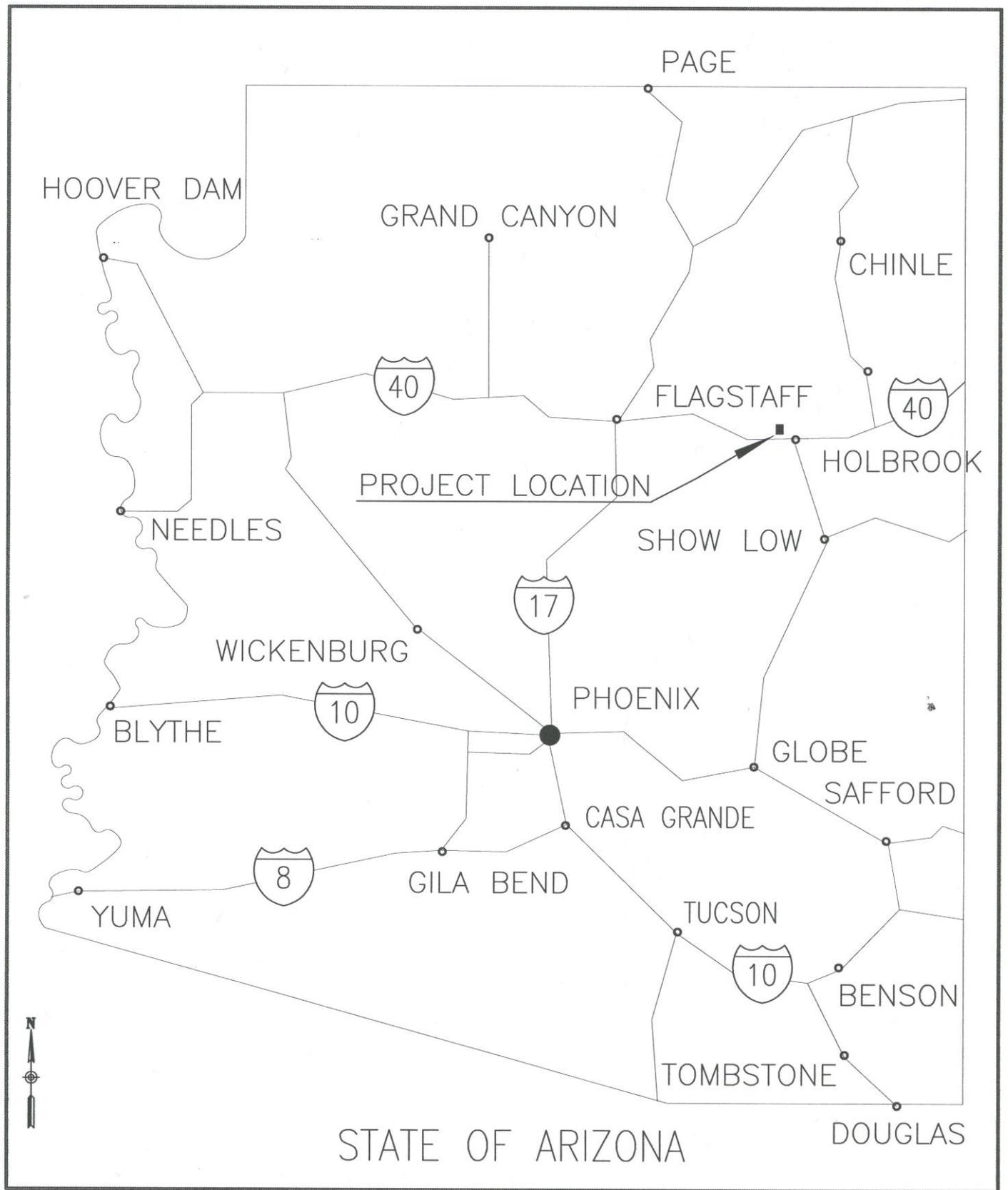
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http://www.azdot.gov/Highways/Roadway_Engineering/Drainage_Design/PDF/ADOTHighwayDrainageDesignManual_Hydrology.pdf.

National Oceanic and Atmospheric Administration (NOAA). 2008. NOAA's National Weather Service, Hydrometeorological Design Studies Center, Precipitation Frequency Data Service. NOAA Atlas 14, Arizona. Available at <http://hdsc.nws.noaa.gov/hdsc/pfds> (accessed January 2009).

United States Department of Agriculture, National Resources Conservation Service (NRCS) 2008. Web Soil Survey. Available at <http://websoilsurvey.nrcs.usda.gov/app/> (accessed February 4, 2009).

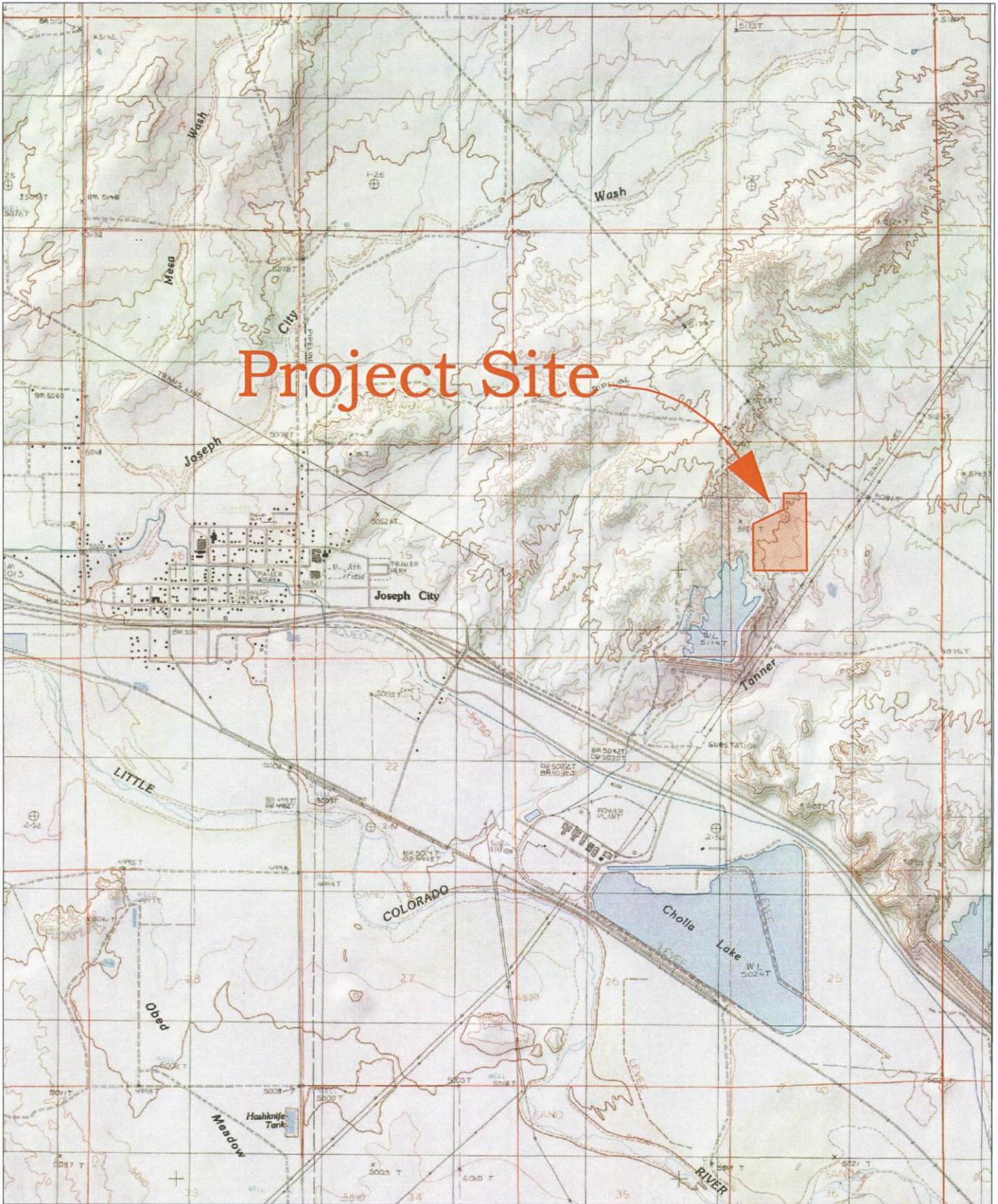
FIGURES



Site Location Map



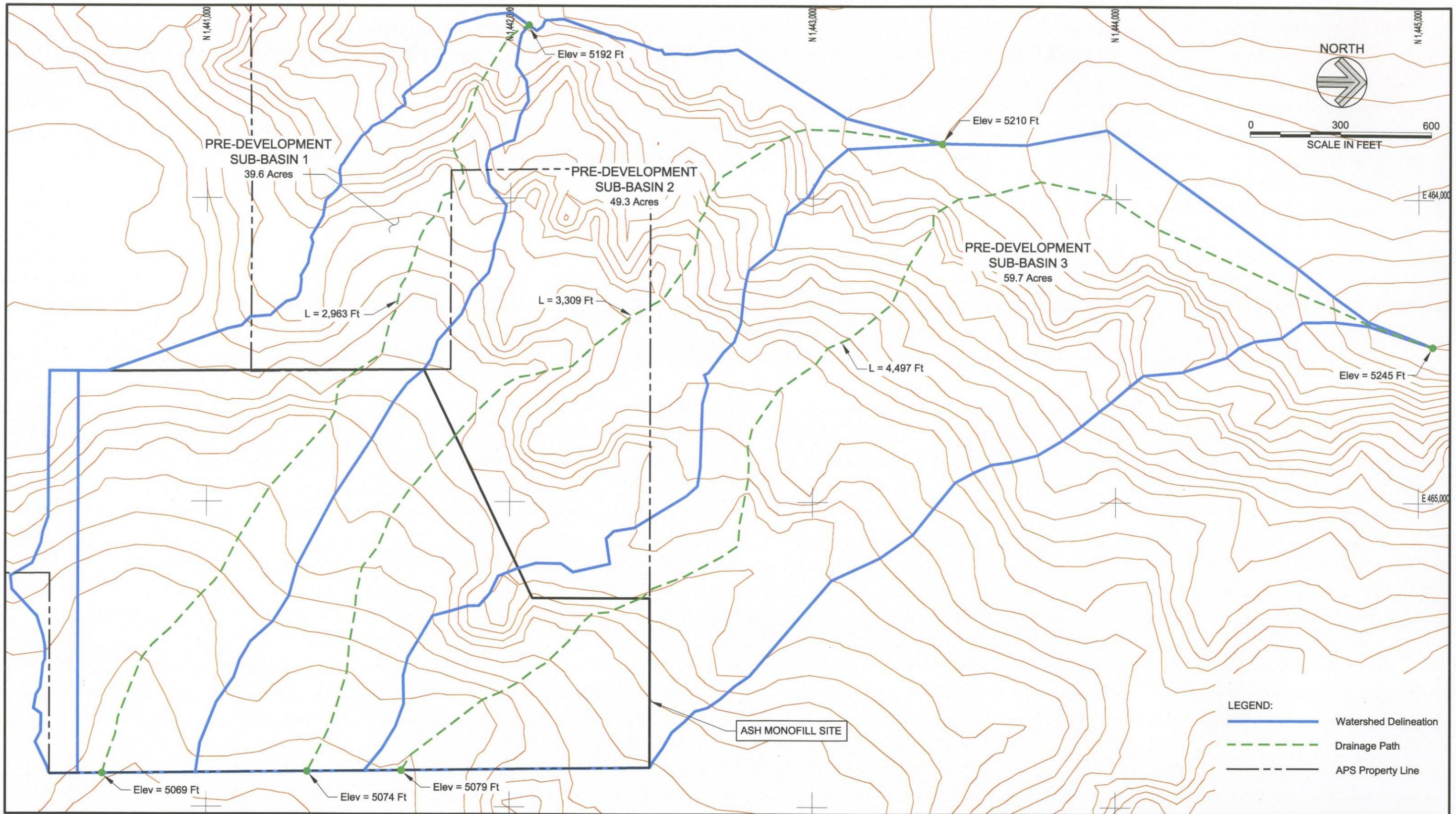
Figure 1



Site Location Map

Ash Monofill Drainage Study
Navajo County



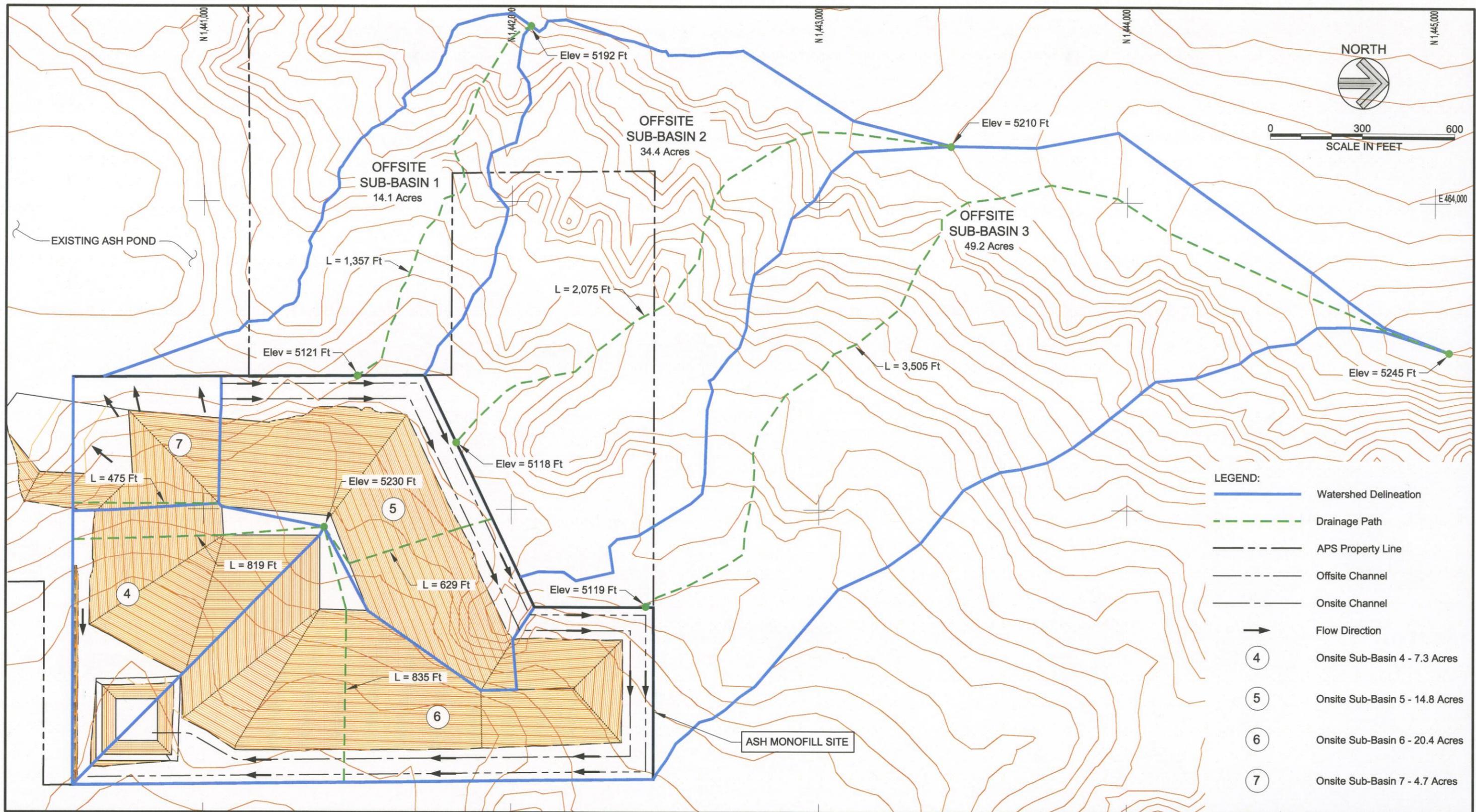


Source: Topo provided by USGS 5-Ft Topographic Map

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PRE-DEVELOPMENT DELINEATION

Figure 4



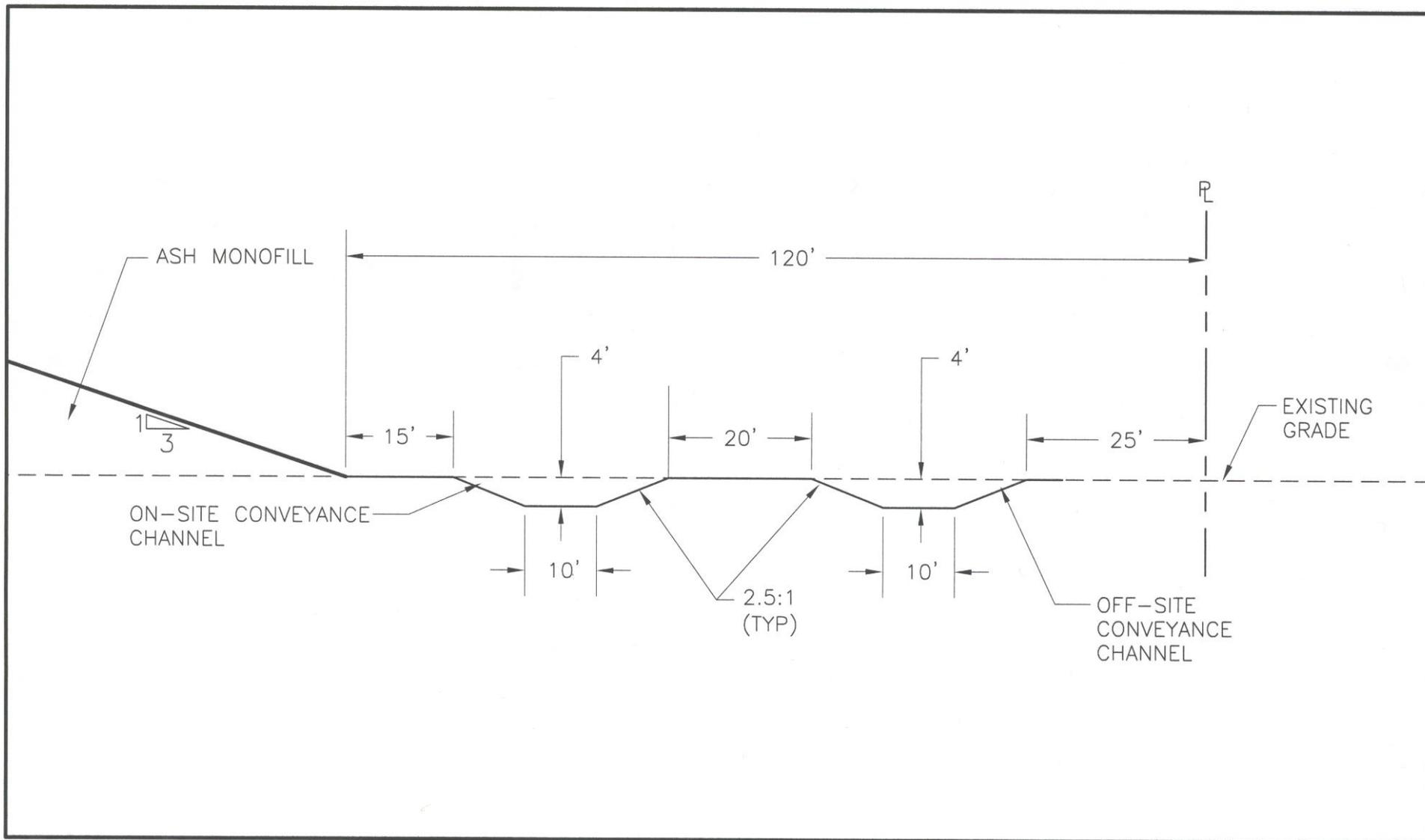


Source: Topo provided by USGS 5-Ft Topographic Map
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 POST-DEVELOPMENT DELINEATION



Figure 5

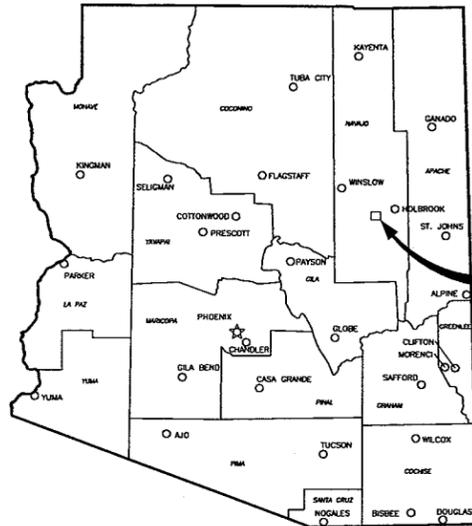


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 CHOLLA ASH MONOFILL
 OFFSITE AND ONSITE PERIMETER CHANNELS

DRAINAGE DRAWINGS

ARIZONA PUBLIC SERVICE CHOLLA GENERATING STATION CHOLLA ASH MONOFILL APP

JOSEPH CITY, ARIZONA



LOCATION MAP

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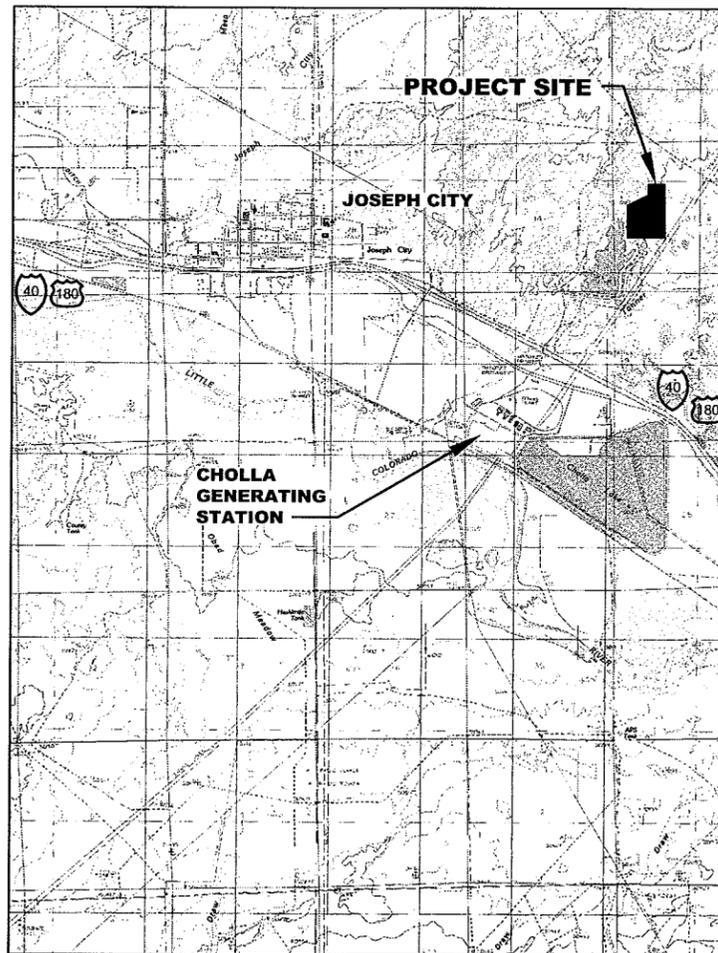


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VICINITY MAP

0 1500 3000 6000 9000
SCALE IN FEET



DRAWING INDEX

DRAWING NO.	REV.	DRAWING TITLE
5548-CHOLLA-ASH-XX01	A	COVER SHEET
5548-CHOLLA-ASH-XX02	A	GENERAL SITE PLAN
5548-CHOLLA-ASH-XX03	A	SECTION AND DETAILS
5548-CHOLLA-ASH-XX04	A	PLAN AND PROFILES
5548-CHOLLA-ASH-XX05	A	PLAN AND PROFILES
5548-CHOLLA-ASH-XX06	A	SECTION AND DETAILS

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NO.	DATE	REVISION	DWN	CHD	EXD	RWVD	APVD	W.A.
A	02-09	APP SUBMITTAL	RBZ	KLP	CDW	TER		

CHOLLA GENERATING STATION
ASH MONOFILL APP

COVER SHEET

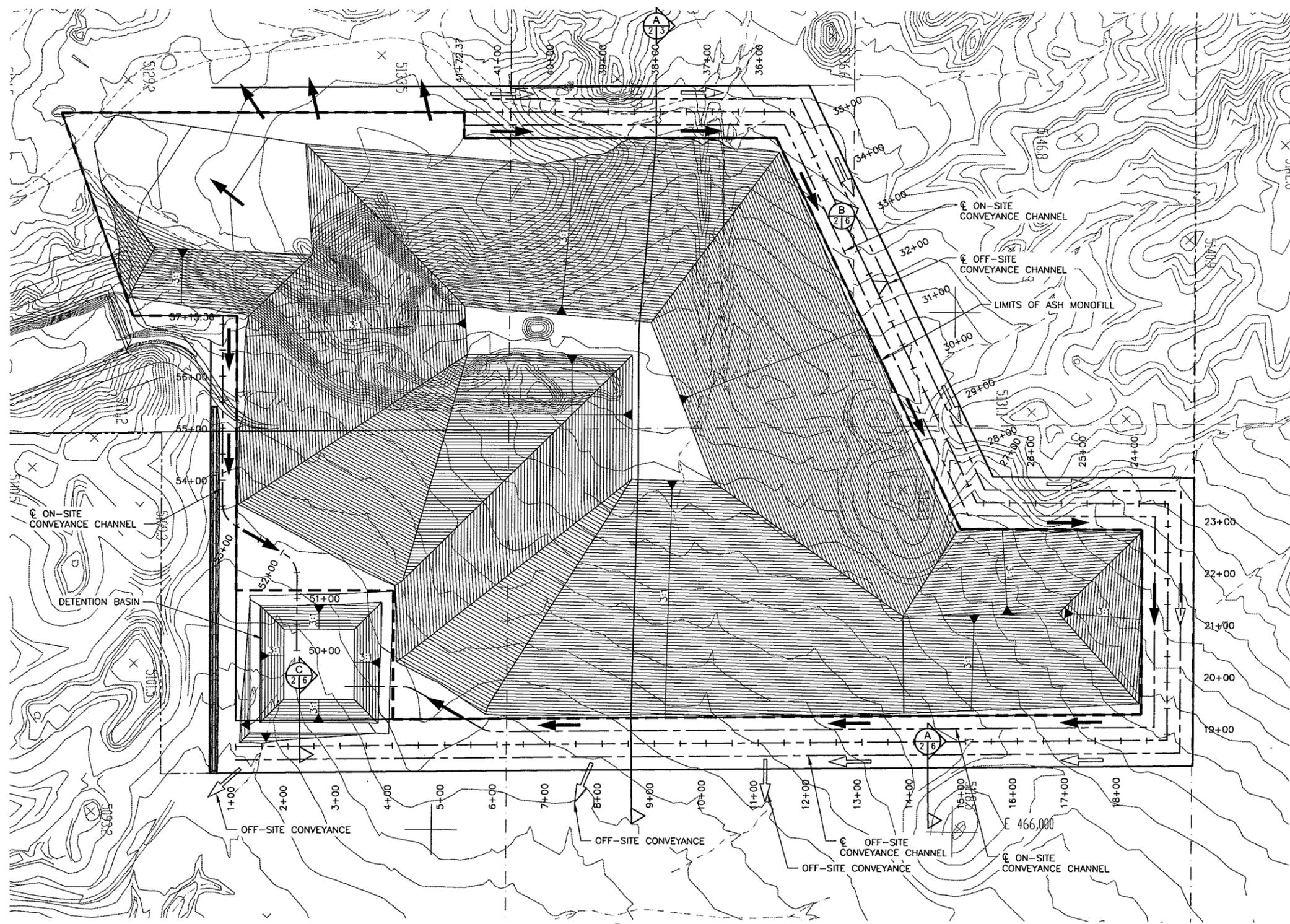


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CHOLLA GENERATING STATION
 ASH MONOFILL APP
 GENERAL SITE PLAN

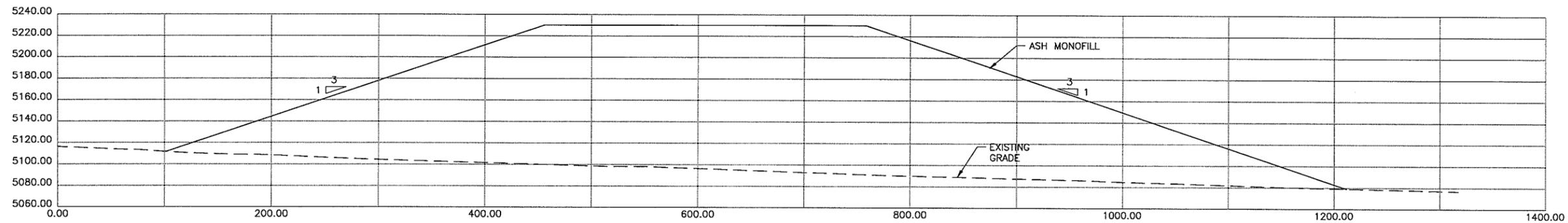


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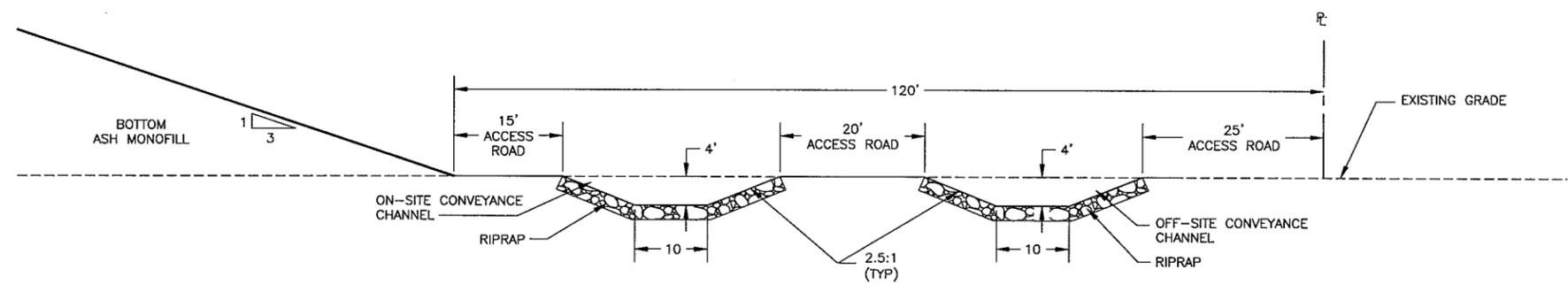
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SECTION A
 0 50 100
 SCALE IN FEET



SECTION B
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 SCALE IN FEET

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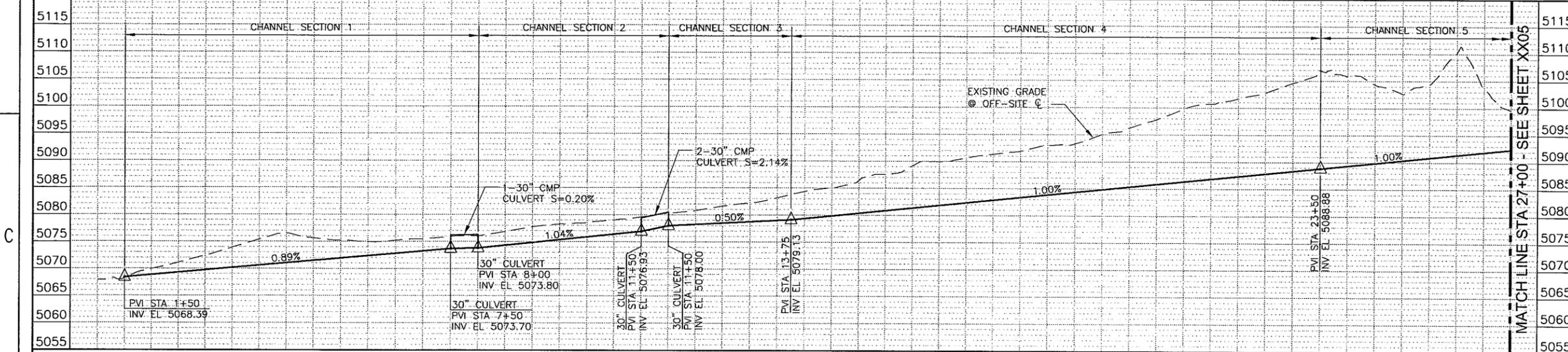
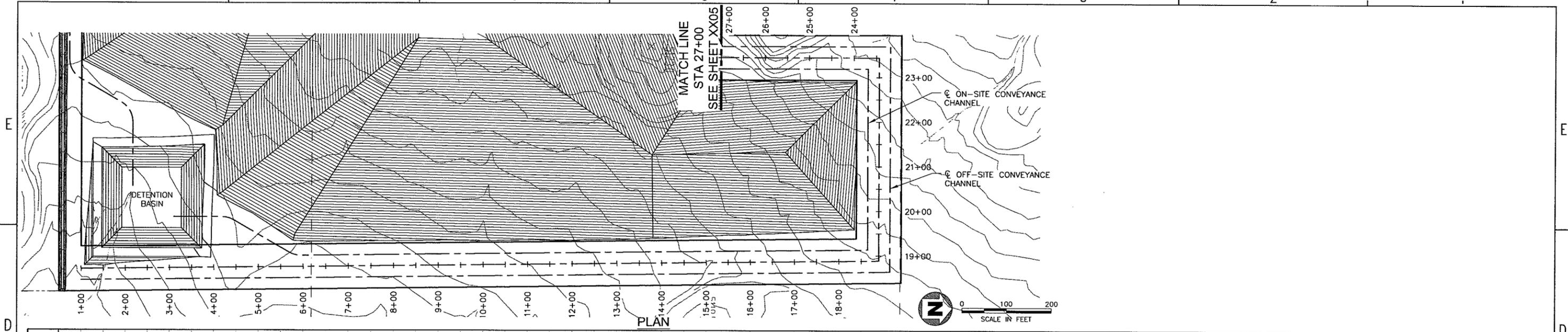
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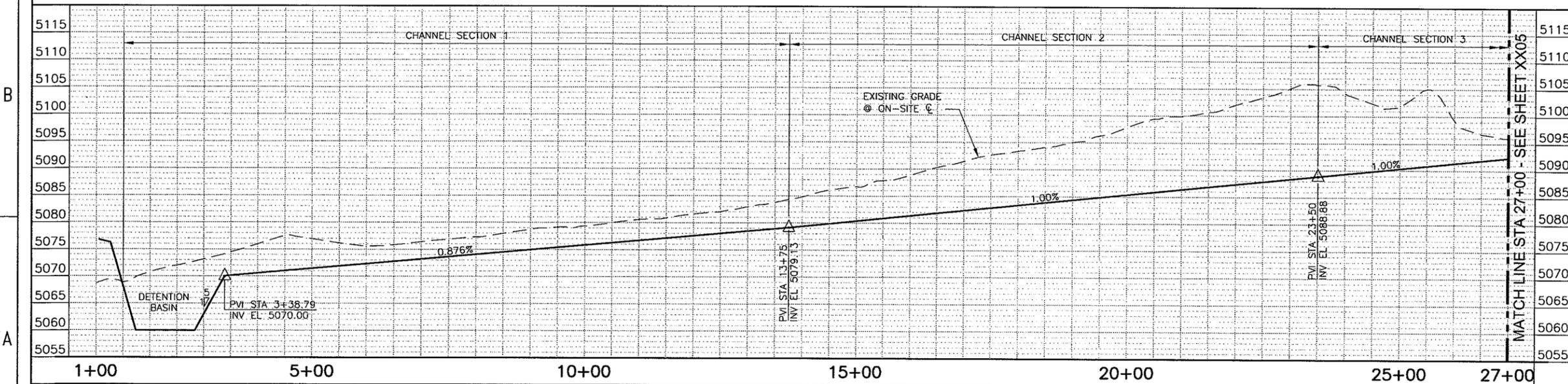
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OFFSITE CHANNEL PROFILE



ONSITE CHANNEL PROFILE

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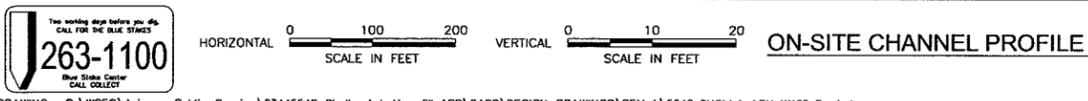
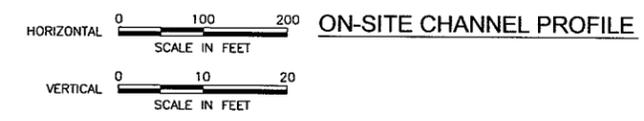
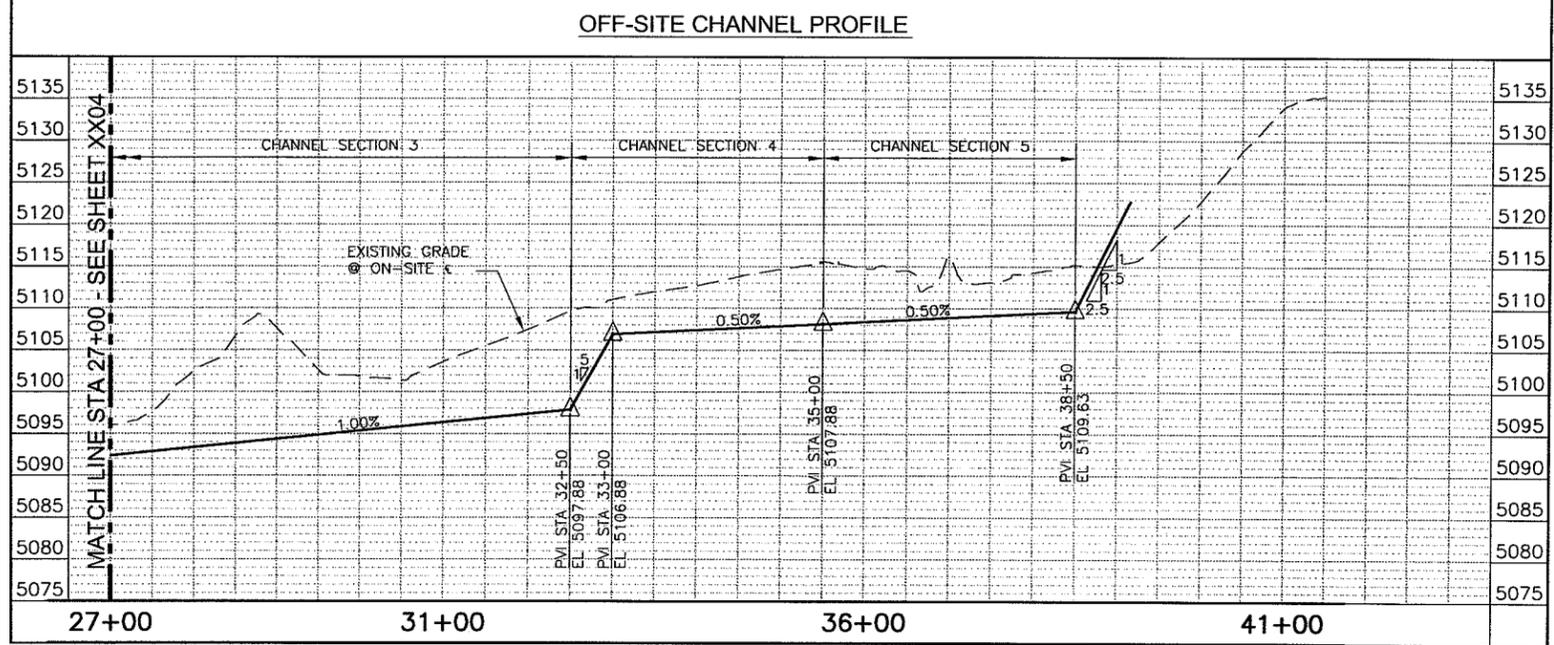
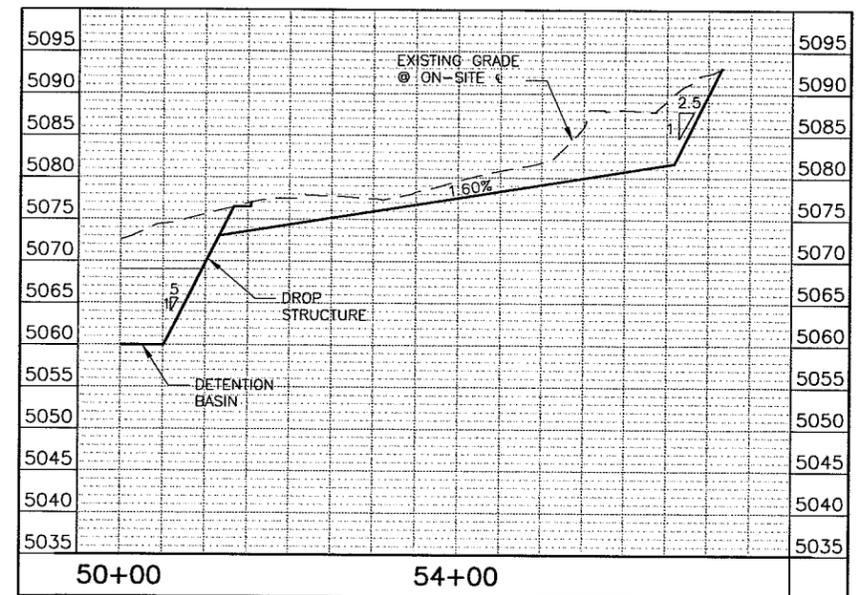
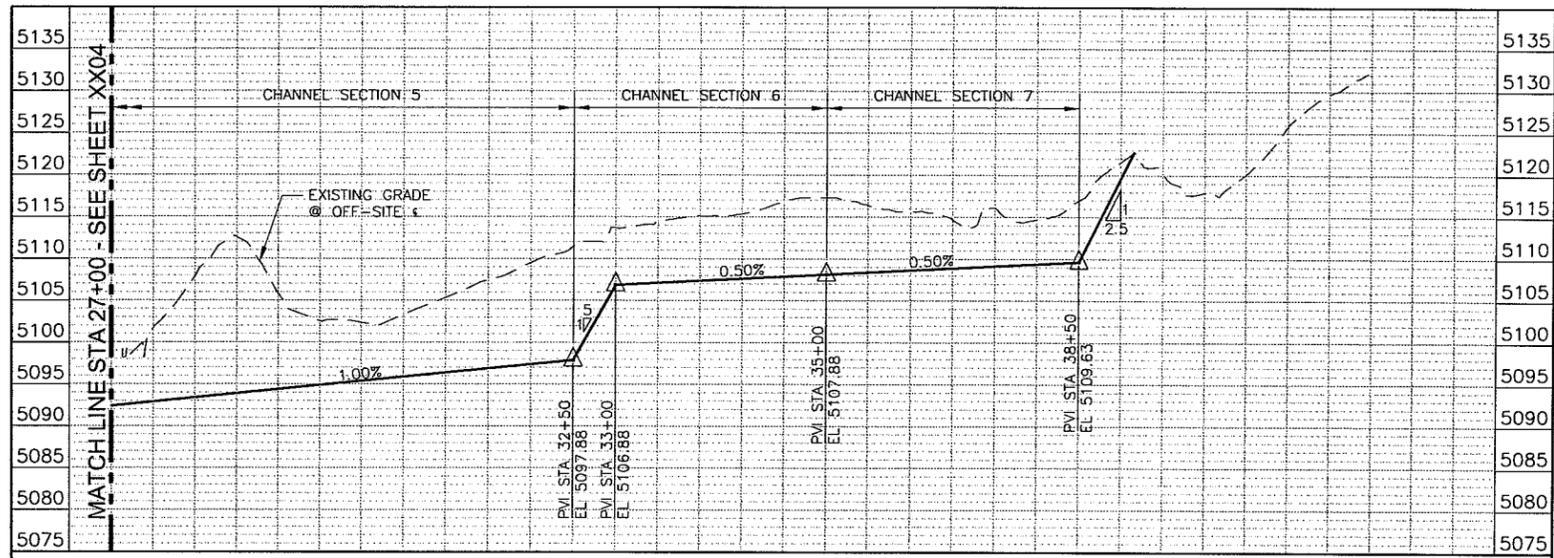
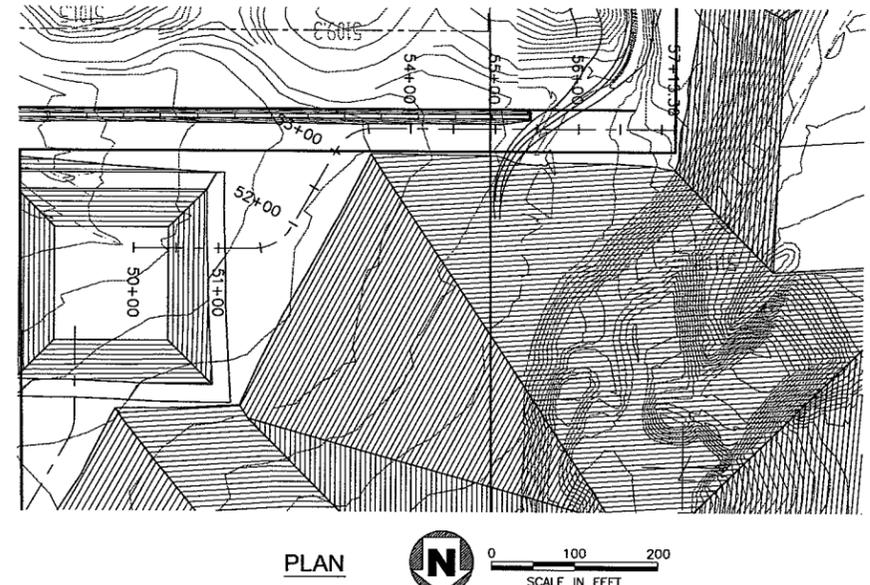
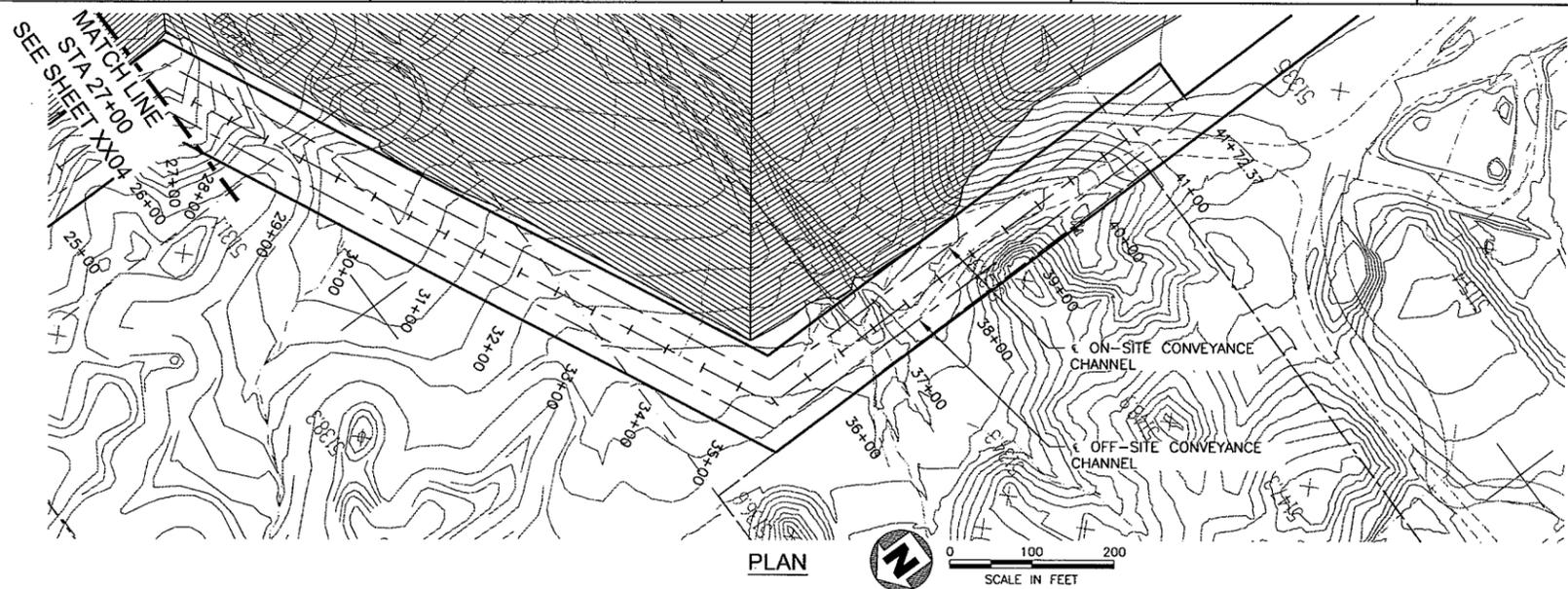
A		02-09	APP SUBMITTAL	RBZ	KLP	CDW	TER		
NO.	DATE	REVISION		DWN	CHD	EXD	RWD	APVD	W.A.
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PLAN AND PROFILE									
APS									
SCALE NOTED		DATE 02-09							
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CHD	KLP			ENGINEERING SUPERVISOR				XXX	
EXD	CDW	UNIT	DISC	TYPE	SYS	NUMBER	SHEET		
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HORIZONTAL SCALE IN FEET 0 100 200
 VERTICAL SCALE IN FEET 0 10 20

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A	02-09	APP SUBMITTAL	RBZ	KLP	CDW	TER			
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PLAN AND PROFILE									
APS									
SCALE NOTED DATE 02-09									
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EXD	KLP								
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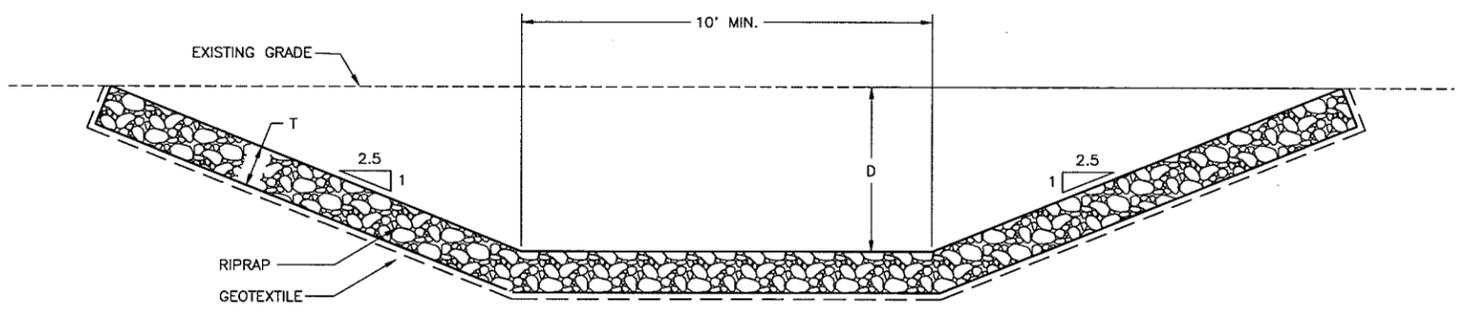
B

A

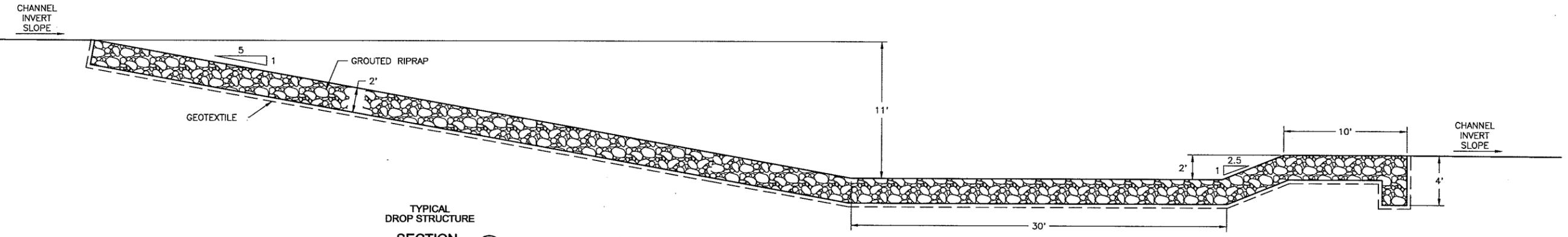
ON-SITE CHANNEL TABLE						
CHANNEL SECTION	STA	STA	RIPRAP D ₅₀ (IN)	T (FT)	CHANNEL DEPTH	CHANNEL SLOPE
1	1+50	13+75	4	1	3	0.876%
2	13+75	23+50	4	1	4	1.00%
3	23+50	32+50	4	1	4	1.00%
4	33+00	35+00	4	1	4	0.50%
5	35+00	38+50	4	1	4	0.50%
1	50+00	56+58.13	4	1	4	1.60%

OFF-SITE CHANNEL TABLE						
CHANNEL SECTION	STA	STA	RIPRAP D ₅₀ (IN)	T (FT)	CHANNEL DEPTH	CHANNEL SLOPE
1	1+50	7+50	4	1	3	0.885%
2	8+00	11+00	4	1	4	1.04%
3	11+50	13+75	4	1	4	0.50%
4	13+75	23+50	4/6	1	4	1.00%
5	23+50	32+50	4	1	4	1.00%
6	33+00	35+00	4	1	4	0.5%
7	35+00	38+50	4	1	3	0.5%

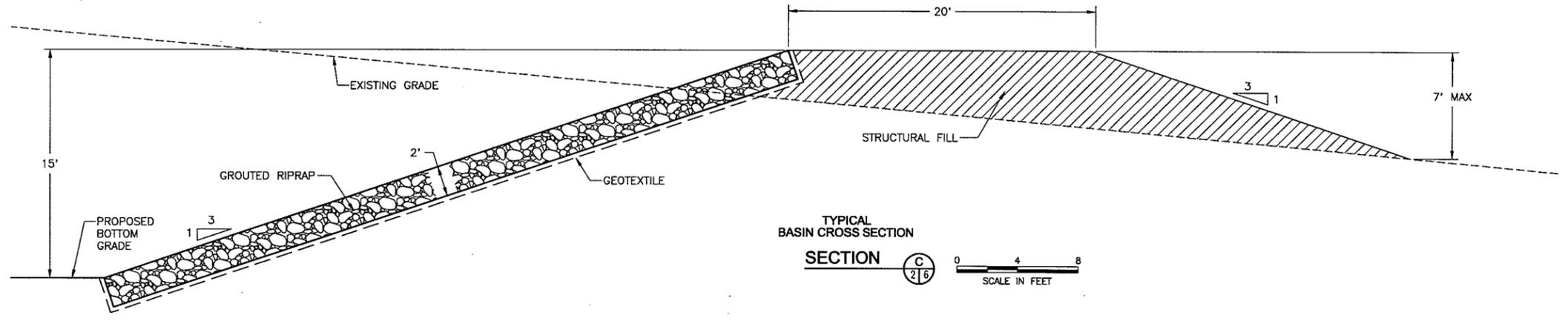
NOTE:
 1. DROP STRUCTURE D₅₀=1FT, THICKNESS IS 2xD₅₀=2FT AND WILL BE GROUTED.



TYPICAL CHANNEL SECTION (A) 2/6
 SCALE IN FEET



TYPICAL DROP STRUCTURE SECTION (B) 2/6
 SCALE IN FEET



TYPICAL BASIN CROSS SECTION (C) 2/6
 SCALE IN FEET

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CHOLLA GENERATING STATION ASH MONOFILL APP																		
SECTION AND DETAILS																		
APS																		
SCALE NOTED DATE 02-09																		
DWN	RBZ	APPROVED														W.A.		
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EXD	CDW	UNIT	DISC	TYPE	SYS	NUMBER	SHEET											
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APPENDIX A
RAINFALL CALCULATIONS

CALCULATION COVER SHEET

Client: Arizona Public Service Project Name: Cholla Ash Monofill

Project/Calculation Number: 23445548

Title: Rainfall Data for the Rational Method

Total Number of Pages (including cover sheet): 24

Total Number of Computer Runs: _____

Prepared by: Michelle C. West, EIT *Michelle C. West* Date: 2/3/2009

Checked by: Danette Lucas, EIT *Danette Lucas* Date: 2/5/2009

Description and Purpose:

The purpose of this calculation was to determine the rainfall data required in the Rational Method calculation package.

Included in this package is the rainfall data including I-D-F worksheets required for the ADOT method as well as the I-D-F curves.

Design Basis/References/Assumptions

The rainfall data was based on NOAA Atlas 14, and the rainfall AZ zone map has been included. The NOAA data was located based on Latitude and Longitude coordinates for the Cholla Bottom Ash Dam near the project area.

The I-D-F worksheet and I-D-F curve printouts were generated from an excel spreadsheet located: P:\WRES\Arizona_Public_Service\23445548_Cholla_Ash Monofill APP\Channel Design\Hydrology\ADOT IDF-Rainfall-Data.xls

Remarks/Conclusions/Results:

See attached worksheets

Calculation Approved by: _____

Project Manager/Date

Revision No.:

Description of Revision:

Approved by:

Project Manager/Date

RAINFALL DATA CALCULATION
CHOLLA ASH MONOFILL HYDROLOGY ANALYSIS
CHOLLA GENERATING STATION
ARIZONA PUBLIC SERVICE

Problem Statement

The object of this calculation is to determine the rainfall data required in the Rational Method calculation package for the hydrology analysis of the proposed Cholla Ash Monofill.

The Rainfall Data was calculated using the procedure outlined in the Arizona Department of Transportation (ADOT) Highway Drainage Design Manual Hydrology.

Required Deliverables

- I-D-F worksheets and I-D-F curves for use with the Rational Method as required by the ADOT method.
- Rainfall intensity and depth for 100-year storm event, 24-hour duration.

Data Available

- Cholla Bottom Ash Dam location (Latitude 34.97 N, Longitude 110.29 W)
- NOAA Atlas 14 Rainfall Data for site specific latitude and longitude
- Arizona Zone Rainfall Map

Results

The printout I-D-F worksheets and I-D-F curves were generated from the following Excel spreadsheet are attached:

P:\WRES\Arizona_Public_Service\23445548_Cholla_Ash Monofill APP\Channel Design\Hydrology\ADOT IDF-Rainfall-Data.xls

REFERENCES

NOAA Atlas 14 Point Precipitation Frequency Estimates. www.noaa.gov.

ADOT Highway Drainage Design Manual Hydrology. March 1993.

WORKSHEETS

	A	B	C	D	E	F	G	H	I
1	ARIZONA DEPARTMENT OF TRANSPORTATION								
2	HYDROLOGIC DESIGN DATA								
3									
4	Project No.	23445548							
5	Project Name	Cholla Power Plant - Ash Fill			Date	2/3/2009			
6	Location/Station	Joseph City, AZ							
7	Designer	MCW			Checker	DRL			
8									
9	Figure 1-2								
10	RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET								
11									
12	PART A								
13									
14	Determine rainfall depths from Precipitation Data (NOAA Atlas 14):								
15									
16	2-year, 6-hour						$P_{2,6'}$ =	0.96	
17	2-year, 24-hour						$P_{2,24'}$ =	1.25	
18	100-year, 6-hour						$P_{100,6'}$ =	2.28	
19	100-year, 24-hour						$P_{100,24'}$ =	2.77	
20									
21	PART B								
22									
23	Compute the following:								
24									
25	2-year, 1-hour	$-0.011+0.942(P_{2,6'})^2$					$P_{2,1'}$ =	0.68	
26		$(P_{2,24'})$							
27	100-year, 1-hour	$0.494+0.755(P_{100,6'})^2$					$P_{100,1'}$ =	1.91	
28		$(P_{100,24'})$							
29	2-year, 2-hour	$0.341(P_{2,6'})+0.659(P_{2,1'})$					$P_{2,2'}$ =	0.78	
30	2-year, 3-hour	$0.569(P_{2,6'})+0.431(P_{2,1'})$					$P_{2,3'}$ =	0.84	
31	2-year, 12-hour	$0.500(P_{2,6'})+0.500(P_{2,24'})$					$P_{2,12'}$ =	1.11	
32	100-year, 2-hour	$0.341(P_{100,6'})+0.659(P_{100,1'})$					$P_{100,2'}$ =	2.04	
33	100-year, 3-hour	$0.569(P_{100,6'})+0.431(P_{100,1'})$					$P_{100,3'}$ =	2.12	
34	100-year, 12-hour	$0.500(P_{100,6'})+0.500(P_{100,24'})$					$P_{100,12'}$ =	2.53	

	A	B	C	D	E	F	G	H	I
1	ARIZONA DEPARTMENT OF TRANSPORTATION								
2	HYDROLOGIC DESIGN DATA								
3									
4	Project No.	23445548			Date	39847			
5	Project Name	Cholla Power Plant - Ash Fill			Date	39847			
6	Location/Station	Joseph City, AZ			Checker	DRL			
7	Location/Station	MCW			Checker	DRL			
8									
9	Figure 1-2								
10	RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET								
11									
12	PART A - FORMULAS								
13									
14	Determine rainfall depths from isopluvial maps (ADOT Highway Drainage Design Manual, Appendix B):								
15									
16	2-year, 6-hour				$P_{2,6} =$	0.96			
17	2-year, 24-hour				$P_{2,24} =$	1.25			
18	100-year, 6-hour				$P_{100,6} =$	2.28			
19	100-year, 24-hour				$P_{100,24} =$	2.77			
20									
21	PART B - FORMULAS								
22									
23	Compute the following:								
24									
25	2-year, 1-hour	$-0.011 + 0.942(P_{2,6})^2$			$P_{2,1} = (-0.011 + (0.942 * 116^2) / 117)$				
26		$(P_{2,24})$							
27	100-year, 1-hour	$0.494 + 0.755(P_{100,6})^2$			$P_{100,1} = 0.494 + (0.755 * 118^2) / 119$				
28		$(P_{100,24})$							
29	2-year, 2-hour	$0.341(P_{2,6}) + 0.659(P_{2,1})$			$P_{2,2} = 0.341 * 116 + 0.659 * 117$				
30	2-year, 3-hour	$0.569(P_{2,6}) + 0.431(P_{2,1})$			$P_{2,3} = 0.569 * 116 + 0.431 * 117$				
31	2-year, 12-hour	$0.500(P_{2,6}) + 0.500(P_{2,24})$			$P_{2,12} = 0.5 * 116 + 0.5 * 117$				
32	100-year, 2-hour	$0.341(P_{100,6}) + 0.659(P_{100,1})$			$P_{100,2} = 0.341 * 118 + 0.659 * 119$				
33	100-year, 3-hour	$0.569(P_{100,6}) + 0.431(P_{100,1})$			$P_{100,3} = 0.569 * 118 + 0.431 * 119$				
34	100-year, 12-hour	$0.500(P_{100,6}) + 0.500(P_{100,24})$			$P_{100,12} = 0.5 * 118 + 0.5 * 119$				

	A	B	C	D	E	F	G	H	I
1	Figure 1-2 RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET								
2									
3									
4									

	PART C								
--	---------------	--	--	--	--	--	--	--	--

6 Determine the short-duration rainfall zone (**Figure 1-1**):

9 Zone = 6

11 Determine the short duration rainfall ratios (**Table 1-1**):

Duration (Minutes)	Ratio	
	2-Year	100-Year
5	A= 0.35	E= 0.32
10	B= 0.54	F= 0.50
15	C= 0.65	G= 0.62
30	D= 0.83	H= 0.81

23 Compute the following:

2-year, 5-min	(A)(P _{2,1'})=	P _{2,5"} =	0.24
2-year, 10-min	(B)(P _{2,1'})=	P _{2,10"} =	0.37
2-year, 15-min	(C)(P _{2,1'})=	P _{2,15"} =	0.44
2-year, 30-min	(D)(P _{2,1'})=	P _{2,30"} =	0.57
100-year, 5-min	(E)(P _{100,1'})=	P _{100,5"} =	0.61
100-year, 10-min	(F)(P _{100,1'})=	P _{100,10"} =	0.96
100-year, 15-min	(G)(P _{100,1'})=	P _{100,15"} =	1.18
100-year, 30-min	(H)(P _{100,1'})=	P _{100,30"} =	1.55

	A	B	C	D	E	F	G	H	I
1	Figure 1-2								
2	RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET								
3									
4									

	PART C - FORMULAS								
--	--------------------------	--	--	--	--	--	--	--	--

7 Determine the short-duration rainfall zone (Figure 1-1):

8

9 Zone = 6

11 Determine the short duration rainfall ratios (Table 1-1):

Duration (Minutes)	Ratio	
	2-Year	100-Year
5	A= =IF(\$E\$9=6,0.35,IF(\$E\$9=8,0.34,"NG"))	E= =IF(\$E\$9=6,0.32,IF(\$E\$9=8,0.3,"NG"))
10	B= =IF(\$E\$9=6,0.54,IF(\$E\$9=8,0.51,"NG"))	F= =IF(\$E\$9=6,0.5,IF(\$E\$9=8,0.46,"NG"))
15	C= =IF(\$E\$9=6,0.65,IF(\$E\$9=8,0.62,"NG"))	G= =IF(\$E\$9=6,0.62,IF(\$E\$9=8,0.59,"NG"))
30	D= =IF(\$E\$9=6,0.83,IF(\$E\$9=8,0.82,"NG"))	H= =IF(\$E\$9=6,0.81,IF(\$E\$9=8,0.8,"NG"))

23 Compute the following:

2-year, 5-min	(A)(P _{2,1})=	P _{2,5} = =E15*Parts A & B!\$I25
2-year, 10-min	(B)(P _{2,1})=	P _{2,10} = =E16*Parts A & B!\$I25
2-year, 15-min	(C)(P _{2,1})=	P _{2,15} = =E17*Parts A & B!\$I25
2-year, 30-min	(D)(P _{2,1})=	P _{2,30} = =E18*Parts A & B!\$I25
100-year, 5-min	(E)(P _{100,1})=	P _{100,5} = ='Parts A & B!\$I27*Part C!G15
100-year, 10-min	(F)(P _{100,1})=	P _{100,10} = ='Parts A & B!\$I27*Part C!G16
100-year, 15-min	(G)(P _{100,1})=	P _{100,15} = ='Parts A & B!\$I27*Part C!G17
100-year, 30-min	(H)(P _{100,1})=	P _{100,30} = ='Parts A & B!\$I27*Part C!G18

	A	B	C	D	E	F	G	H	I	J
1	Figure 1-2									
2	RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET									
3										
4										

	PART D									
--	--------	--	--	--	--	--	--	--	--	--

6

7

8 For any flood frequency (T-yr) other than the 2-year or 100-year, calculate the rainfall depth
 9 for each rainfall duration (t) by the following equation:

10

$$P_{T,t} = (X)(P_{2,t}) + (Y)(P_{100,t})$$

11

12

Frequency (T-yr)	X	Y
5-year	0.674	0.278
10-year	0.496	0.449
25-year	0.293	0.669
50-year	0.146	0.835
500-year	-0.337	1.381

13

14

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18

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20

21

22 Selected Frequency (T-year)

23

24 5 yr X= 0.674 Y= 0.278

25

26	5-min	(X)(P _{2,5'}) + (Y)(P _{100,5'}) =	(P _{5,5'}) =	0.33
27	10-min	(X)(P _{2,10'}) + (Y)(P _{100,10'}) =	(P _{5,10'}) =	0.51
28	15-min	(X)(P _{2,15'}) + (Y)(P _{100,15'}) =	(P _{5,15'}) =	0.63
29	30-min	(X)(P _{2,30'}) + (Y)(P _{100,30'}) =	(P _{5,30'}) =	0.81
30	1-hour	(X)(P _{2,1'}) + (Y)(P _{100,1'}) =	(P _{5,1'}) =	0.99
31	2-hour	(X)(P _{2,2'}) + (Y)(P _{100,2'}) =	(P _{5,2'}) =	1.09
32	3-hour	(X)(P _{2,3'}) + (Y)(P _{100,3'}) =	(P _{5,3'}) =	1.16
33	6-hour	(X)(P _{2,6'}) + (Y)(P _{100,6'}) =	(P _{5,6'}) =	1.28
34	12-hour	(X)(P _{2,12'}) + (Y)(P _{100,12'}) =	(P _{5,12'}) =	1.45
35	24-hour	(X)(P _{2,24'}) + (Y)(P _{100,24'}) =	(P _{5,24'}) =	1.61

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46 NOTE: P_{5,5'} : 5 = flood frequency Year, 5" = rainfall duration
 47 "stands for minutes, ' stands for hours.

**Figure 1-2
RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET**

PART D

For any flood frequency (T-yr) other than the 2-year or 100-year, calculate the rainfall depth for each rainfall duration (t) by the following equation:

$$P_{T,t} = (X)(P_{2,t}) + (Y)(P_{100,t})$$

Frequency (T-yr)	X	Y
5-year	0.674	0.278
10-year	0.496	0.449
25-year	0.293	0.669
50-year	0.146	0.835
500-year	-0.337	1.381

Selected Frequency (T-year)

10 yr X= 0.496 Y= 0.449

5-min	$(X)(P_{2,5'}) + (Y)(P_{100,5'}) =$	$(P_{10,5'}) =$	<u>0.39</u>
10-min	$(X)(P_{2,10'}) + (Y)(P_{100,10'}) =$	$(P_{10,10'}) =$	<u>0.61</u>
15-min	$(X)(P_{2,15'}) + (Y)(P_{100,15'}) =$	$(P_{10,15'}) =$	<u>0.75</u>
30-min	$(X)(P_{2,30'}) + (Y)(P_{100,30'}) =$	$(P_{10,30'}) =$	<u>0.98</u>
1-hour	$(X)(P_{2,1'}) + (Y)(P_{100,1'}) =$	$(P_{10,1'}) =$	<u>1.20</u>
2-hour	$(X)(P_{2,2'}) + (Y)(P_{100,2'}) =$	$(P_{10,2'}) =$	<u>1.30</u>
3-hour	$(X)(P_{2,3'}) + (Y)(P_{100,3'}) =$	$(P_{10,3'}) =$	<u>1.37</u>
6-hour	$(X)(P_{2,6'}) + (Y)(P_{100,6'}) =$	$(P_{10,6'}) =$	<u>1.50</u>
12-hour	$(X)(P_{2,12'}) + (Y)(P_{100,12'}) =$	$(P_{10,12'}) =$	<u>1.68</u>
24-hour	$(X)(P_{2,24'}) + (Y)(P_{100,24'}) =$	$(P_{10,24'}) =$	<u>1.86</u>

	A	B	C	D	E	F	G	H	I	J																																																			
1	Figure 1-2 RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET																																																												
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Figure 1-2
RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET

PART D

For any flood frequency (T-yr) other than the 2-year or 100-year, calculate the rainfall depth for each rainfall duration (t) by the following equation:

$$P_{T,t} = (X)(P_{2,t}) + (Y)(P_{100,t})$$

Frequency (T-yr)	X	Y
5-year	0.674	0.278
10-year	0.496	0.449
25-year	0.293	0.669
50-year	0.146	0.835
500-year	-0.337	1.381

Selected Frequency (T-year)

50 yr X= 0.146 Y= 0.835

5-min	$(X)(P_{2,5'}) + (Y)(P_{100,5'}) =$	$(P_{50,5'}) =$	<u>0.55</u>
10-min	$(X)(P_{2,10'}) + (Y)(P_{100,10'}) =$	$(P_{50,10'}) =$	<u>0.85</u>
15-min	$(X)(P_{2,15'}) + (Y)(P_{100,15'}) =$	$(P_{50,15'}) =$	<u>1.05</u>
30-min	$(X)(P_{2,30'}) + (Y)(P_{100,30'}) =$	$(P_{50,30'}) =$	<u>1.38</u>
1-hour	$(X)(P_{2,1'}) + (Y)(P_{100,1'}) =$	$(P_{50,1'}) =$	<u>1.70</u>
2-hour	$(X)(P_{2,2'}) + (Y)(P_{100,2'}) =$	$(P_{50,2'}) =$	<u>1.81</u>
3-hour	$(X)(P_{2,3'}) + (Y)(P_{100,3'}) =$	$(P_{50,3'}) =$	<u>1.89</u>
6-hour	$(X)(P_{2,6'}) + (Y)(P_{100,6'}) =$	$(P_{50,6'}) =$	<u>2.04</u>
12-hour	$(X)(P_{2,12'}) + (Y)(P_{100,12'}) =$	$(P_{50,12'}) =$	<u>2.27</u>
24-hour	$(X)(P_{2,24'}) + (Y)(P_{100,24'}) =$	$(P_{50,24'}) =$	<u>2.50</u>

**Figure 1-2
RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET**

PART D

For any flood frequency (T-yr) other than the 2-year or 100-year, calculate the rainfall depth for each rainfall duration (t) by the following equation:

$$P_{T,t} = (X)(P_{2,t}) + (Y)(P_{100,t})$$

Frequency (T-yr)	X	Y
5-year	0.674	0.278
10-year	0.496	0.449
25-year	0.293	0.669
50-year	0.146	0.835
500-year	-0.337	1.381

Selected Frequency (T-year)

500 yr X= -0.337 Y= 1.381

5-min	$(X)(P_{2,5'}) + (Y)(P_{100,5'}) =$	$(P_{500,5'}) =$	<u>0.76</u>
10-min	$(X)(P_{2,10'}) + (Y)(P_{100,10'}) =$	$(P_{500,10'}) =$	<u>1.20</u>
15-min	$(X)(P_{2,15'}) + (Y)(P_{100,15'}) =$	$(P_{500,15'}) =$	<u>1.49</u>
30-min	$(X)(P_{2,30'}) + (Y)(P_{100,30'}) =$	$(P_{500,30'}) =$	<u>1.95</u>
1-hour	$(X)(P_{2,1'}) + (Y)(P_{100,1'}) =$	$(P_{500,1'}) =$	<u>2.41</u>
2-hour	$(X)(P_{2,2'}) + (Y)(P_{100,2'}) =$	$(P_{500,2'}) =$	<u>2.55</u>
3-hour	$(X)(P_{2,3'}) + (Y)(P_{100,3'}) =$	$(P_{500,3'}) =$	<u>2.65</u>
6-hour	$(X)(P_{2,6'}) + (Y)(P_{100,6'}) =$	$(P_{500,6'}) =$	<u>2.83</u>
12-hour	$(X)(P_{2,12'}) + (Y)(P_{100,12'}) =$	$(P_{500,12'}) =$	<u>3.11</u>
24-hour	$(X)(P_{2,24'}) + (Y)(P_{100,24'}) =$	$(P_{500,24'}) =$	<u>3.40</u>

Figure 1-2
RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET

PART D - FORMULAS

For any flood frequency (T-yr) other than the 2-year or 100-year, calculate the rainfall depth for each rainfall duration (t) by the following equation:

$$P_{T,t} = (X)(P_{2,t}) + (Y)(P_{100,t})$$

Frequency (T-yr)	X	Y
5-year	0.674	0.278
10-year	0.496	0.449
25-year	0.293	0.669
50-year	0.146	0.835
500-year	-0.337	1.381

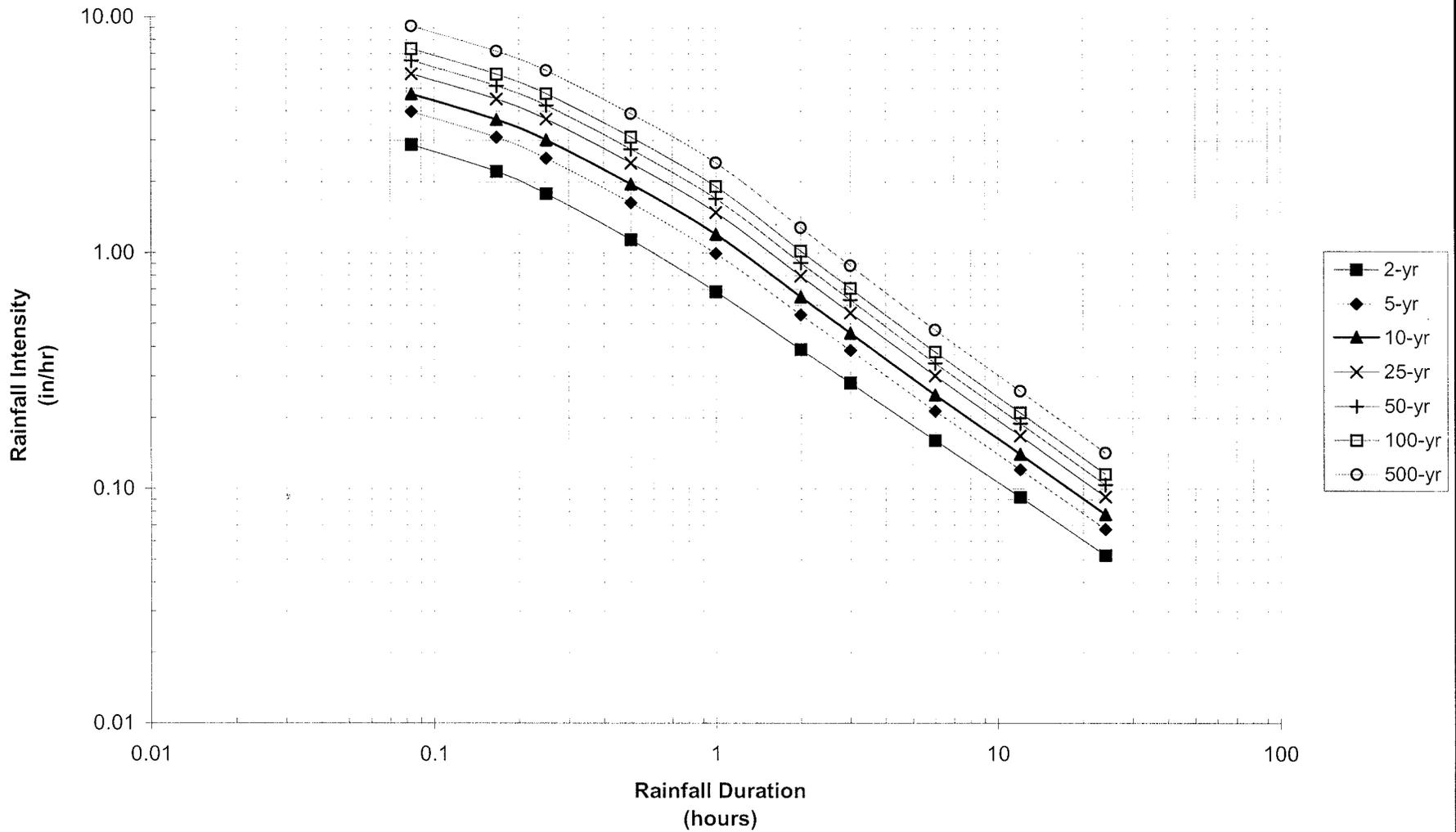
Selected Frequency (T-year)

500	yr	X= =IF(\$B\$24=5,D15,IF(\$B\$24=10,D16,IF(\$B\$24=25,D17,IF(\$B\$24=50,D18,IF(\$B\$24=500,D19,"NG")))))	Y= =IF(\$B\$24=5,E15,IF(\$B\$24=10,E16,IF(\$B\$24=25,E17,IF(\$B\$24=50,E18,IF(\$B\$24=500,E19,"NG")))))
5-min		$(X)(P_{2,5'}) + (Y)(P_{100,5'}) =$	$(P_{=B$24,5'}) =$ =SD\$24*Part C!G25+Part D!\$F\$24*Part C!G33
10-min		$(X)(P_{2,10'}) + (Y)(P_{100,10'}) =$	$(P_{=B$24,10'}) =$ =SD\$24*Part C!G27+Part D!\$F\$24*Part C!G35
15-min		$(X)(P_{2,15'}) + (Y)(P_{100,15'}) =$	$(P_{=B$24,15'}) =$ =SD\$24*Part C!G29+Part D!\$F\$24*Part C!G37
30-min		$(X)(P_{2,30'}) + (Y)(P_{100,30'}) =$	$(P_{=B$24,30'}) =$ =SD\$24*Part C!G31+Part D!\$F\$24*Part C!G39
1-hour		$(X)(P_{2,1'}) + (Y)(P_{100,1'}) =$	$(P_{=B$24,1'}) =$ =SD\$24*Parts A & B!I25+Part D!\$F\$24*Parts A & B!I27
2-hour		$(X)(P_{2,2'}) + (Y)(P_{100,2'}) =$	$(P_{=B$24,2'}) =$ =SD\$24*Parts A & B!I29+Part D!\$F\$24*Parts A & B!I32
3-hour		$(X)(P_{2,3'}) + (Y)(P_{100,3'}) =$	$(P_{=B$24,3'}) =$ =SD\$24*Parts A & B!I30+Part D!\$F\$24*Parts A & B!I33
6-hour		$(X)(P_{2,6'}) + (Y)(P_{100,6'}) =$	$(P_{=B$24,6'}) =$ =SD\$24*Parts A & B!I16+Part D!\$F\$24*Parts A & B!I18
12-hour		$(X)(P_{2,12'}) + (Y)(P_{100,12'}) =$	$(P_{=B$24,12'}) =$ =SD\$24*Parts A & B!I31+Part D!\$F\$24*Parts A & B!I34
24-hour		$(X)(P_{2,24'}) + (Y)(P_{100,24'}) =$	$(P_{=B$24,24'}) =$ =SD\$24*Parts A & B!I17+Part D!\$F\$24*Parts A & B!I19

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											2	5	10	25	50	100	500																																																																																															
5-min											0.24	0.33	0.39	0.48	0.55	0.61	0.76																																																																																															
10-min											0.37	0.51	0.61	0.75	0.85	0.96	1.20																																																																																															
15-min											0.44	0.63	0.75	0.92	1.05	1.18	1.49																																																																																															
30-min											0.57	0.81	0.98	1.20	1.38	1.55	1.95																																																																																															
1-hour											0.68	0.99	1.20	1.48	1.70	1.91	2.41																																																																																															
2-hour											0.78	1.09	1.30	1.59	1.81	2.04	2.55																																																																																															
3-hour											0.84	1.16	1.37	1.67	1.89	2.12	2.65																																																																																															
6-hour											0.96	1.28	1.50	1.81	2.04	2.28	2.83																																																																																															
12-hour											1.11	1.45	1.68	2.01	2.27	2.53	3.11																																																																																															
24-hour											1.25	1.61	1.86	2.22	2.50	2.77	3.40																																																																																															
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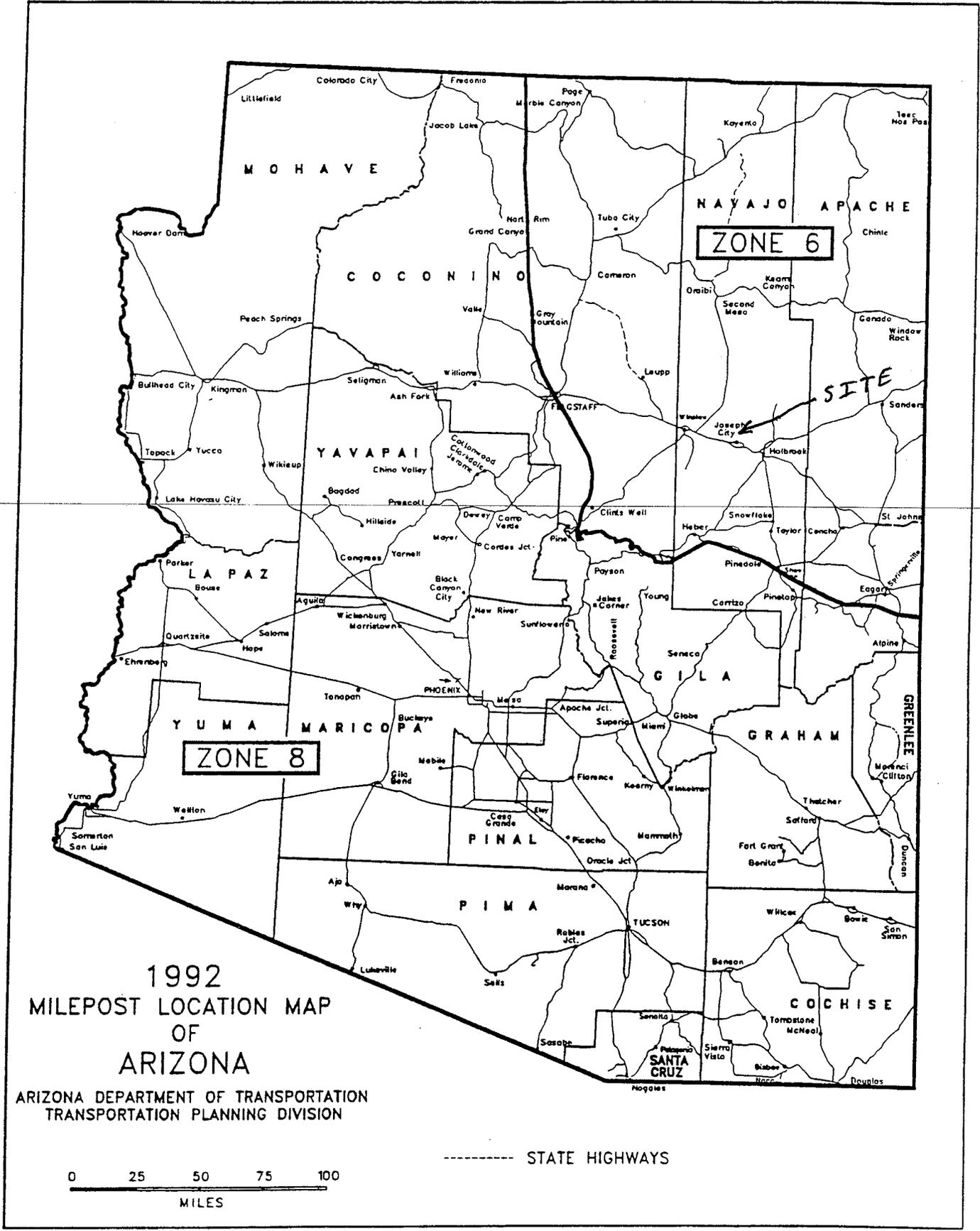
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1	Figure 1-2 RAINFALL DEPTH-DURATION-FREQUENCY (D-D-F) WORKSHEET																																																																																																																
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5	PART E - FORMULAS																																																																																																																
6	Tabulate the rainfall Depth-Duration-Frequency statistics below: <table border="1" style="margin: 10px auto; width: 80%;"> <thead> <tr> <th rowspan="2">Duration</th> <th colspan="7">Rainfall Depth, In Inches</th> </tr> <tr> <th colspan="7">Frequency, In Years</th> </tr> <tr> <th></th> <th>2</th> <th>5</th> <th>10</th> <th>25</th> <th>50</th> <th>100</th> <th>500</th> </tr> </thead> <tbody> <tr> <td>5-min</td> <td>=Part C!G25</td> <td>=Part D, 5-yr!J26</td> <td>=Part D, 10-yr!\$J26</td> <td>=Part D, 25-yr!\$J26</td> <td>=Part D, 50-yr!\$J26</td> <td>=Part C!G33</td> <td>=Part D, 500-yr !J26</td> </tr> <tr> <td>10-min</td> <td>=Part C!G27</td> <td>=Part D, 5-yr!J28</td> <td>=Part D, 10-yr!\$J28</td> <td>=Part D, 25-yr!\$J28</td> <td>=Part D, 50-yr!\$J28</td> <td>=Part C!G35</td> <td>=Part D, 500-yr !J28</td> </tr> <tr> <td>15-min</td> <td>=Part C!G29</td> <td>=Part D, 5-yr!J30</td> <td>=Part D, 10-yr!\$J30</td> <td>=Part D, 25-yr!\$J30</td> <td>=Part D, 50-yr!\$J30</td> <td>=Part C!G37</td> <td>=Part D, 500-yr !J30</td> </tr> <tr> <td>30-min</td> <td>=Part C!G31</td> <td>=Part D, 5-yr!J32</td> <td>=Part D, 10-yr!\$J32</td> <td>=Part D, 25-yr!\$J32</td> <td>=Part D, 50-yr!\$J32</td> <td>=Part C!G39</td> <td>=Part D, 500-yr !J32</td> </tr> <tr> <td>1-hour</td> <td>=Parts A & B!!I25</td> <td>=Part D, 5-yr!J34</td> <td>=Part D, 10-yr!\$J34</td> <td>=Part D, 25-yr!\$J34</td> <td>=Part D, 50-yr!\$J34</td> <td>=Parts A & B!!I27</td> <td>=Part D, 500-yr !J34</td> </tr> <tr> <td>2-hour</td> <td>=Parts A & B!!I29</td> <td>=Part D, 5-yr!J36</td> <td>=Part D, 10-yr!\$J36</td> <td>=Part D, 25-yr!\$J36</td> <td>=Part D, 50-yr!\$J36</td> <td>=Parts A & B!!I32</td> <td>=Part D, 500-yr !J36</td> </tr> <tr> <td>3-hour</td> <td>=Parts A & B!!I30</td> <td>=Part D, 5-yr!J38</td> <td>=Part D, 10-yr!\$J38</td> <td>=Part D, 25-yr!\$J38</td> <td>=Part D, 50-yr!\$J38</td> <td>=Parts A & B!!I33</td> <td>=Part D, 500-yr !J38</td> </tr> <tr> <td>6-hour</td> <td>=Parts A & B!!I16</td> <td>=Part D, 5-yr!J40</td> <td>=Part D, 10-yr!\$J40</td> <td>=Part D, 25-yr!\$J40</td> <td>=Part D, 50-yr!\$J40</td> <td>=Parts A & B!!I18</td> <td>=Part D, 500-yr !J40</td> </tr> <tr> <td>12-hour</td> <td>=Parts A & B!!I31</td> <td>=Part D, 5-yr!J42</td> <td>=Part D, 10-yr!\$J42</td> <td>=Part D, 25-yr!\$J42</td> <td>=Part D, 50-yr!\$J42</td> <td>=Parts A & B!!I34</td> <td>=Part D, 500-yr !J42</td> </tr> <tr> <td>24-hour</td> <td>=Parts A & B!!I17</td> <td>=Part D, 5-yr!J44</td> <td>=Part D, 10-yr!\$J44</td> <td>=Part D, 25-yr!\$J44</td> <td>=Part D, 50-yr!\$J44</td> <td>=Parts A & B!!I19</td> <td>=Part D, 500-yr !J44</td> </tr> </tbody> </table>										Duration	Rainfall Depth, In Inches							Frequency, In Years								2	5	10	25	50	100	500	5-min	=Part C!G25	=Part D, 5-yr!J26	=Part D, 10-yr!\$J26	=Part D, 25-yr!\$J26	=Part D, 50-yr!\$J26	=Part C!G33	=Part D, 500-yr !J26	10-min	=Part C!G27	=Part D, 5-yr!J28	=Part D, 10-yr!\$J28	=Part D, 25-yr!\$J28	=Part D, 50-yr!\$J28	=Part C!G35	=Part D, 500-yr !J28	15-min	=Part C!G29	=Part D, 5-yr!J30	=Part D, 10-yr!\$J30	=Part D, 25-yr!\$J30	=Part D, 50-yr!\$J30	=Part C!G37	=Part D, 500-yr !J30	30-min	=Part C!G31	=Part D, 5-yr!J32	=Part D, 10-yr!\$J32	=Part D, 25-yr!\$J32	=Part D, 50-yr!\$J32	=Part C!G39	=Part D, 500-yr !J32	1-hour	=Parts A & B!!I25	=Part D, 5-yr!J34	=Part D, 10-yr!\$J34	=Part D, 25-yr!\$J34	=Part D, 50-yr!\$J34	=Parts A & B!!I27	=Part D, 500-yr !J34	2-hour	=Parts A & B!!I29	=Part D, 5-yr!J36	=Part D, 10-yr!\$J36	=Part D, 25-yr!\$J36	=Part D, 50-yr!\$J36	=Parts A & B!!I32	=Part D, 500-yr !J36	3-hour	=Parts A & B!!I30	=Part D, 5-yr!J38	=Part D, 10-yr!\$J38	=Part D, 25-yr!\$J38	=Part D, 50-yr!\$J38	=Parts A & B!!I33	=Part D, 500-yr !J38	6-hour	=Parts A & B!!I16	=Part D, 5-yr!J40	=Part D, 10-yr!\$J40	=Part D, 25-yr!\$J40	=Part D, 50-yr!\$J40	=Parts A & B!!I18	=Part D, 500-yr !J40	12-hour	=Parts A & B!!I31	=Part D, 5-yr!J42	=Part D, 10-yr!\$J42	=Part D, 25-yr!\$J42	=Part D, 50-yr!\$J42	=Parts A & B!!I34	=Part D, 500-yr !J42	24-hour	=Parts A & B!!I17	=Part D, 5-yr!J44	=Part D, 10-yr!\$J44	=Part D, 25-yr!\$J44	=Part D, 50-yr!\$J44	=Parts A & B!!I19	=Part D, 500-yr !J44
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ADOT Intensity-Duration-Frequency Curves



REFERENCES

FIGURE 1-1
SHORT-DURATION RAINFALL RATIO ZONES FOR ARIZONA



Navajo

Abitibi Consolidated Snowflake Division

State ID	NID	Dam Name	Hazard Class	Location	Dam Type	Height	Capacity	Dam Size	Req. IDF
09.35	AZ00227	Ash Lagoons	Low	S21,T13N,R19E	Earth	25	451	Small	100-year
09.36	AZ00231	Mill Pond	Low	S17,T13N,R19E	Earth	30	2200	Intermediate	

Arizona Game & Fish Department

State ID	NID	Dam Name	Hazard Class	Location	Dam Type	Height	Capacity	Dam Size	Req. IDF
09.20	AZ00042	Black Canyon	High	S24,T11N,R15E	Earth	60	1581	Intermediate	0.5 PMF
09.19	AZ00051	Fool Hollow	High	S12,T10N,R21E	Earthrock	60	3217	Intermediate	0.5 PMF

Arizona Public Service Company, Cholla Power Plant

State ID	NID	Dam Name	Hazard Class	Location	Dam Type	Height	Capacity	Dam Size	Req. IDF
09.27	AZ00178	Cholla Bottom Ash Pond	High	S13,T18N,R19E	Earth	73	2200	Intermediate	
09.28	AZ00179	Cholla Fly Ash Pond	High	S30,T18N,R20E	Earth	80	18000	Intermediate	

Arizona Public Service Company, Phoenix Office

State ID	NID	Dam Name	Hazard Class	Location	Dam Type	Height	Capacity	Dam Size	Req. IDF
09.29	AZ00180	Cholla Cooling Pond	Significant	S26,T18N,R19E	Earth	13	2200	Small	

City of Show Low

State ID	NID	Dam Name	Hazard Class	Location	Dam Type	Height	Capacity	Dam Size	Req. IDF
09.13	AZ00023	Jaques	High	S10,T9N,R22E	Earth	65	6000	Intermediate	

City of Winslow

State ID	NID	Dam Name	Hazard Class	Location	Dam Type	Height	Capacity	Dam Size	Req. IDF
09.03	AZ00057	Clear Creek #1	Low	S10,T18N,R16E	Masonry	7	350	Small	

<p>Graphical Locater</p>  <p>Home Page USA map XY- data TRS- data</p>	<p>Montana State University</p>  <p>Environmental Statistics Group</p>	<p>USF&WS</p> 	<p>USDOE</p> 
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The selected location is:

Latitude/Longitude 34.9584°N, 110.2784°W (34°, 57', 30.4" N; 110°, 16', 42.3" W)
 The legal description is: Arizona, Gila & Salt River Meridian T18N,R19E,sec13.
 UTM zone 12 (X,Y) 565879 , 3868672

sec's 11, 12, 13, 14

The elevation is 1554 m (5098 ft)
 The gradient is: 0.0 percent
 There is no aspect direction.
 The local roughness is: 0.0 or flat
 The location as decimal degrees (X,Y;Z) = -110.2784, 34.9584; 1554 m

The state and county are Arizona: Navajo County 4017
 The HUC is Middle Little Colorado 15020008; Place point in HUC
 The Omernik ecoregion is Arizona/New Mexico Plateau (less typical) 22
 The 1:100,000 map (if available); Switch to TerraServer
 Zoom on that location with radius = 2 km; 5 km; 10 km; 20 km; 30 km; custom.

Nearby named places (in order by distance)

1. Cholla Cooling Pond Dam; Arizona: Navajo Co. -110.2774, 34.9550 at a distance of 391 m
2. Cholla Fly Ash Pond; Arizona: Navajo Co. -110.2674, 34.9600 at a distance of 1023 m
3. Cholla Fly Ash Pond Dam; Arizona: Navajo Co. -110.2674, 34.9600 at a distance of 1023 m
4. Cholla Bottom Ash Pond; Arizona: Navajo Co. -110.2890, 34.9667 at a distance of 1332 m
5. Cholla Bottom Ash Pond Dam; Arizona: Navajo Co. -110.2890, 34.9667 at a distance of 1332 m
6. Cholla Power Generating Plant; Arizona: Navajo Co. -110.2979, 34.9394 at a distance of 2759 m
7. Cholla Lake; Arizona: Navajo Co. -110.2838, 34.9306 at a distance of 3125 m
8. The Old Fort Historical Monument; Arizona: Navajo Co. -110.3210, 34.9558 at a distance of 3898 m
9. Joseph City Elementary School; Arizona: Navajo Co. -110.3318, 34.9647 at a distance of 4923 m
10. Joseph City High School; Arizona: Navajo Co. -110.3318, 34.9653 at a distance of 4933 m



The 7.5 minute series topographic maps for that area

Humpy Camp Well	Blairs Spring	Lee Mountain
Apache Butte	Joseph City	Holbrook
Chimney Canyon	Saunders Draw	Porter Canyon

NOAA 14 TABLES



POINT PRECIPITATION FREQUENCY ESTIMATES FROM NOAA ATLAS 14



21

Arizona 34.9667 N 110.289 W 5200 feet

from "Precipitation-Frequency Atlas of the United States" NOAA Atlas 14, Volume 1, Version 4
G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M. Yekta, and D. Riley
NOAA, National Weather Service, Silver Spring, Maryland, 2006

Extracted: Thu Jan 22 2009

Confidence Limits	Seasonality	Location Maps	Other Info.	GIS data	Maps	Docs	Return to Sta
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Precipitation Frequency Estimates (inches)																		
ARI* (years)	5 min	10 min	15 min	30 min	60 min	120 min	3 hr	6 hr	12 hr	24 hr	48 hr	4 day	7 day	10 day	20 day	30 day	45 day	60 day
1	0.18	0.27	0.33	0.45	0.55	0.63	0.68	0.77	0.88	1.00	1.12	1.25	1.43	1.57	2.04	2.39	2.87	3.30
2	0.23	0.34	0.43	0.58	0.71	0.81	0.86	0.96	1.09	1.25	1.40	1.55	1.77	1.96	2.54	2.97	3.56	4.09
5	0.31	0.47	0.58	0.78	0.96	1.07	1.11	1.21	1.36	1.57	1.74	1.92	2.18	2.40	3.09	3.61	4.32	4.92
10	0.37	0.57	0.70	0.94	1.17	1.29	1.33	1.43	1.57	1.83	2.01	2.22	2.51	2.74	3.52	4.08	4.89	5.54
25	0.47	0.71	0.88	1.19	1.47	1.62	1.65	1.74	1.87	2.19	2.39	2.63	2.95	3.20	4.07	4.68	5.60	6.31
50	0.55	0.83	1.03	1.38	1.71	1.89	1.91	2.00	2.10	2.47	2.68	2.95	3.29	3.54	4.48	5.12	6.11	6.85
100	0.63	0.96	1.19	1.60	1.99	2.18	2.20	2.28	2.34	2.77	2.98	3.29	3.63	3.88	4.88	5.54	6.59	7.35
200	0.72	1.10	1.36	1.84	2.27	2.50	2.52	2.59	2.61	3.07	3.29	3.62	3.98	4.21	5.26	5.94	7.03	7.81
500	0.85	1.30	1.61	2.17	2.69	2.97	2.99	3.04	3.08	3.49	3.70	4.08	4.44	4.66	5.75	6.43	7.56	8.35
1000	0.96	1.46	1.81	2.44	3.02	3.35	3.38	3.43	3.47	3.81	4.02	4.43	4.79	4.99	6.11	6.78	7.91	8.70

* These precipitation frequency estimates are based on a partial duration series. ARI is the Average Recurrence Interval. Please refer to NOAA Atlas 14 Document for more information. NOTE: Formatting forces estimates near zero to appear as zero.

* Upper bound of the 90% confidence interval Precipitation Frequency Estimates (inches)																		
ARI** (years)	5 min	10 min	15 min	30 min	60 min	120 min	3 hr	6 hr	12 hr	24 hr	48 hr	4 day	7 day	10 day	20 day	30 day	45 day	60 day
1	0.20	0.31	0.38	0.51	0.64	0.73	0.77	0.86	0.97	1.09	1.22	1.35	1.54	1.70	2.19	2.56	3.08	3.53
2	0.26	0.40	0.49	0.66	0.82	0.93	0.98	1.08	1.22	1.36	1.52	1.67	1.91	2.11	2.73	3.18	3.82	4.37
5	0.35	0.54	0.67	0.90	1.11	1.23	1.27	1.36	1.51	1.70	1.88	2.07	2.35	2.58	3.32	3.85	4.62	5.25
10	0.43	0.65	0.81	1.09	1.34	1.48	1.51	1.60	1.74	1.98	2.18	2.39	2.70	2.94	3.77	4.35	5.21	5.89
25	0.54	0.82	1.01	1.36	1.69	1.85	1.87	1.94	2.07	2.37	2.58	2.83	3.18	3.43	4.36	4.99	5.95	6.70
50	0.62	0.95	1.18	1.59	1.96	2.15	2.17	2.23	2.32	2.67	2.90	3.17	3.54	3.79	4.80	5.46	6.49	7.27
100	0.73	1.10	1.37	1.84	2.28	2.49	2.50	2.55	2.60	2.99	3.22	3.53	3.91	4.16	5.22	5.91	7.00	7.80
200	0.83	1.27	1.57	2.12	2.62	2.87	2.88	2.91	2.91	3.32	3.56	3.90	4.29	4.53	5.64	6.34	7.46	8.30
500	0.99	1.51	1.87	2.52	3.12	3.42	3.43	3.44	3.48	3.78	4.02	4.41	4.80	5.01	6.18	6.88	8.04	8.89
1000	1.12	1.71	2.12	2.85	3.52	3.88	3.90	3.91	3.95	4.15	4.38	4.81	5.20	5.39	6.58	7.26	8.43	9.27

* The upper bound of the confidence interval at 90% confidence level is the value which 5% of the simulated quantile values for a given frequency are greater than.

** These precipitation frequency estimates are based on a partial duration series. ARI is the Average Recurrence Interval.

Please refer to NOAA Atlas 14 Document for more information. NOTE: Formatting prevents estimates near zero to appear as zero.

* Lower bound of the 90% confidence interval Precipitation Frequency Estimates (inches)																		
ARI** (years)	5 min	10 min	15 min	30 min	60 min	120 min	3 hr	6 hr	12 hr	24 hr	48 hr	4 day	7 day	10 day	20 day	30 day	45 day	60 day
1	0.15	0.23	0.29	0.39	0.48	0.56	0.60	0.69	0.80	0.93	1.04	1.16	1.33	1.47	1.90	2.24	2.69	3.09
2	0.20	0.30	0.37	0.50	0.62	0.71	0.76	0.86	0.99	1.16	1.29	1.44	1.65	1.82	2.36	2.78	3.34	3.83

5	0.27	0.41	0.51	0.68	0.84	0.94	0.98	1.09	1.23	1.45	1.61	1.79	2.03	2.24	2.88	3.38	4.05	4.62
10	0.32	0.49	0.61	0.82	1.01	1.13	1.17	1.27	1.42	1.69	1.86	2.07	2.33	2.56	3.28	3.82	4.58	5.19
25	0.40	0.61	0.75	1.01	1.25	1.39	1.43	1.54	1.67	2.01	2.20	2.44	2.74	2.97	3.79	4.38	5.24	5.91
50	0.46	0.70	0.87	1.17	1.45	1.60	1.64	1.75	1.86	2.26	2.46	2.73	3.04	3.28	4.16	4.78	5.71	6.41
100	0.53	0.80	0.99	1.34	1.65	1.83	1.86	1.97	2.05	2.51	2.72	3.02	3.35	3.58	4.51	5.16	6.15	6.87
200	0.59	0.90	1.12	1.51	1.87	2.06	2.09	2.19	2.26	2.77	2.98	3.31	3.65	3.88	4.86	5.52	6.56	7.29
500	0.69	1.04	1.29	1.74	2.15	2.39	2.42	2.52	2.56	3.12	3.33	3.68	4.05	4.26	5.27	5.94	7.04	7.78
1000	0.76	1.15	1.43	1.93	2.38	2.64	2.69	2.78	2.81	3.38	3.59	3.97	4.34	4.54	5.58	6.25	7.37	8.10

22

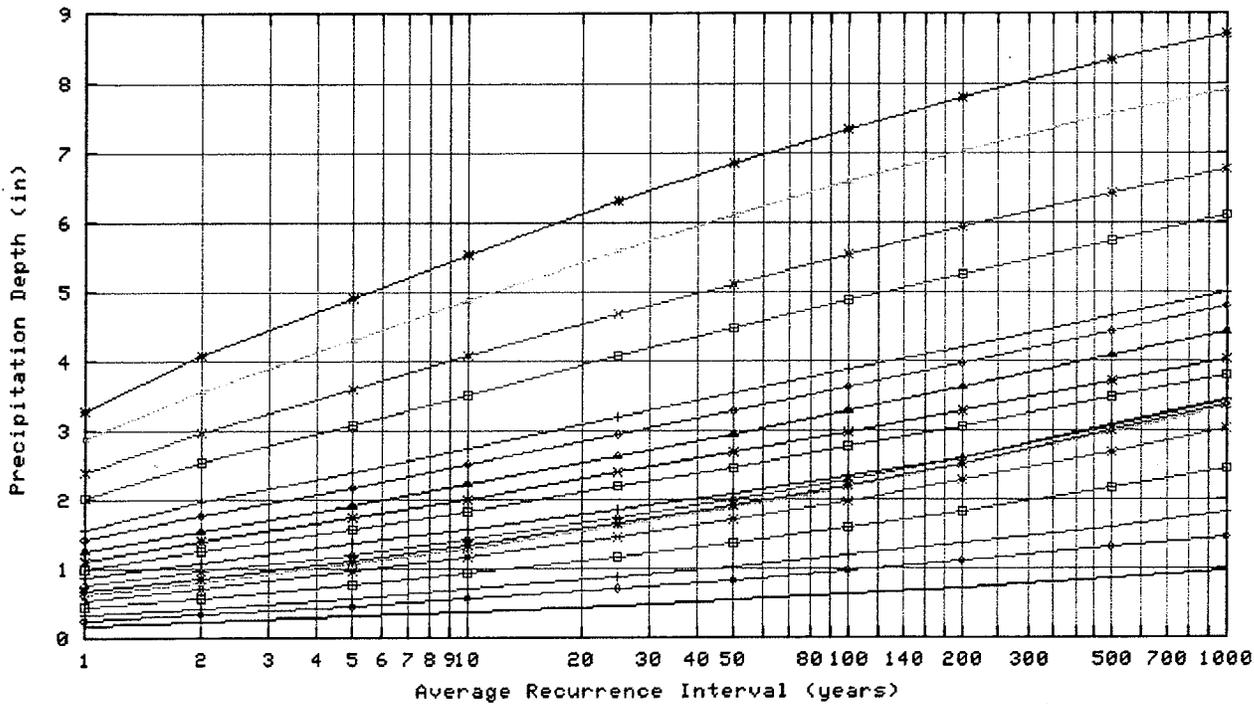
* The lower bound of the confidence interval at 90% confidence level is the value which 5% of the simulated quantile values for a given frequency are less than.

** These precipitation frequency estimates are based on a partial duration maxima series. ARI is the Average Recurrence Interval.

Please refer to NOAA Atlas 14 Document for more information. NOTE: Formatting prevents estimates near zero to appear as zero.

Text version of tables

Partial duration based Point Precipitation Frequency Estimates - Version: 4
34.9667 N 110.289 W 5200 ft

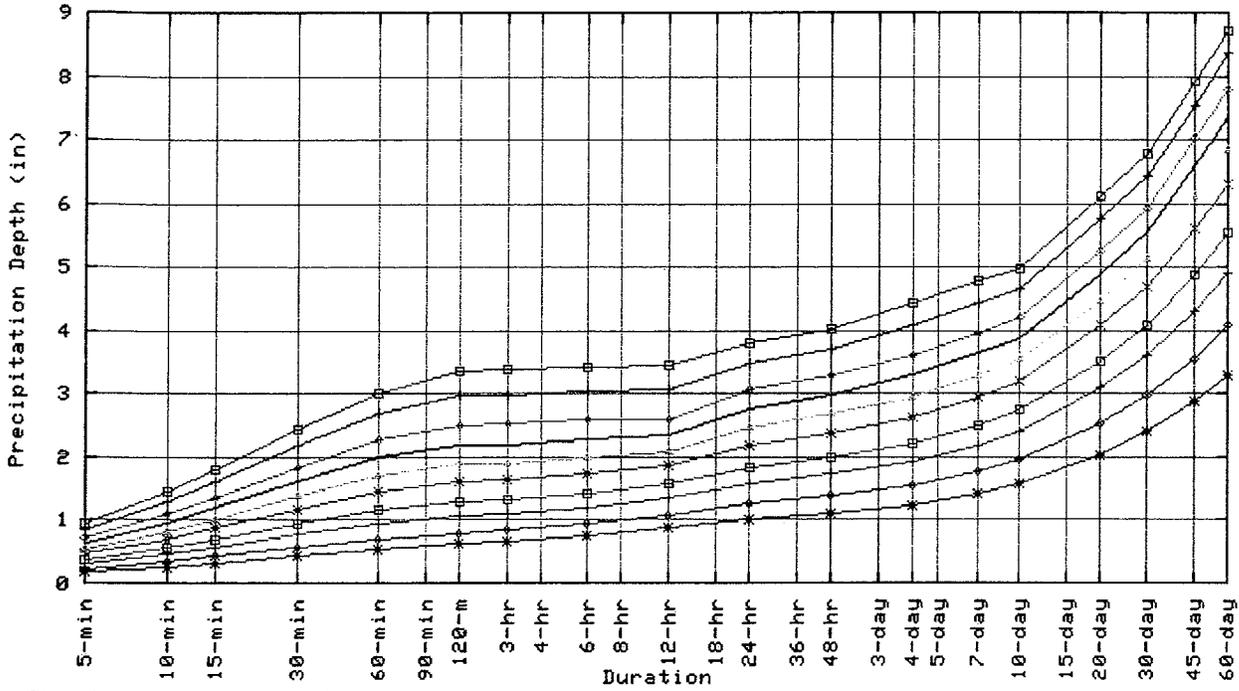


Thu Jan 22 19:00:32 2009

Duration			
5-min	—	3-hr	*
10-min	+	6-hr	o
15-min	+	12-hr	+
30-min	—	24-hr	—
60-min	—	48-hr	—
		4-day	—
		7-day	—
		10-day	—
		20-day	—
		30-day	—
		60-day	—

Partial duration based Point Precipitation Frequency Estimates - Version: 4
 34.9667 N 110.289 W 5200 ft

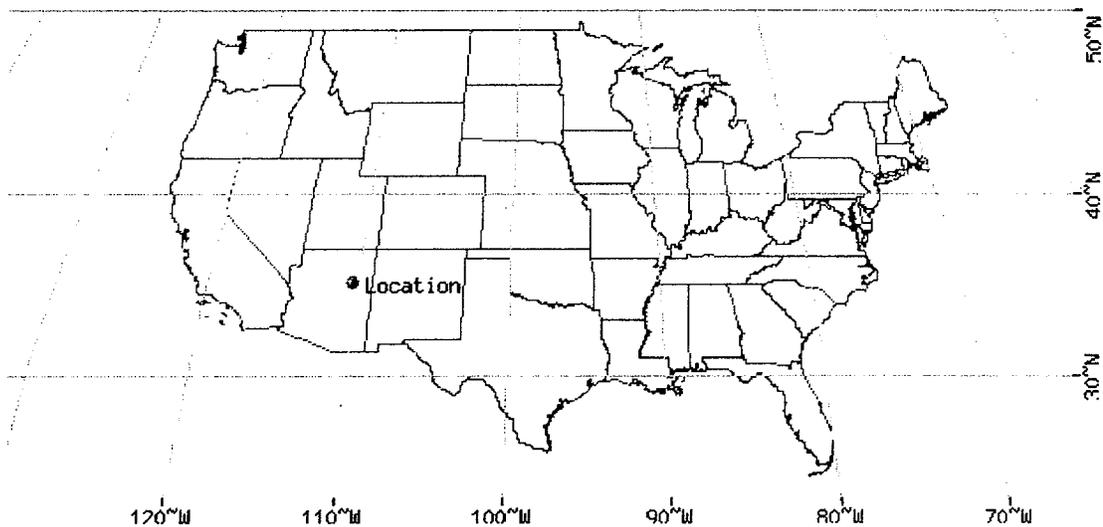
23



Thu Jan 22 19:00:32 2009

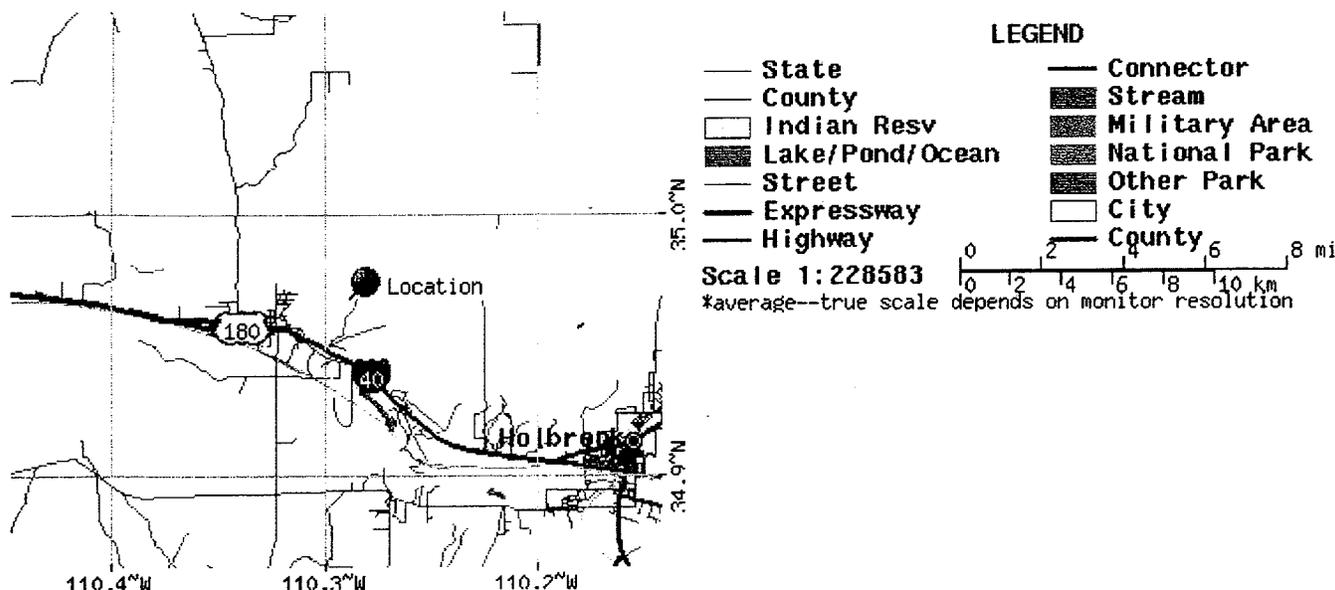
Average Recurrence Interval (years)	
1	*
2	+
5	+
10	+
25	*
100	—
200	+
500	+
1000	+

Maps -



These maps were produced using a direct map request from the U.S. Census Bureau Mapping and Cartographic Resources Tiger Map Server.

Please read disclaimer for more information.



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Other Maps/Photographs -

View [USGS digital orthophoto quadrangle \(DOQ\)](#) covering this location from TerraServer; [USGS Aerial Photograph](#) may also be available from this site. A DOQ is a computer-generated image of an aerial photograph in which image displacement caused by terrain relief and camera tilts has been removed. It combines the image characteristics of a photograph with the geometric qualities of a map. Visit the [USGS](#) for more information.

Watershed/Stream Flow Information -

Find the [Watershed](#) for this location using the U.S. Environmental Protection Agency's site.

Climate Data Sources -

Precipitation frequency results are based on data from a variety of sources, but largely NCDC. The following links provide general information about observing sites in the area, regardless of if their data was used in this study. For detailed information about the stations used in this study, please refer to [NOAA Atlas 14 Document](#).

Using the [National Climatic Data Center's \(NCDC\)](#) station search engine, locate other climate stations within:

...OR... of this location (34.9667/-110.2890). Digital ASCII data can be obtained directly from [NCDC](#).

Find [Natural Resources Conservation Service \(NRCS\) SNOTEL \(SNOWpack TELEmetry\)](#) stations by visiting the [Western Regional Climate Center's state-specific SNOTEL station maps](#).

Hydrometeorological Design Studies Center
 DOC/NOAA/National Weather Service
 1325 East-West Highway
 Silver Spring, MD 20910
 (301) 713-1669
 Questions?: HDSC.Questions@noaa.gov

[Disclaimer](#)

APPENDIX B
HYDROLOGY

ON-SITE HYDROLOGY

CALCULATION COVER SHEET

Client: Arizona Public Service Project Name: Cholla Ash Monofill

Project/Calculation Number: 23445548

Title: Rational Method Calculations/Inputs

Total Number of Pages (including cover sheet): _____

Total Number of Computer Runs: _____

Prepared by: Michelle C. West, EIT *Michelle C. West* Date: 2/3/2009

Checked by: Danette Lucas, EIT *Danette Lucas* Date: 2/9/2009

Description and Purpose:

The purpose of this calculation is to estimate the peak flows required for the offsite and onsite drainage channel design, and to calculate the runoff volume for the onsite drainage detention basin.

The Rational Method and inputs were calculated in an excel spreadsheet. This package contains the input calculations and reference material used to determine C values, K_b values and T_c values. The Rainfall data included in this package is provided in the Rainfall Data calculation package.

The Peak Flow printouts were generated from an excel spreadsheet located:

P:\WRES\Arizona_Public_Service\23445548_Cholla_Ash Monofill APP\Channel Design\Hydrology\Calculations\Rational Method\ADOT-IDF-Cholla-Ash-Monofill_2-9-09.xls

Design Basis/References/Assumptions

The drainage basins and flow paths that the rational method calculation are based on are located in the following CADD files:

P:\WRES\Arizona_Public_Service\23445548_Cholla_Ash Monofill APP\Channel Design\Hydrology\Topo_Base_mcw_working.dwg

P:\WRES\Arizona_Public_Service\23445548_Cholla_Ash Monofill APP\Channel Design\Hydrology\X-5548-SitePlan_onsite_mcw_working.dwg

Figures are included which show the proposed development area, the offsite and onsite drainage basin delineations, and the longest flowpaths for each basin.

Remarks/Conclusions/Results:

See attached printouts.

Calculation Approved by: _____

Project Manager/Date

Revision No.: _____ Description of Revision: _____ Approved by: _____

Project Manager/Date

RATIONAL METHOD CALCULATION
CHOLLA ASH MONOFILL HYDROLOGY ANALYSIS
CHOLLA GENERATING STATION
ARIZONA PUBLIC SERVICE

Problem Statement

The object of this calculation is to calculate the peak flows required for the onsite and offsite drainage channel design of the proposed Cholla Ash Monofill, and to calculate the volume required for the detention basin for the onsite drainage.

The peak flows for the drainage basins were calculated using the procedure outlined in the Arizona Department of Transportation (ADOT) Highway Drainage Design Manual Hydrology.

Required Deliverables

- Times of Concentration and Peak flows for each drainage basin, both offsite pre-development (100-year, 24-hour storm event) and onsite post-development (25-year, 24-hour storm event).
- Volume of runoff for onsite post-development detention basin.

Data Available

- Rainfall Data provided in Rainfall Data Calculation package
- I-D-F Data and Curves provided in Rainfall Data Calculation package
- Drainage Area (Total pre-development drainage area is approximately 150 acres, total post-development offsite drainage area is approximately 98 acres, see Fig 1)
- USDA NRCS web soil survey report for hydrologic soil group ratings by soil map unit to provide basis for C value.
- Tables from ADOT Highway Drainage Design Manual Hydrology providing C values and K_b values for the calculation
- Contour Data provided by USGS (5-foot contours) and Arizona Public Service (2-foot contours)
- Proposed Ash Monofill contour data.

Results

The printouts of the offsite and onsite peak flows were generated from the following Excel spreadsheet are attached:

P:\WRES\Arizona_Public_Service\23445548_Cholla_Ash Monofill APP\Channel Design\Hydrology\Calculations\Rational Method\ADOT-IDF-Cholla-Ash-Monofill_2-9-09.xls

P:\WRES\Arizona_Public_Service\23445548_Cholla_Ash Monofill APP\Channel Design\Hydrology\Calculations\Rational Method\ADOT Rational Method Calc-2-9-09_draft.doc

The total runoff volume for the onsite post-development drainage basins is 5.5 acre feet, including the proposed area of the detention basin itself.

REFERENCES

ADOT Highway Drainage Design Manual Hydrology. March 1993.

URS Corporation. Rainfall Data Calculation Package. Cholla Ash Monofill. 2009.

USDA NRCS. Web Soil Survey. Soil Properties and Qualities, Hydrologic Soil Group. Accessed February 4, 2009. <http://websoilsurvey.nrcs.usda.gov/app/WebSoilSurvey.aspx>

WORKSHEETS

**Cholla Ash Monofill
Cholla Generating Station
Arizona Public Service**

7

	A	C	D	E	F	G	H	I										
1	<table border="1" style="margin: auto;"> <thead> <tr> <th style="width: 50%;">Return Frequency</th> <th style="width: 50%;">Rainfall Depth (inches)</th> </tr> </thead> <tbody> <tr><td>2-year, 6-hour</td><td>0.96</td></tr> <tr><td>2-year, 24-hour</td><td>1.25</td></tr> <tr><td>100-year, 6-hour</td><td>2.28</td></tr> <tr><td>100-year, 24-hour</td><td>2.77</td></tr> </tbody> </table>								Return Frequency	Rainfall Depth (inches)	2-year, 6-hour	0.96	2-year, 24-hour	1.25	100-year, 6-hour	2.28	100-year, 24-hour	2.77
Return Frequency									Rainfall Depth (inches)									
2-year, 6-hour									0.96									
2-year, 24-hour									1.25									
100-year, 6-hour									2.28									
100-year, 24-hour									2.77									
2																		
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7																		
8																		
9																		
10	Short-Duration Rainfall Zone = 6																	
11																		
12	ADOT RAINFALL INTENSITY-DURATION-FREQUENCY																	
13	SITE SPECIFIC I-D-F TABLE																	
14																		
15	Rainfall Intensity (inches/hour)																	
16	Duration	Frequency (N-year)																
17		2	5	10	25	50	100	500										
18	5-min	2.87	3.97	4.72	5.75	6.55	7.34	9.17										
19	10-min	2.21	3.09	3.67	4.48	5.11	5.73	7.17										
20	15-min	1.78	2.52	3.01	3.69	4.22	4.74	5.95										
21	30-min	1.13	1.63	1.95	2.40	2.75	3.10	3.89										
22	1-hour	0.68	0.99	1.20	1.48	1.70	1.91	2.41										
23	2-hour	0.39	0.55	0.65	0.80	0.91	1.02	1.28										
24	3-hour	0.28	0.39	0.46	0.56	0.63	0.71	0.88										
25	6-hour	0.16	0.21	0.25	0.30	0.34	0.38	0.47										
26	12-hour	0.09	0.12	0.14	0.17	0.19	0.21	0.26										
27	24-hour	0.05	0.07	0.08	0.09	0.10	0.12	0.14										
28																		
29	ADOT RAINFALL DEPTH-DURATION-FREQUENCY																	
30	SITE SPECIFIC D-D-F TABLE																	
31																		
32	Rainfall Depth (inches)																	
33	Duration	Frequency (N-year)																
34		2	5	10	25	50	100	500										
35	5-min	0.24	0.33	0.39	0.48	0.55	0.61	0.76										
36	10-min	0.37	0.51	0.61	0.75	0.85	0.96	1.20										
37	15-min	0.44	0.63	0.75	0.92	1.05	1.18	1.49										
38	30-min	0.57	0.81	0.98	1.20	1.38	1.55	1.95										
39	1-hour	0.68	0.99	1.20	1.48	1.70	1.91	2.41										
40	2-hour	0.78	1.09	1.30	1.59	1.81	2.04	2.55										
41	3-hour	0.84	1.16	1.37	1.67	1.89	2.12	2.65										
42	6-hour	0.96	1.28	1.50	1.81	2.04	2.28	2.83										
43	12-hour	1.11	1.45	1.68	2.01	2.27	2.53	3.11										
44	24-hour	1.25	1.61	1.86	2.22	2.50	2.77	3.40										

**Cholla Ash Monofill
Cholla Generating Station
Arizona Public Service**

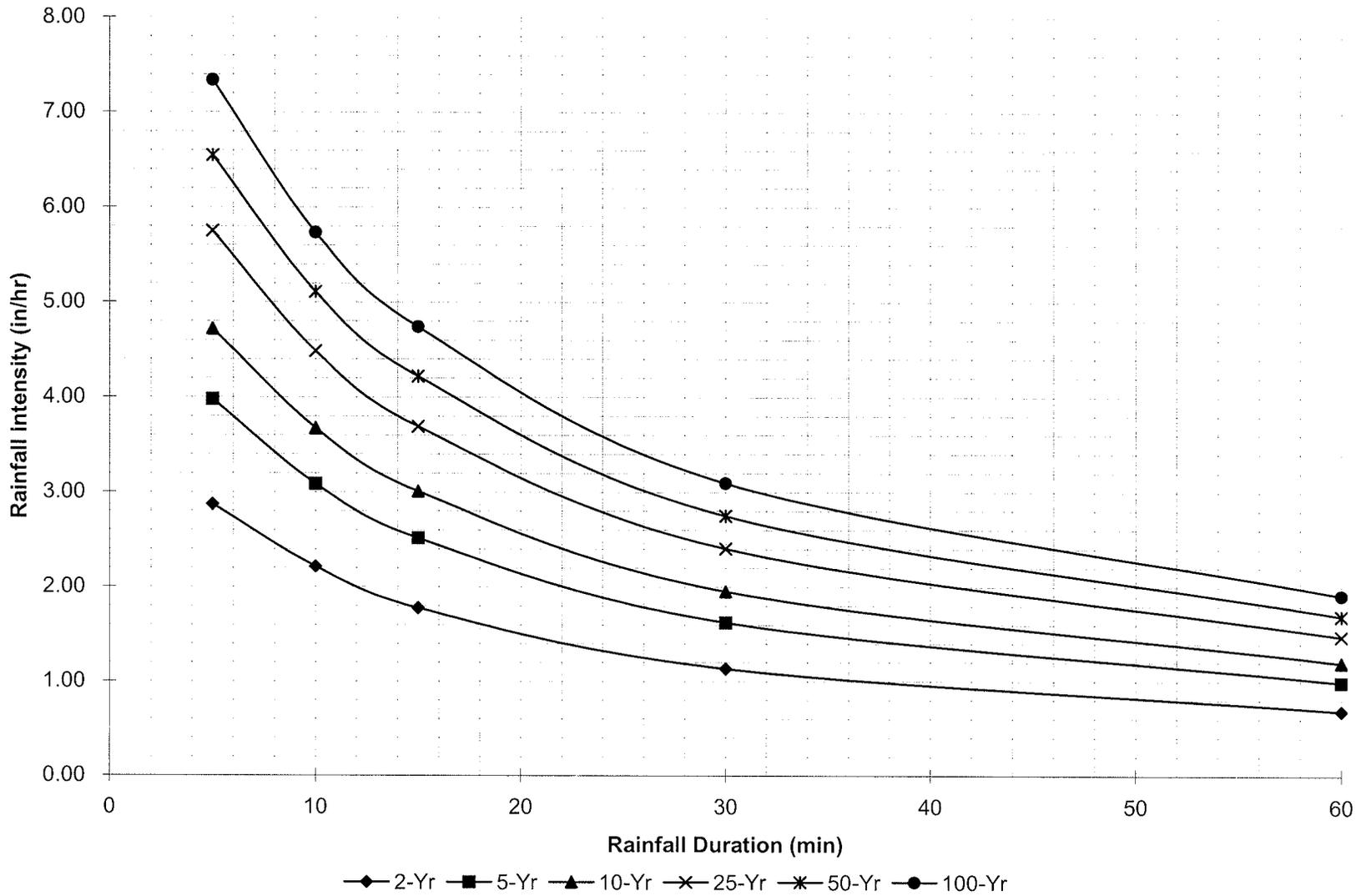
8

	A	C	D	E	F	G	H	I		
45										
46	Procedure Intermediate Calculations:									
47										
48			Rainfall Depth (inches)			Rainfall Depth (inches)				
49	2-year, 1-hour		0.68		2-year, 5-min		0.24			
50	2-year, 2-hour		0.78		2-year, 10-min		0.37			
51	2-year, 3-hour		0.84		2-year, 15-min		0.44			
52	2-year, 6-hour		0.96		2-year, 30-min		0.57			
53	2-year, 12-hour		1.11		100-year, 5-min		0.61			
54	2-year, 24-hour		1.25		100-year, 10-min		0.96			
55	100-year, 1-hour		1.91		100-year, 15-min		1.18			
56	100-year, 2-hour		2.04		100-year, 30-min		1.55			
57	100-year, 3-hour		2.12							
58	100-year, 6-hour		2.28							
59	100-year, 12-hour		2.53							
60	100-year, 24-hour		2.77							
61										
62	Procedure Look-up Tables:									
63										
64		Duration		Zone Ratio			Frequency		Adjustment Ratio	
65		(Minutes)	6	8		(N-yr)	X	Y		
66		5	0.35	0.34		5	0.674	0.278		
67		10	0.54	0.51		10	0.496	0.449		
68		15	0.65	0.62		25	0.293	0.669		
69		30	0.83	0.82		50	0.146	0.835		
70		5	0.32	0.30		500	-0.337	1.381		
71		10	0.50	0.46						
72		15	0.62	0.59						
73		30	0.81	0.80						
74										

	A	C	D	E	F	G	H	I	
1									
2									
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4									
5									
6									
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17	Duration								
18	5-min	=C35/SB18	=D35/SB18						
19	10-min	=C36/SB19	=D36/SB19						
20	15-min	=C37/SB20	=D37/SB20						
21	30-min	=C38/SB21	=D38/SB21						
22	1-hour	=C39/SB22	=D39/SB22						
23	2-hour	=C40/SB23	=D40/SB23						
24	3-hour	=C41/SB24	=D41/SB24						
25	6-hour	=C42/SB25	=D42/SB25						
26	12-hour	=C43/SB26	=D43/SB26						
27	24-hour	=C44/SB27	=D44/SB27						
28									
29									
30									
31									
32									
33	Duration								
34									
35	5-min	=H48	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$H48+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$H52)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$H48+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$H52)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$H48+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$H52)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$H48+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$H52)	=H52	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$H48+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$H52)	
36	10-min	=H49	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$H49+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$H53)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$H49+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$H53)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$H49+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$H53)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$H49+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$H53)	=H53	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$H49+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$H53)	
37	15-min	=H50	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$H50+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$H54)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$H50+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$H54)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$H50+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$H54)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$H50+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$H54)	=H54	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$H50+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$H54)	
38	30-min	=H51	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$H51+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$H55)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$H51+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$H55)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$H51+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$H55)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$H51+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$H55)	=H55	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$H51+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$H55)	
39	1-hour	=D48	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$D48+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$D54)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$D48+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$D54)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$D48+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$D54)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$D48+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$D54)	=D54	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$D48+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$D54)	
40	2-hour	=D49	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$D49+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$D55)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$D49+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$D55)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$D49+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$D55)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$D49+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$D55)	=D55	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$D49+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$D55)	
41	3-hour	=D50	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$D50+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$D56)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$D50+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$D56)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$D50+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$D56)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$D50+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$D56)	=D56	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$D50+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$D56)	
42	6-hour	=D51	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$D51+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$D57)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$D51+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$D57)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$D51+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$D57)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$D51+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$D57)	=D57	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$D51+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$D57)	
43	12-hour	=D52	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$D52+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$D58)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$D52+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$D58)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$D52+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$D58)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$D52+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$D58)	=D58	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$D52+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$D58)	
44	24-hour	=D53	=VLOOKUP(D\$34,\$G\$65:\$I\$69,2)*\$D53+(VLOOKUP(D\$34,\$G\$65:\$I\$69,3)*\$D59)	=VLOOKUP(E\$34,\$G\$65:\$I\$69,2)*\$D53+(VLOOKUP(E\$34,\$G\$65:\$I\$69,3)*\$D59)	=VLOOKUP(F\$34,\$G\$65:\$I\$69,2)*\$D53+(VLOOKUP(F\$34,\$G\$65:\$I\$69,3)*\$D59)	=VLOOKUP(G\$34,\$G\$65:\$I\$69,2)*\$D53+(VLOOKUP(G\$34,\$G\$65:\$I\$69,3)*\$D59)	=D59	=VLOOKUP(I\$34,\$G\$65:\$I\$69,2)*\$D53+(VLOOKUP(I\$34,\$G\$65:\$I\$69,3)*\$D59)	
45									
46	Procedure Intermediate Calculations:								
47									
48	2-year, 1-hour	=0.011+(0.942*(SF5^2))/SF56							
49	2-year, 2-hour	=0.341*(SF5)+0.659*(SD\$48)							
50	2-year, 3-hour	=0.569*(SF5)+0.431*(SD\$48)							
51	2-year, 6-hour	=F5							
52	2-year, 12-hour	=0.5*(SF5)+0.5*(SF56)							
53	2-year, 24-hour	=F6							
54	100-year, 1-hour	=0.494+(0.755*(SF7^2))/SF\$8							
55	100-year, 2-hour	=0.341*(SF7)+0.659*(SD\$54)							
56	100-year, 3-hour	=0.569*(SF7)+0.431*(SD\$54)							
57	100-year, 6-hour	=F7							
58	100-year, 12-hour	=0.5*(SF7)+0.5*(SF\$8)							
59	100-year, 24-hour	=F8							
60									
61	Procedure Look-up Tables:								
62									
63									
64	Duration (Minutes)		Zone Ratio						
65	5	0.35	0.34						
66	10	0.54	0.51						
67	15	0.65	0.62						
68	30	0.83	0.82						
69	5	0.32	0.3						
70	10	0.5	0.46						
71	15	0.62	0.59						
72	30	0.81	0.8						
73									

Cholla Ash Monofill
Cholla Generating Station
Arizona Public Service

Site Specific IDF Graph



Project Name: Cholla Power Plant- Ash Fill Subject: 25-Year Peak Discharge Onsite Location: Joseph City, AZ	Date: 02/06/09 Computed By: MCW Checked By:																																																			
<div style="background-color: #cccccc; padding: 2px; margin-bottom: 5px;">Ash Fill Channel - On-site Basin 4</div> <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 30%;">25-yr, 24-hr Precipitation =</td> <td style="width: 30%;">2.22 in</td> <td style="width: 40%;"></td> </tr> <tr> <td>25-yr, 10 min Intensity =</td> <td>4.48 in/hr</td> <td></td> </tr> <tr> <td>Hydrologic Zone =</td> <td>6</td> <td></td> </tr> <tr> <td colspan="3"> </td> </tr> <tr> <td>Length of Longest Flowpath =</td> <td>819 feet</td> <td></td> </tr> <tr> <td></td> <td>0.16 miles</td> <td></td> </tr> <tr> <td>Upper Elevation=</td> <td>5230.00 feet</td> <td></td> </tr> <tr> <td>Lower Elevation=</td> <td>5086.00 feet</td> <td></td> </tr> <tr> <td>Slope of Longest Flowpath :</td> <td>928.58 ft/mi</td> <td></td> </tr> <tr> <td>Kb =</td> <td>0.08</td> <td></td> </tr> </table>	25-yr, 24-hr Precipitation =	2.22 in		25-yr, 10 min Intensity =	4.48 in/hr		Hydrologic Zone =	6					Length of Longest Flowpath =	819 feet			0.16 miles		Upper Elevation=	5230.00 feet		Lower Elevation=	5086.00 feet		Slope of Longest Flowpath :	928.58 ft/mi		Kb =	0.08		Q=CIA <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 30%;">C_D=</td> <td style="width: 30%;">0.60</td> <td style="width: 40%;"></td> </tr> <tr> <td>I=</td> <td>4.48 in/hr</td> <td></td> </tr> <tr> <td>Area =</td> <td>8.89 acres</td> <td></td> </tr> <tr> <td colspan="3"> </td> </tr> <tr> <td>Total Area =</td> <td>8.89 acres</td> <td></td> </tr> <tr> <td colspan="3"> </td> </tr> <tr> <td>Design Peak Flow =</td> <td><u>23.9</u> cfs</td> <td></td> </tr> </table>	C _D =	0.60		I=	4.48 in/hr		Area =	8.89 acres					Total Area =	8.89 acres					Design Peak Flow =	<u>23.9</u> cfs	
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<p>Project Name: Cholla Power Plant- Ash Fill</p> <p>Subject: 25-Year Peak Discharge Onsite</p> <p>Location: Joseph City, AZ</p>	<p>Date: 02/06/09</p> <p>Computed By: MCW</p> <p>Checked By:</p>																								
<p>Ash Fill Channel - On-site Basin 5</p> <p>25-yr, 24-hr Precipitation = 2.22 in</p> <p>25-yr, 10 min Intensity = 4.48 in/hr</p> <p>Hydrologic Zone = 6</p> <p>Length of Longest Flowpath = 629 feet 0.12 miles</p> <p>Upper Elevation= 5230.00 feet</p> <p>Lower Elevation= 5103.00 feet</p> <p>Slope of Longest Flowpath : 1065.40 ft/mi</p> <p>Kb = 0.08</p>	<p>Q=CIA</p> <p>C_D = 0.60</p> <p>I = 4.48 in/hr</p> <p>Area = 13.72 acres</p> <p>Total Area = 13.72 acres</p> <p>Design Peak Flow = <u>36.9</u> cfs</p>																								
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<p>Project Name: Cholla Power Plant- Ash Fill</p> <p>Subject: 25-Year Peak Discharge Onsite</p> <p>Location: Joseph City, AZ</p>	<p>Date: 02/06/09</p> <p>Computed By: MCW</p> <p>Checked By:</p>																								
<p>Ash Fill Channel - On-site Basin 6</p> <p>25-yr, 24-hr Precipitation = 2.22 in</p> <p>25-yr, 10 min Intensity = 4.48 in/hr</p> <p>Hydrologic Zone = 6</p> <p>Length of Longest Flowpath = 835 feet 0.16 miles</p> <p>Upper Elevation= 5230.00 feet</p> <p>Lower Elevation= 5077.00 feet</p> <p>Slope of Longest Flowpath : 967.47 ft/mi</p> <p>Kb = 0.08</p>	<p>Q=CIA</p> <p>C_D = 0.60</p> <p>I = 4.48 in/hr</p> <p>Area = 19.87 acres</p> <p>Total Area = 19.87 acres</p> <p>Design Peak Flow = <u>53.5</u> cfs</p>																								
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REFERENCES

TABLE 2-1
RESISTANCE COEFFICIENT (K_b) FOR USE WITH THE
RATIONAL METHOD T_c EQUATION

Description of Landform	K_b	
	Defined Drainage Network	Overland Flow Only
Mountain, with forest and dense ground cover (overland slopes - 50% or greater)	0.15	0.30
Mountain, with rough rock and boulder cover (overland slopes - 50% or greater)	0.12	0.25
Foothills (overland slopes - 10% to 50%)	0.10	0.20
Alluvial fans, Pediments and Rangeland (overland slopes - 10% or less)	0.05	0.10
Irrigated Pasture ^a	—	0.20
Tilled Agricultural Fields ^a	—	0.08
URBAN		
Residential, L is less than 1,000 ft ^b	0.04	—
Residential, L is greater than 1,000 ft ^b	0.025	—
Grass; parks, cemeteries, etc. ^a	—	0.20
Bare ground; playgrounds, etc. ^a	—	0.08
Paved; parking lots, etc. ^a	—	0.02

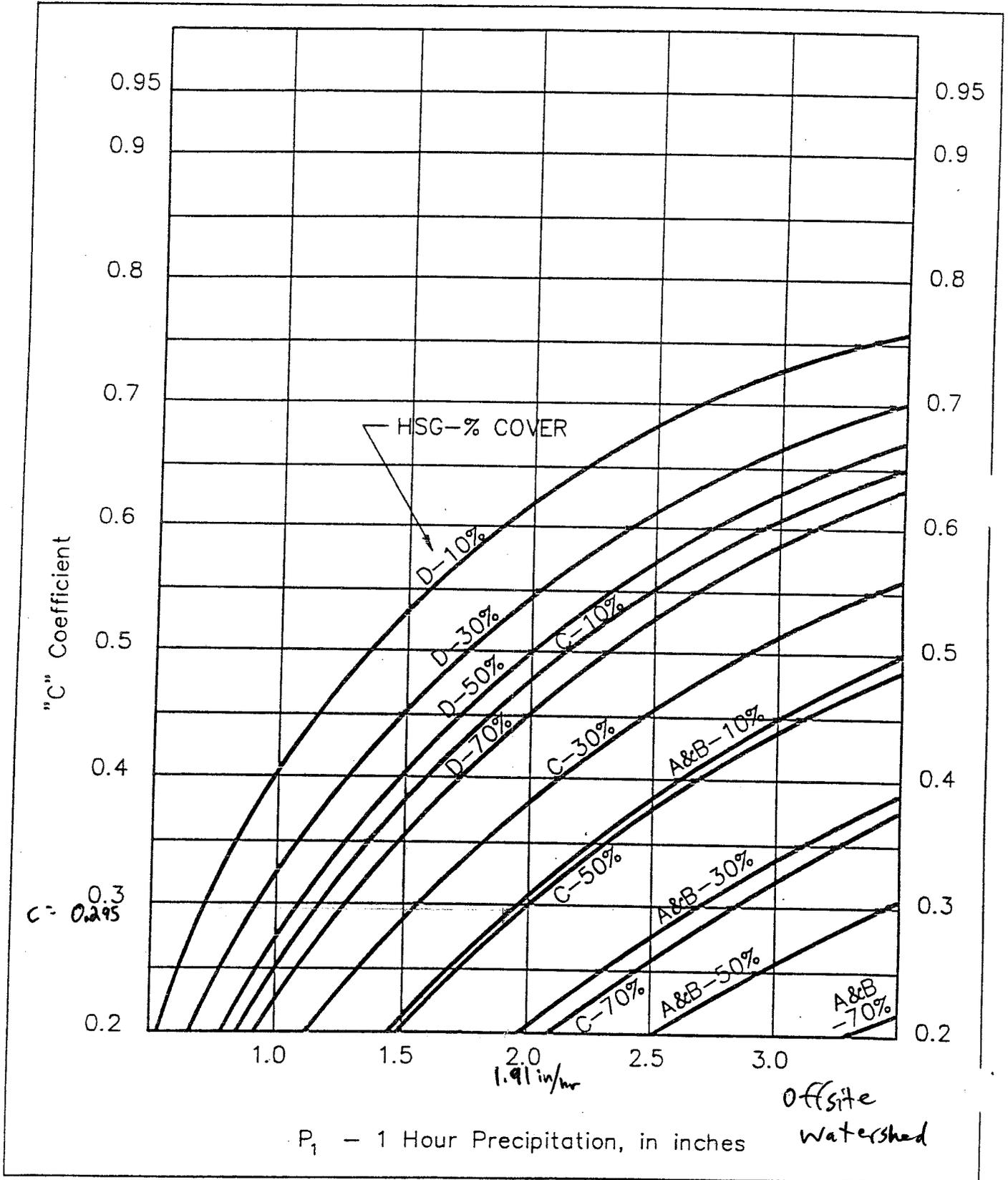
← K_b
offsite

Notes: a - No defined drainage network.
b - L is length in the T_c equation. Streets serve as drainage network.

onsite K_b assumed = 0.08

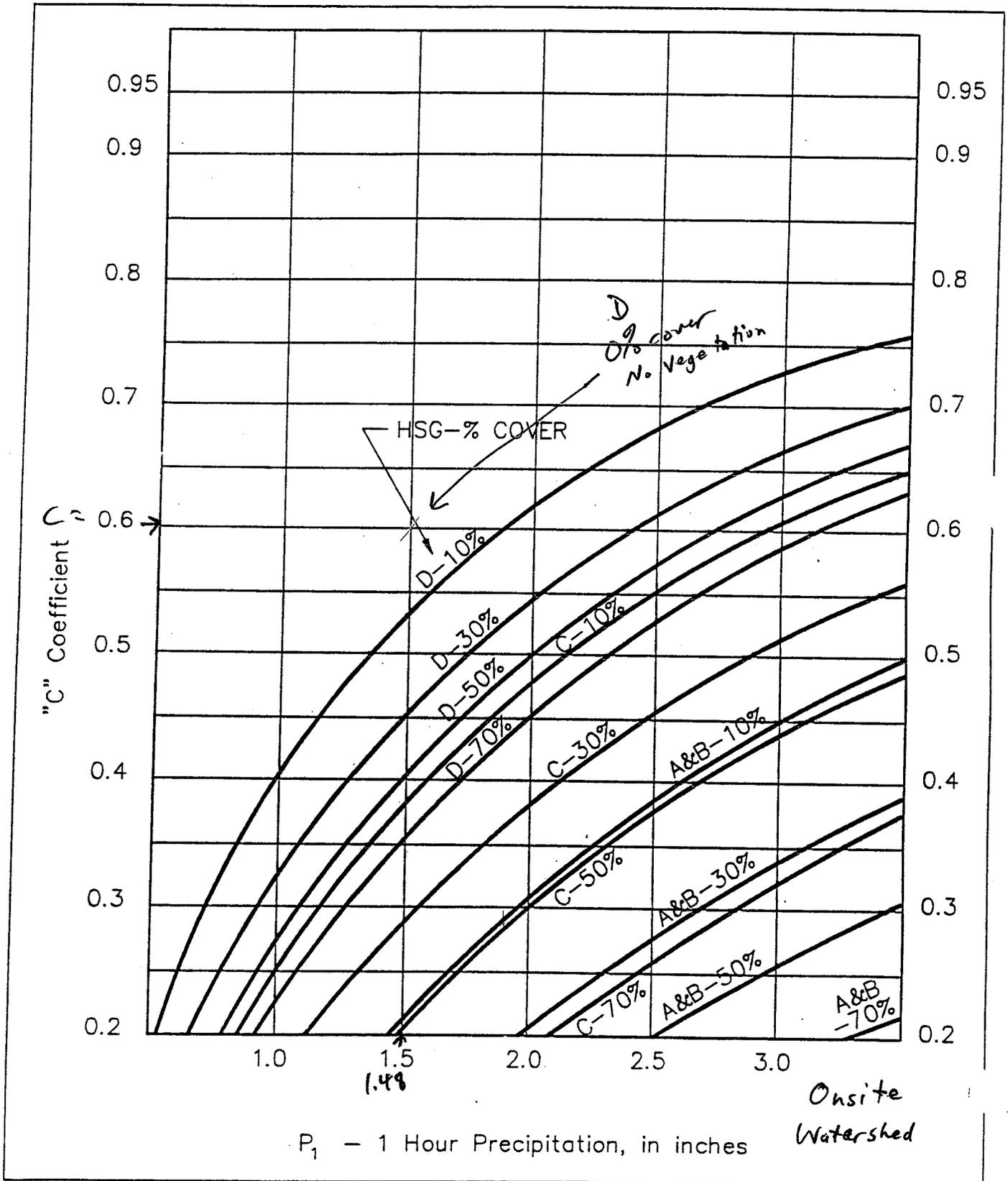
**FIGURE 2-5
RATIONAL "C" COEFFICIENT
UPLAND RANGELAND
(GRASS & BRUSH)**

AS A FUNCTION OF RAINFALL DEPTH, HYDROLOGIC SOIL GROUP (HSG),
AND % OF VEGETATION COVER



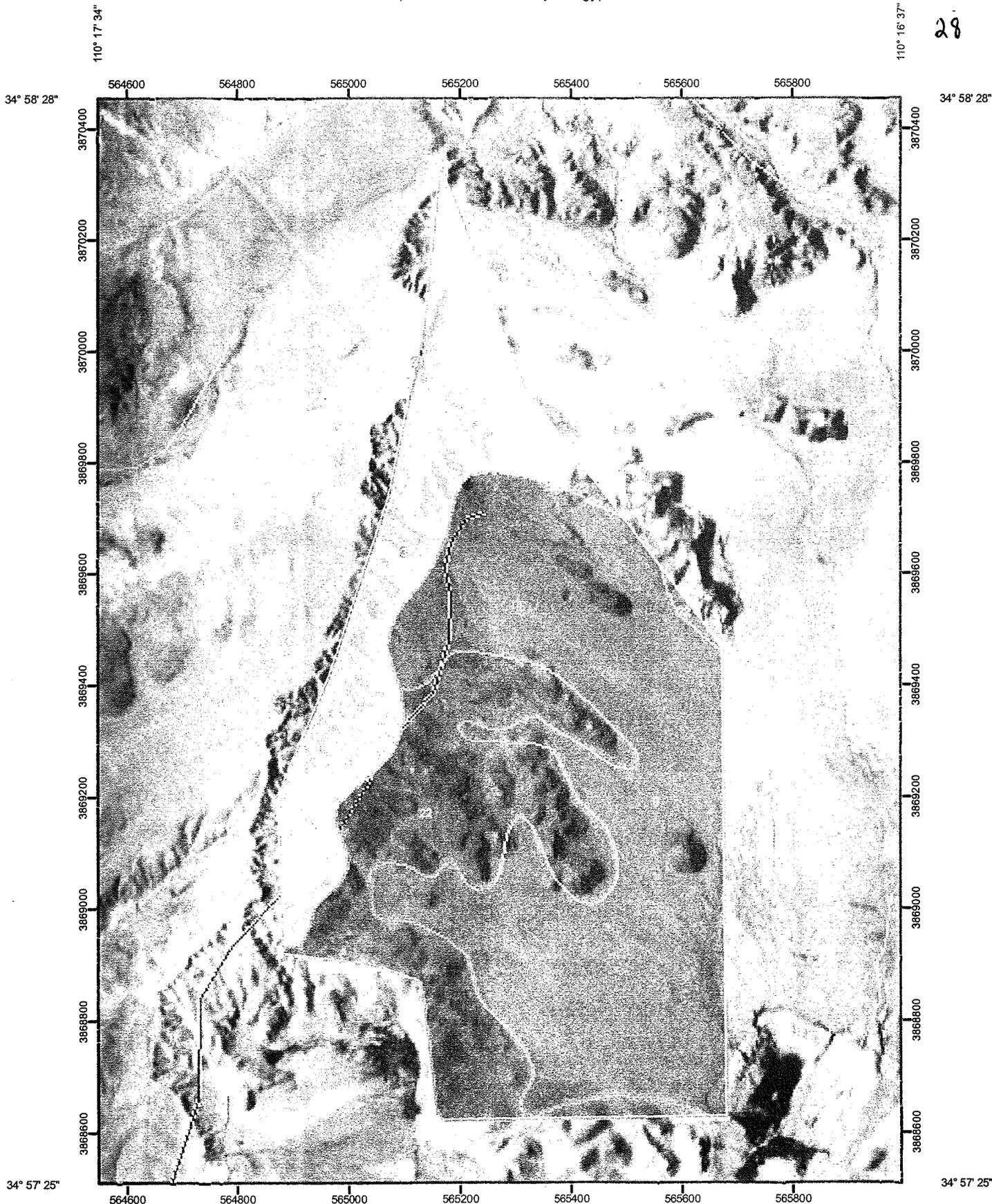
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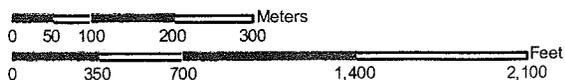


Soil Properties and Qualities—Navajo County Area, Arizona, Central Part
(Cholla Ash Monofill Hydrology)

28



Map Scale: 1:9,280 if printed on A size (8.5" x 11") sheet.



Soil Properties and Qualities—Navajo County Area, Arizona, Central Part
(Cholla Ash Monofill Hydrology)

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Map Units

Soil Ratings

 A

 A/D

 B

 B/D

 C

 C/D

 D

Not rated or not available

Political Features

 Cities

Water Features

 Oceans

Streams and Canals

Transportation

 Rails

 Interstate Highways

 US Routes

Major Roads

 Local Roads

MAP INFORMATION

Map Scale: 1:9,280 if printed on A size (8.5" × 11") sheet.

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for accurate map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL: <http://websoilsurvey.nrcs.usda.gov>
Coordinate System: UTM Zone 12N NAD83

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Navajo County Area, Arizona, Central Part
Survey Area Data: Version 10, Sep 11, 2008

Date(s) aerial images were photographed: 6/21/1997

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Soil Properties and Qualities

Soil Properties and Qualities— Summary by Map Unit — Navajo County Area, Arizona, Central Part				
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
3	Badland-Torriorthents association, 1 to 30 percent slopes		43.8	21.4%
9	Burnswick sandy clay loam, 1 to 5 percent slopes	B	106.0	51.7%
21	Grieta sandy loam, 3 to 10 percent slopes	B	0.2	0.1%
22	Gypsiorthids-Torriorthents association, 5 to 60 percent slopes	B	55.0	26.8%
Totals for Area of Interest			205.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

*Pre-
Development*

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

*Assumed
Post-
Development
Onsite*

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Lower

OFF-SITE HYDROLOGY

Project Name: **Cholla Power Plant- Ash Fill**
 Subject: **100-Year Peak Discharge**
 Location: **Joseph City, AZ**

Date: **02/03/09**
 Computed By: **MCW**
 Checked By:

Ash Fill Channel - Post Development Off-1

100-yr, 24-hr Precipitation = 2.77 in
 100-yr, 10 min Intensity = 5.73 in/hr
 Hydrologic Zone = 6

Length of Longest Flowpath = 1,357 feet
 0.26 miles

Upper Elevation= 5192.00 feet
 Lower Elevation= 5121.00 feet

Slope of Longest Flowpath = 276.26 ft/mi
 Kb = 0.10

Q=CIA

C_{50B} = 0.30

I = 5.73 in/hr
 Area = 14.10 acres

Total Area = 14.10 acres

Design Peak Flow = 23.8 cfs

$T_c = 11.4L^{0.50}K_b^{0.52}S^{-0.31}I^{-0.38}$

Trials

Tc (min)	I (in/hr)	Calculated Tc (min)
5	7.34	8.6
9	6.00	9.3
10	5.73	9.4
15	4.74	10.1
30	3.10	11.9
60	1.91	14.3

OK*

Denotes information that needs to be entered.

*Note: Minimum allowed Tc = 10 minutes

Tc = time of concentration (hrs)
 L = length of longest flowpath (miles)
 Kb = watershed resistance coefficient
 S = slope of longest flowpath
 I = average rainfall intensity (in/hr)

Hydrograph Data
Based on Site Specific Watershed and Rainfall Data
FORMULAS

	A	B	C	D	E	F	G	H	I	J
1	Project Name: Cholla Power Plant- Ash Fill					Date:		39847		
2	Subject: 100-Year Peak Discharge					Computed By:		MCW		
3	Location: Joseph City, AZ					Checked By:				
4										
5	Ash Fill Channel - Pre-development Drainage Area 1					Q=CIA				
6								C _{50B} = 0.295		
7	100-yr, 6-hr Precipitation =					=IDF Data!H42 in		C = 0		
8	100-yr, 10 min Intensity					=IDF Data!H19 in/hr		C _{Composite} = (H6*H10+H7*H11)/H12		
9	Hydrologic Zone =					=IDF Data!F10		I = B26 in/hr		
10								50B Area = 59.65 acres		
11								Area = 0 acres		
12	Length of Longest Flowpath =					4497.4 feet		Total Area = =H10+H11 acres		
13						=D12/5280 miles				
14	Upper Elevation=					5245 feet				
15	Lower Elevation=					5079 feet		Design Peak Flow = =H8*H9*H12 cfs		
16	Slope of Longest Flowpath =					=(D14-D15)/D13 ft/mi				
17						Kb = 0.1				
18										
19	$T_c = 11.4L^{0.50} K_b^{0.52} S^{-0.31} I^{-0.38}$									
20								Tc = time of concentration (hrs)		
21	Trials							L = length of longest flowpath (miles)		
22						Calculated		Kb = watershed resistance coefficient		
23	Tc (min)	I (in/hr)			Tc (min)			S = slope of longest flowpath		
24	5	=IDF Data!H18			=(11.4*\$D\$13^0.5*\$D\$17^0.52*\$D\$16^-0.31*B24^0.38)*60			I = average rainfall intensity (in/hr)		
25	15	=IDF Data!H20			=(11.4*\$D\$13^0.5*\$D\$17^0.52*\$D\$16^-0.31*B25^0.38)*60					
26	22	3.8			=(11.4*\$D\$13^0.5*\$D\$17^0.52*\$D\$16^-0.31*B26^0.38)*60	OK				
27	30	3.1			=(11.4*\$D\$13^0.5*\$D\$17^0.52*\$D\$16^-0.31*B27^0.38)*60					
28	60	1.91			=(11.4*\$D\$13^0.5*\$D\$17^0.52*\$D\$16^-0.31*B28^0.38)*60					
29										
30										
31										
32										

Denotes information that needs to be entered.

APPENDIX C
HYDRAULC CALCULATIONS

APPENDIX C
HYDRAULC CALCULATIONS

ON-SITE HYDRAULIC CALCULATIONS

**ON-SITE CHANNEL
NORMAL DEPTH CALCULATIONS**

Worksheet for DROP Onsite SOUTH channel(25 cfs-5:1)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.20000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	25.00 ft ³ /s

Results

Normal Depth	0.29 ft
Flow Area	3.14 ft ²
Wetted Perimeter	11.58 ft
Top Width	11.46 ft
Critical Depth	0.55 ft
Critical Slope	0.02311 ft/ft
Velocity	7.96 ft/s
Velocity Head	0.98 ft
Specific Energy	1.28 ft
Froude Number	2.68
Flow Type	Supercritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.29 ft
Critical Depth	0.55 ft
Channel Slope	0.20000 ft/ft
Critical Slope	0.02311 ft/ft

Worksheet for Onsite SOUTH channel (25 cfs - 1.6% slope)

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01600 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	25.00 ft ³ /s

Results

Normal Depth	0.61 ft
Flow Area	7.09 ft ²
Wetted Perimeter	13.31 ft
Top Width	13.07 ft
Critical Depth	0.55 ft
Critical Slope	0.02311 ft/ft
Velocity	3.53 ft/s
Velocity Head	0.19 ft
Specific Energy	0.81 ft
Froude Number	0.84
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.61 ft
Critical Depth	0.55 ft
Channel Slope	0.01600 ft/ft
Critical Slope	0.02311 ft/ft

Worksheet for DROP STRUCTURE Onsite sec-1(95 cfs-5:1)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.20000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	95.00 ft ³ /s

Results

Normal Depth	0.64 ft
Flow Area	7.43 ft ²
Wetted Perimeter	13.45 ft
Top Width	13.20 ft
Critical Depth	1.26 ft
Critical Slope	0.01845 ft/ft
Velocity	12.79 ft/s
Velocity Head	2.54 ft
Specific Energy	3.18 ft
Froude Number	3.01
Flow Type	Supercritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.64 ft
Critical Depth	1.26 ft
Channel Slope	0.20000 ft/ft
Critical Slope	0.01845 ft/ft

Worksheet for Onsite channel sec-1(95 cfs - 0.885% slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.00885 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	95.00 ft ³ /s

Results

Normal Depth	1.55 ft
Flow Area	21.43 ft ²
Wetted Perimeter	18.32 ft
Top Width	17.73 ft
Critical Depth	1.26 ft
Critical Slope	0.01845 ft/ft
Velocity	4.43 ft/s
Velocity Head	0.31 ft
Specific Energy	1.85 ft
Froude Number	0.71
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.55 ft
Critical Depth	1.26 ft
Channel Slope	0.00885 ft/ft
Critical Slope	0.01845 ft/ft

Worksheet for Onsite channel sec-2(95 cfs - 1.04% slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01040 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	95.00 ft ³ /s

Results

Normal Depth	1.48 ft
Flow Area	20.25 ft ²
Wetted Perimeter	17.96 ft
Top Width	17.39 ft
Critical Depth	1.26 ft
Critical Slope	0.01846 ft/ft
Velocity	4.69 ft/s
Velocity Head	0.34 ft
Specific Energy	1.82 ft
Froude Number	0.77
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.48 ft
Critical Depth	1.26 ft
Channel Slope	0.01040 ft/ft
Critical Slope	0.01846 ft/ft

Worksheet for Onsite channel sec-3(95 cfs - 0.5% slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.00500 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	95.00 ft ³ /s

Results

Normal Depth	1.80 ft
Flow Area	26.19 ft ²
Wetted Perimeter	19.72 ft
Top Width	19.02 ft
Critical Depth	1.26 ft
Critical Slope	0.01845 ft/ft
Velocity	3.63 ft/s
Velocity Head	0.20 ft
Specific Energy	2.01 ft
Froude Number	0.55
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.80 ft
Critical Depth	1.26 ft
Channel Slope	0.00500 ft/ft
Critical Slope	0.01845 ft/ft

Worksheet for Onsite channel sec-4(95 cfs - 1.0% slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035	
Channel Slope	0.01000	ft/ft
Left Side Slope	2.50	ft/ft (H:V)
Right Side Slope	2.50	ft/ft (H:V)
Bottom Width	10.00	ft
Discharge	95.00	ft ³ /s

Results

Normal Depth	1.49	ft
Flow Area	20.54	ft ²
Wetted Perimeter	18.05	ft
Top Width	17.47	ft
Critical Depth	1.26	ft
Critical Slope	0.01845	ft/ft
Velocity	4.63	ft/s
Velocity Head	0.33	ft
Specific Energy	1.83	ft
Froude Number	0.75	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	1.49	ft
Critical Depth	1.26	ft
Channel Slope	0.01000	ft/ft
Critical Slope	0.01845	ft/ft

Worksheet for Onsite channel sec-5(40 cfs - 1.0% slope)

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	40.00 ft ³ /s

Results

Normal Depth	0.92 ft
Flow Area	11.34 ft ²
Wetted Perimeter	14.96 ft
Top Width	14.61 ft
Critical Depth	0.74 ft
Critical Slope	0.02127 ft/ft
Velocity	3.53 ft/s
Velocity Head	0.19 ft
Specific Energy	1.11 ft
Froude Number	0.71
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.92 ft
Critical Depth	0.74 ft
Channel Slope	0.01000 ft/ft
Critical Slope	0.02127 ft/ft

Worksheet for Onsite channel sec-6(40 cfs - 1.0% slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	40.00 ft ³ /s

Results

Normal Depth	0.92 ft
Flow Area	11.34 ft ²
Wetted Perimeter	14.96 ft
Top Width	14.61 ft
Critical Depth	0.74 ft
Critical Slope	0.02127 ft/ft
Velocity	3.53 ft/s
Velocity Head	0.19 ft
Specific Energy	1.11 ft
Froude Number	0.71
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.92 ft
Critical Depth	0.74 ft
Channel Slope	0.01000 ft/ft
Critical Slope	0.02127 ft/ft

Worksheet for DROP STRUCTURE Onsite Sec-7 (40 cfs-5:1)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.20000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	40.00 ft ³ /s

Results

Normal Depth	0.39 ft
Flow Area	4.24 ft ²
Wetted Perimeter	12.08 ft
Top Width	11.93 ft
Critical Depth	0.74 ft
Critical Slope	0.02127 ft/ft
Velocity	9.44 ft/s
Velocity Head	1.39 ft
Specific Energy	1.77 ft
Froude Number	2.79
Flow Type	Supercritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.39 ft
Critical Depth	0.74 ft
Channel Slope	0.20000 ft/ft
Critical Slope	0.02127 ft/ft

Worksheet for Onsite channel sec-7(40 cfs - 1.0% slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	40.00 ft ³ /s

Results

Normal Depth	0.92 ft
Flow Area	11.34 ft ²
Wetted Perimeter	14.96 ft
Top Width	14.61 ft
Critical Depth	0.74 ft
Critical Slope	0.02127 ft/ft
Velocity	3.53 ft/s
Velocity Head	0.19 ft
Specific Energy	1.11 ft
Froude Number	0.71
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.92 ft
Critical Depth	0.74 ft
Channel Slope	0.01000 ft/ft
Critical Slope	0.02127 ft/ft

Worksheet for Onsite channel sec-8(40 cfs - 0.5% slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.00500 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	40.00 ft ³ /s

Results

Normal Depth	1.12 ft
Flow Area	14.35 ft ²
Wetted Perimeter	16.04 ft
Top Width	15.60 ft
Critical Depth	0.74 ft
Critical Slope	0.02127 ft/ft
Velocity	2.79 ft/s
Velocity Head	0.12 ft
Specific Energy	1.24 ft
Froude Number	0.51
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.12 ft
Critical Depth	0.74 ft
Channel Slope	0.00500 ft/ft
Critical Slope	0.02127 ft/ft

Worksheet for Onsite channel sec-9(40 cfs - 0.5% slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.00500 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	40.00 ft ³ /s

Results

Normal Depth	1.12 ft
Flow Area	14.35 ft ²
Wetted Perimeter	16.04 ft
Top Width	15.60 ft
Critical Depth	0.74 ft
Critical Slope	0.02127 ft/ft
Velocity	2.79 ft/s
Velocity Head	0.12 ft
Specific Energy	1.24 ft
Froude Number	0.51
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.12 ft
Critical Depth	0.74 ft
Channel Slope	0.00500 ft/ft
Critical Slope	0.02127 ft/ft

**ON-SITE CHANNEL
RIP-RAP SIZING CALCULATIONS**

Cholla Ash Monofill
Riprap Comparison
Onsite Channels Drop Structures

Cholla Ash Monofill Drainage Channel Riprap Rock Size (feet)											
Onsite Channel	Method							Min	Max	Chosen Rock Size (D50)	Layer Thickness (ft)
	USACE (D30)	ASCE	USBR	USGS	ISBASH	HEC-11	Maricopa Cty				
Section 1 (bank)	0.09	0.14	0.26	0.38	0.25	0.05	0.17	0.05	0.38	0.33	1.00
Section 1 (bottom)	0.09	0.13	0.26	0.38	0.25	0.04		0.04	0.38	0.33	1.00
Section 2 (bank)	0.11	0.16	0.29	0.43	0.28	0.06	0.17	0.06	0.43	0.33	1.00
Section 2 (bottom)	0.11	0.14	0.29	0.43	0.28	0.05		0.05	0.43	0.33	1.00
Section 3 (bank)	0.05	0.09	0.17	0.23	0.17	0.03	0.08	0.03	0.23	0.33	1.00
Section 3 (bottom)	0.05	0.09	0.17	0.23	0.17	0.02		0.02	0.23	0.33	1.00
Section 4 (bank)	0.10	0.15	0.29	0.42	0.27	0.06	0.17	0.06	0.42	0.33	1.00
Section 4 (bottom)	0.10	0.14	0.29	0.42	0.27	0.04		0.04	0.42	0.33	1.00
Section 5 (bank)	0.06	0.09	0.16	0.22	0.16	0.03	0.08	0.03	0.22	0.33	1.00
Section 5 (bottom)	0.06	0.08	0.16	0.22	0.16	0.02		0.02	0.22	0.33	1.00
Section 6 (bank)	0.06	0.09	0.16	0.22	0.16	0.03	0.08	0.03	0.22	0.33	1.00
Section 6 (bottom)	0.06	0.08	0.16	0.22	0.16	0.02		0.02	0.22	0.33	1.00
Section 7 (bank)	0.06	0.09	0.16	0.22	0.16	0.03	0.08	0.03	0.22	0.33	1.00
Section 7 (bottom)	0.06	0.08	0.16	0.22	0.16	0.02		0.02	0.22	0.33	1.00
Section 8 (bank)	0.03	0.05	0.10	0.12	0.10	0.01	0.08	0.01	0.12	0.33	1.00
Section 8 (bottom)	0.03	0.05	0.10	0.12	0.10	0.01		0.01	0.12	0.33	1.00
Section 9 (bank)	0.03	0.05	0.10	0.12	0.10	0.01	0.08	0.01	0.12	0.33	1.00
Section 9 (bottom)	0.03	0.05	0.10	0.12	0.10	0.01		0.01	0.12	0.33	1.00
Drop Struc On-sec 1-basin (bnk)	1.61	1.16	2.33	5.02	2.09	1.23	3.50	1.16	5.02	1.00*	2.00
Drop Struc On-sec 1-basin (btm)	1.61	1.07	2.33	5.02	2.09	0.92		0.92	5.02	1.00*	2.00
Drop Struc On-sec 7- 5:1(bnk)	0.85	0.63	1.24	2.39	1.14	0.50	1.83	0.50	2.39	1.00*	2.00
Drop Struc On-sec 7- 5:1(btm)	0.85	0.58	1.24	2.39	1.14	0.37		0.37	2.39	1.00*	2.00
SOUTH Channel											
Section 1 (bank)	0.07	0.09	0.16	0.22	0.16	0.03	0.08	0.03	0.22	0.33	1.00
Section 1 (bottom)	0.07	0.08	0.16	0.22	0.16	0.02		0.02	0.22	0.33	1.00
Drop Struc On-SOUTH-basin (bnk)	0.60	0.45	0.88	1.58	0.81	0.34	1.33	0.34	1.58	1.00*	2.00
Drop Struc On-SOUTH-basin (btm)	0.60	0.42	0.88	1.58	0.81	0.26		0.26	1.58	1.00*	2.00

*NOTE: All drop structures and basins will consist of grouted riprap.

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash Onsite Drop Structure SOUTH channel										
5	Design Flood Freque 100 -yr										
6	Location/Station: 32+50 to 33+00										
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ =	Median diameter of the riprap materials, ft									
21	V _a =	Average velocity in the main channel, ft/s									
22	d _{avg} =	Average depth of flow in the main channel, ft									
23	K ₁ =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	φ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	7.96	ft/s								
32	d _{avg} =	0.29	ft								
33	D ₅₀ =	16	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	1.25	ft								
40	d ₅₀ (inch) =	16	inch <----D50 = 16 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 16 inch										

**Cholla Ash Monofill
Riprap Calculation**

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name:	Cholla Ash ONSITE SOUTH Channel									
5	Design Flood Freque	100 -yr									
6	Location/Station:	35+00 to 38+50									
7											
8											
9	<u>Relevant Equations</u>										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d_{50} =	Median diameter of the riprap materials, ft									
21	V_a =	Average velocity in the main channel, ft/s									
22	d_{avg} =	Average depth of flow in the main channel, ft									
23	K_1 =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	ϕ =	Riprap material's angle of repose, degree									
26											
27											
28	<u>Input Parameters</u>										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V_a =	3.53	ft/s								
32	d_{avg} =	0.61	ft								
33	D_{50} =	1	inch Assume a D_{50} and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	ϕ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K_1 =	0.82									
39	d_{50} =	0.08 ft									
40	d_{50} (inch) =	1	inch <---- D_{50} = 1 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d_{50}) = 1 inch										

**Cholla Ash Monofill
Riprap Calculation**

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash Onsite Drop Structure channel section 1										
5	Design Flood Freque 100 -yr										
6	Location/Station: 32+50 to 33+00										
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ =	Median diameter of the riprap materials, ft									
21	V _a =	Average velocity in the main channel, ft/s									
22	d _{avg} =	Average depth of flow in the main channel, ft									
23	K ₁ =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	φ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	12.79	ft/s								
32	d _{avg} =	0.64	ft								
33	D ₅₀ =	42	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	3.49	ft								
40	d ₅₀ (inch) =	42	inch <----D50 = 42 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 42 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name:	Cholla Ash ONSITE Channel section 1									
5	Design Flood Freque	100 -yr									
6	Location/Station:	35+00 to 38+50									
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d_{50} =	Median diameter of the riprap materials, ft									
21	V_a =	Average velocity in the main channel, ft/s									
22	d_{avg} =	Average depth of flow in the main channel, ft									
23	K_1 =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	ϕ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V_a =	4.43	ft/s								
32	d_{avg} =	1.55	ft								
33	D_{50} =	2	inch Assume a D_{50} and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	ϕ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K_1 =	0.82									
39	d_{50} =	0.09 ft									
40	d_{50} (inch) =	2	inch <---- D_{50} = 2 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d_{50}) = 2 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash ONSITE Channel section 2										
5	Design Flood Freque 100 -yr										
6	Location/Station: 35+00 to 38+50										
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ = Median diameter of the riprap materials, ft										
21	V _a = Average velocity in the main channel, ft/s										
22	d _{avg} = Average depth of flow in the main channel, ft										
23	K ₁ = Bank angle correction factor										
24	θ = Bank angle with the horizontal, degree										
25	φ = Riprap material's angle of repose, degree										
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a = <input type="text" value="4.69"/> ft/s										
32	d _{avg} = <input type="text" value="1.48"/> ft										
33	D ₅₀ = <input type="text" value="2"/> inch Assume a D ₅₀ and then calculate if it is stable.										
34	θ = <input type="text" value="21.80"/> degree [2.5:1 (H:V)]										
35	φ = <input type="text" value="41.0"/> degree From Figure 6.14 of the Manual for rounded riprap - attached.										
36											
37	Hence,										
38	K ₁ = <input type="text" value="0.82"/>										
39	d ₅₀ = <input type="text" value="0.11"/> ft										
40	d ₅₀ (inch) = <input type="text" value="2"/> inch <----D50 = 2 inches is stable.										
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = <input type="text" value="2"/> inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K		
1	Calculation of Riprap Size for Channel Lining												
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)												
3													
4	Channel Name: Cholla Ash ONSITE Channel section 3												
5	Design Flood Freque 100 -yr												
6	Location/Station: 35+00 to 38+50												
7													
8													
9	Relevant Equations												
10													
11													
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$												
13													
14													
15													
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$												
17													
18													
19	Where,												
20	d ₅₀ = Median diameter of the riprap materials, ft												
21	V _a = Average velocity in the main channel, ft/s												
22	d _{avg} = Average depth of flow in the main channel, ft												
23	K ₁ = Bank angle correction factor												
24	θ = Bank angle with the horizontal, degree												
25	φ = Riprap material's angle of repose, degree												
26													
27													
28	Input Parameters												
29	(Based on output from FlowMaster and based on the Manual)												
30													
31	V _a = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">3.63</td><td>ft/s</td></tr></table>											3.63	ft/s
3.63	ft/s												
32	d _{avg} = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">1.8</td><td>ft</td></tr></table>											1.8	ft
1.8	ft												
33	D ₅₀ = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">1</td><td>inch</td></tr></table> Assume a D ₅₀ and then calculate if it is stable.											1	inch
1	inch												
34	θ = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">21.80</td><td>degree</td></tr></table> [2.5:1 (H:V)]											21.80	degree
21.80	degree												
35	φ = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">41.0</td><td>degree</td></tr></table> From Figure 6.14 of the Manual for rounded riprap - attached.											41.0	degree
41.0	degree												
36													
37	Hence,												
38	K ₁ = 0.82												
39	d ₅₀ = 0.05 ft												
40	d ₅₀ (inch) = 1 inch <----D50 = 1 inches is stable.												
41													
42													
43	Therefore, proposed design riprap size (d ₅₀) = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">1 inch</td></tr></table>											1 inch	
1 inch													

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name:	Cholla Ash ONSITE Channel section 4									
5	Design Flood Freque	100 -yr									
6	Location/Station:	35+00 to 38+50									
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ =	Median diameter of the riprap materials, ft									
21	V _a =	Average velocity in the main channel, ft/s									
22	d _{avg} =	Average depth of flow in the main channel, ft									
23	K ₁ =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	φ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	4.63	ft/s								
32	d _{avg} =	1.49	ft								
33	D ₅₀ =	2	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	0.11	ft								
40	d ₅₀ (inch) =	2	inch <----D50 = 2 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 2 inch										

**Cholla Ash Monofill
Riprap Calculation**

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash ONSITE Channel section 5										
5	Design Flood Freque 100 -yr										
6	Location/Station: 35+00 to 38+50										
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ =	Median diameter of the riprap materials, ft									
21	V _a =	Average velocity in the main channel, ft/s									
22	d _{avg} =	Average depth of flow in the main channel, ft									
23	K ₁ =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	φ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	3.53	ft/s								
32	d _{avg} =	0.92	ft								
33	D ₅₀ =	1	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	0.06	ft								
40	d ₅₀ (inch) =	1	inch <----D50 = 1 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 1 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash ONSITE Channel section 6										
5	Design Flood Freque 100 -yr										
6	Location/Station: 35+00 to 38+50										
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where.										
20	d ₅₀ =	Median diameter of the riprap materials, ft									
21	V _a =	Average velocity in the main channel, ft/s									
22	d _{avg} =	Average depth of flow in the main channel, ft									
23	K ₁ =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	φ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	3.53	ft/s								
32	d _{avg} =	0.92	ft								
33	D ₅₀ =	1	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	0.06	ft								
40	d ₅₀ (inch) =	1	inch <----D50 = 1 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 1 inch										

**Cholla Ash Monofill
Riprap Calculation**

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name:	Cholla Ash ONSITE Drop Structure section 7									
5	Design Flood Freque	100 -yr									
6	Location/Station:	35+00 to 38+50									
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d_{50} =	Median diameter of the riprap materials, ft									
21	V_a =	Average velocity in the main channel, ft/s									
22	d_{avg} =	Average depth of flow in the main channel, ft									
23	K_1 =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	ϕ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V_a =	9.44	ft/s								
32	d_{avg} =	0.39	ft								
33	D_{50} =	22	inch Assume a D_{50} and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	ϕ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K_1 =	0.82									
39	d_{50} =	1.80	ft								
40	d_{50} (inch) =	22	inch <---- D_{50} = 22 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d_{50}) = 22 inch										

**Cholla Ash Monofill
Riprap Calculation**

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash ONSITE Channel section 7										
5	Design Flood Freque 100 -yr										
6	Location/Station: 35+00 to 38+50										
7											
8											
9	<u>Relevant Equations</u>										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ =	Median diameter of the riprap materials, ft									
21	V _a =	Average velocity in the main channel, ft/s									
22	d _{avg} =	Average depth of flow in the main channel, ft									
23	K ₁ =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	φ =	Riprap material's angle of repose, degree									
26											
27											
28	<u>Input Parameters</u>										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	3.53	ft/s								
32	d _{avg} =	0.92	ft								
33	D ₅₀ =	1	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	0.06	ft								
40	d ₅₀ (inch) =	1	inch <----D50 = 1 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 1 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name:	Cholla Ash ONSITE Channel section 8									
5	Design Flood Freque	100 -yr									
6	Location/Station:	35+00 to 38+50									
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d_{50} =	Median diameter of the riprap materials, ft									
21	V_a =	Average velocity in the main channel, ft/s									
22	d_{avg} =	Average depth of flow in the main channel, ft									
23	K_1 =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	ϕ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V_a =	2.79	ft/s								
32	d_{avg} =	1.12	ft								
33	D_{50} =	1	inch Assume a D_{50} and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	ϕ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K_1 =	0.82									
39	d_{50} =	0.03 ft									
40	d_{50} (inch) =	1 inch		<---- D_{50} = 1 inches is stable.							
41											
42											
43	Therefore, proposed design riprap size (d_{50}) = 1 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name:	Cholla Ash ONSITE Channel section 9									
5	Design Flood Freque	100 -yr									
6	Location/Station:	35+00 to 38+50									
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d_{50} =	Median diameter of the riprap materials, ft									
21	V_a =	Average velocity in the main channel, ft/s									
22	d_{avg} =	Average depth of flow in the main channel, ft									
23	K_1 =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	ϕ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V_a =	2.79	ft/s								
32	d_{avg} =	1.12	ft								
33	D_{50} =	1	inch Assume a D_{50} and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	ϕ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K_1 =	0.82									
39	d_{50} =	0.03	ft								
40	d_{50} (inch) =	1	<----D50 = 1 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d_{50}) = 1 inch										

Cholla Ash Monofill
Riprap Calculation
Formulas

By: _____
Checked: _____

	A	B	C	D	E	F	G	H
1	Calculation of Riprap Size for Channel Lining							
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)							
3								
4	Channel Name:	Cholla Ash Offsite Channel section 1						
5	Design Flood Frequency:	100		-yr				
6	Location/Station:	1+50 to 7+50						
7								
8								
9	<u>Relevant Equations</u>							
10								
11								
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$							
13								
14	$K_1 = \left[\frac{1 - \sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$							
15								
16								
17								
18								
19	Where,							
20	d_{50} =	Median diameter of the riprap materials, ft						
21	V_a =	Average velocity in the main channel, ft/s						
22	d_{avg} =	Average depth of flow in the main channel, ft						
23	K_1 =	Bank angle correction factor						
24	θ =	Bank angle with the horizontal, degree						
25	ϕ =	Riprap material's angle of repose, degree						
26								
27								
28	<u>Input Parameters</u>							
29	(Based on output from FlowMaster and based on the Manual)							
30								
31	V_a =	7.96	ft/s					
32	d_{avg} =	0.29	ft					
33	D_{50} =	16	inch					
34	θ =	21.8	degree					
35	ϕ =	41	degree					
36	Assume a D_{50} and then calculate if it is stable. [2.5:1 (H:V)] From Figure 6.14 of the Manual for rounded riprap - attached.							
37	Hence,							
38	K_1 =	= (1 - ((SIN(RADIANS(C34)))^2 / (SIN(RADIANS(C35)))^2))^0.5						
39	d_{50} =	= 0.001 * C31^3 / (C32^0.5 * C38^1.5) ft						
40	d_{50} (inch) =	= CEILING(C39 * 12, 1) inch <---- D50 = =C33 inches is stable.						
41								
42								
43	Therefore, proposed design riprap size (d_{50}) = =C33 inch							

Onsite SOUTH Channel-DROP

Date: 02/12/2009 Time: 17:05

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
* COPYRIGHT (c) 2005                                     *
* WEST CONSULTANTS, INC.                                 *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX: 858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB: WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite SOUTH Channel DROP

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel velocity	7.96 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	0.29 ft
Cotangent of Side slope	2.50
Safety Factor	1.1
Riprap Placement	Channel Bank
Rock Type	Angular

Output Results:

Computed D30	0.60 ft
Computed Local Depth Averaged velocity	7.96 ft/s
Local Velocity/Avg. velocity	1.00
Side slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	1.250 ft
Selected Minimum D30	0.61 ft
Selected Minimum D90	0.88 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	67.	169.
w50	34.	50.

Onsite SOUTH Channel-DROP
USGS Method

Input Parameters:

Average Channel Velocity 7.96 ft/s

Output Results:

Computed D50 1.58 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 2.750 ft
Selected Minimum D30 1.34 ft
Selected Minimum D90 1.94 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	719.	1797.
W50	359.	532.
W15	112.	266.

Isbash Method

Input Parameters:

Average Channel Velocity 7.96 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.81 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 1.500 ft
Selected Minimum D30 0.73 ft
Selected Minimum D90 1.06 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	117.	292.
W50	58.	86.
W15	18.	43.

Cal B & SP Method

Input Parameters:

Onsite SOUTH Channel-DROP

 Average Channel Velocity 7.96 ft/s
 Velocity Affecting Bank 10.61 ft/s
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

Computed W 40.71 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel velocity 7.96 ft/s
 Average Flow Depth 3.00 ft
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.

Riprap Placement
Safety Factor

Onsite SOUTH Channel-DROP
Channel Bank
1.1

Output Results:

Computed D50 0.34 ft

** FHWA Gradation**

Gradation Class
Layer Thickness

Facing
1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Date: 02/12/2009 Time: 17:05

```

*****
*                               RIPRAP DESIGN SYSTEM (RDS)                               *
*                               BY                                           *
*                               WEST Consultants, Inc.                         *
*                               *                                               *
*                               *                                               *
* Version 3.0                               March, 2005 *
*                               *                                               *
*                               *                                               *
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* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                               WEB:WWW.WESTCONSULTANTS.COM *
*****

```

Project: Cholla Ash Onsite
Description: Onsite SOUTH Channel DROP_BTM

----- USACE Method -----

Input Parameters:

```

-----
Velocity Type                               Average
Channel Shape                               Trapezoidal
Channel Type                               Straight
Bend Angle (deg)                            N/A
Average Channel Velocity                     7.96 ft/s
Bottom width                                 N/A
Bend Radius                                  N/A
Top Width                                    N/A
Unit Weight of Stone                         165. lbs/cu ft
Riprap Layer Thickness                       1.00
Local Flow Depth                             0.29 ft
Cotangent of Side Slope                      N/A
Safety Factor                                1.1
Riprap Placement                             Channel Bottom
Rock Type                                     Angular

```

Output Results:

```

-----
Computed D30                                0.60 ft
Computed Local Depth Averaged Velocity      7.96 ft/s
Local Velocity/Avg. Velocity                1.00
Side Slope Correction Factor                1.06
Correction for Layer Thickness              1.00
Correction for Secondary Currents           1.00

```

*** Using Gradations from COE ETL 1110-2-120 ***

```

Specific Weight      165.0 lbs/cu ft
Layer Thickness      1.250 ft
Selected Minimum D30 0.61 ft
Selected Minimum D90 0.88 ft

```

Percent Lighter by Weight	Stone Weight, lbs	
	Minimum	Maximum
W100	67.	169.
W50	34.	50.
W15	11.	25.

ASCE Method

Input Parameters:

Local Velocity	7.96 ft/s
Cotangent of Side slope	N/A
Unit Weight of Stone	165. lbs/cu ft
Riprap Placement	Channel Bottom

Output Results:

Computed D50 0.42 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by Weight	Stone Weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

USBR Method

Input Parameters:

Average Channel Velocity	7.96 ft/s
--------------------------	-----------

Output Results:

Computed D50 0.88 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight	165.0 lbs/cu ft
Layer Thickness	1.500 ft

Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

HEC-11 Method

Input Parameters:

Average Channel Velocity 7.96 ft/s
Average Flow Depth 3.00 ft
Unit Weight of Stone 165. lbs/cu ft
Cotangent of Side Slope N/A
Material Angle of Repose deg.
Riprap Placement Channel Bottom
Safety Factor 1.1

Output Results:

Computed D50 0.26 ft

** FHWA Gradation**

Gradation Class Facing
Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock Weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Onsite SOUTH Channel

Date: 02/12/2009 Time: 17:03

```
*****
*                               *
*          RIPRAP DESIGN SYSTEM (RDS)          *
*                               *
*                               *
*                               *
*                               *
*                               *
*          WEST Consultants, Inc.              *
*                               *
*                               *
* Version 3.0                               March, 2005 *
*                               *
*                               *
*          COPYRIGHT (c) 2005                 *
*          WEST CONSULTANTS, INC.            *
*          16870 WEST BERNARDO DRIVE         PH: 858-487-9378 *
*          SUITE 340                         FAX: 858-487-9448 *
*          SAN DIEGO, CA 92127              WEB: WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite SOUTH Channel

USACE Method

Input Parameters:

```
-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    3.53 ft/s
Bottom width                 N/A
Bend Radius                  N/A
Top width                    N/A
Unit weight of Stone        165.7 lbs/cu ft
Riprap Layer Thickness      1.00
Local Flow Depth            0.61 ft
Cotangent of Side Slope    2.50
Safety Factor                1.1
Riprap Placement            Channel Bank
Rock Type                    Angular
```

Output Results:

```
-----
Computed D30                  0.07 ft
Computed Local Depth Averaged Velocity 3.53 ft/s
Local Velocity/Avg. Velocity 1.00
Side Slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
```

*** Using Gradations from COE ETL 1110-2-120 ***

```
Specific Weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

Onsite SOUTH Channel
USGS Method

Input Parameters:

Average Channel Velocity 3.53 ft/s

Output Results:

Computed D50 0.22 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 3.53 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.16 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

Onsite SOUTH Channel

Average Channel Velocity 3.53 ft/s
Velocity Affecting Bank 4.71 ft/s
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Flow Type Impinging

Output Results:

Computed w 0.31 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 3.53 ft/s
Average Flow Depth 3.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Material Angle of Repose 41.00 deg.

Onsite SOUTH Channel_btm

Date: 02/12/2009 Time: 17:03

```
*****
*                               *
*           RIPRAP DESIGN SYSTEM (RDS)           *
*                               *
*                               *
*                               *
*                               *
*                               *
* Version 3.0                               March, 2005 *
*                               *
*                               *
* COPYRIGHT (c) 2005                               *
* WEST CONSULTANTS, INC.                               *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                               WEB:WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite SOUTH Channel_btm

_____ USACE Method _____

Input Parameters:

```
-----
Velocity Type                               Average
Channel Shape                               Trapezoidal
Channel Type                               Straight
Bend Angle (deg)                           N/A
Average Channel velocity                     3.53 ft/s
Bottom width                               N/A
Bend Radius                                 N/A
Top Width                                   N/A
Unit weight of Stone                       165. lbs/cu ft
Riprap Layer Thickness                     1.00
Local Flow Depth                           0.61 ft
Cotangent of Side slope                    N/A
Safety Factor                              1.1
Riprap Placement                           Channel Bottom
Rock Type                                   Angular
```

Output Results:

```
-----
Computed D30                               0.07 ft
Computed Local Depth Averaged velocity     3.53 ft/s
Local velocity/Avg. velocity              1.00
Side Slope Correction Factor              1.06
Correction for Layer Thickness            1.00
Correction for Secondary Currents         1.00
```

*** Using Gradations from COE ETL 1110-2-120 ***

```
Specific weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30    0.37 ft
Selected Minimum D90    0.53 ft
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

Onsite SOUTH Channel_btm
USGS Method

Input Parameters:

Average Channel Velocity 3.53 ft/s

Output Results:

Computed D50 0.22 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 3.53 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.16 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

Onsite SOUTH Channel_btm

 Average Channel Velocity 3.53 ft/s
 Velocity Affecting Bank 4.71 ft/s
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

Computed W 0.31 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 3.53 ft/s
 Average Flow Depth 3.00 ft
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope N/A
 Material Angle of Repose deg.

ONSITE Channel DROP Sec-1

Date: 02/12/2009 Time: 15:41

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                        *
*                WEST Consultants, Inc.                    *
*                *                                        *
* Version 3.0                                           March, 2005 *
*                *                                        *
* COPYRIGHT (c) 2005                                    *
* WEST CONSULTANTS, INC.                                *
* 16870 WEST BERNARDO DRIVE                             PH: 858-487-9378 *
* SUITE 340                                             FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                WEB:WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite Channel SEC-1 DROP

USACE Method

Input Parameters:

```
-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    12.79 ft/s
Bottom width                N/A
Bend Radius                 N/A
Top width                   N/A
Unit weight of Stone        165. lbs/cu ft
Riprap Layer Thickness      1.00
Local Flow Depth            0.64 ft
Cotangent of Side Slope    2.50
Safety Factor                1.1
Riprap Placement            Channel Bank
Rock Type                    Angular
```

Output Results:

```
-----
Computed D30                1.61 ft
Computed Local Depth Averaged Velocity 12.79 ft/s
Local Velocity/Avg. Velocity 1.00
Side Slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
```

*** Using Gradations from COE ETL 1110-2-120 ***

```
Specific weight    165.0 lbs/cu ft
Layer Thickness    3.500 ft
Selected Minimum D30 1.70 ft
Selected Minimum D90 2.47 ft
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	1482.	3704.
w50	741.	1096.

ONSITE Channel DROP Sec-1
232.

W15

548.

ASCE Method

Input Parameters:

Local Velocity 12.79 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 1.16 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 2.000 ft
 Selected Minimum D30 0.97 ft
 Selected Minimum D90 1.41 ft

Percent Lighter by weight	stone weight, lbs	
	Minimum	Maximum
W100	276.	691.
W50	138.	205.
W15	43.	102.

USBR Method

Input Parameters:

Average Channel Velocity 12.79 ft/s

Output Results:

Computed D50 2.33 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 4.000 ft
 Selected Minimum D30 1.95 ft
 Selected Minimum D90 2.82 ft

Percent Lighter by weight	stone weight, lbs	
	Minimum	Maximum
W100	2212.	5529.
W50	1106.	1637.
W15	346.	818.

ONSITE Channel DROP Sec-1
USGS Method

Input Parameters:

Average Channel Velocity 12.79 ft/s

Output Results:

Computed D50 5.02 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Warning: The required stone size is greater than the largest USACE stone gradation.

Irbash Method

Input Parameters:

Average Channel Velocity 12.79 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 2.09 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 4.000 ft
Selected Minimum D30 1.95 ft
Selected Minimum D90 2.82 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	2212.	5529.
w50	1106.	1637.
w15	346.	818.

HEC-11 Method

Input Parameters:

Average Channel velocity 12.79 ft/s
Average Flow Depth 4.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side Slope 2.50
Material Angle of Repose 41.00 deg.
Riprap Placement Channel Bank

ONSITE Channel DROP Sec-1

Safety Factor 1.1

Output Results:

Computed D50 1.23 ft

** FHWA Gradation**

Gradation Class Light
Layer Thickness 2.60 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.80	500.
D50	1.30	200.
D10	0.40	5.

ONSITE Channel DROP Sec-1_btm

Date: 02/12/2009 Time: 15:42

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                      *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
* COPYRIGHT (c) 2005                                     *
* WEST CONSULTANTS, INC.                                 *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127   WEB:WWW.WESTCONSULTANTS.COM     *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite Channel SEC-1 DROP bottom

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	12.79 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	0.64 ft
Cotangent of Side slope	N/A
Safety Factor	1.1
Riprap Placement	Channel Bottom
Rock Type	Angular

Output Results:

Computed D30	1.61 ft
Computed Local Depth Averaged Velocity	12.79 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	3.500 ft
Selected Minimum D30	1.70 ft
Selected Minimum D90	2.47 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	1482.	3704.
w50	741.	1096.

ONSITE Channel DROP Sec-1_btm
232.

W15

548.

ASCE Method

Input Parameters:

Local Velocity 12.79 ft/s
Cotangent of Side slope N/A
Unit Weight of Stone 165. lbs/cu ft
Riprap Placement Channel Bottom

Output Results:

Computed D50 1.07 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 2.000 ft
Selected Minimum D30 0.97 ft
Selected Minimum D90 1.41 ft

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
W100	276.	691.
W50	138.	205.
W15	43.	102.

USBR Method

Input Parameters:

Average Channel velocity 12.79 ft/s

Output Results:

Computed D50 2.33 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 4.000 ft
Selected Minimum D30 1.95 ft
Selected Minimum D90 2.82 ft

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
W100	2212.	5529.
W50	1106.	1637.
W15	346.	818.

ONSITE Channel DROP Sec-1_btm
USGS Method

Input Parameters:

Average Channel Velocity 12.79 ft/s

Output Results:

Computed D50 5.02 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Warning: The required stone size is greater than the largest
USACE stone gradation.

Isbash Method

Input Parameters:

Average Channel Velocity 12.79 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 2.09 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 4.000 ft
Selected Minimum D30 1.95 ft
Selected Minimum D90 2.82 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	2212.	5529.
w50	1106.	1637.
w15	346.	818.

HEC-11 Method

Input Parameters:

Average Channel Velocity 12.79 ft/s
Average Flow Depth 4.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side Slope N/A
Material Angle of Repose deg.
Riprap Placement Channel Bottom

ONSITE Channel DROP Sec-1_btm

Safety Factor 1.1

Output Results:

Computed D50 0.92 ft

** FHWA Gradation**

Gradation Class Facing
Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-1
2.

W15

5.

ASCE Method

Input Parameters:

Local velocity 4.43 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.14 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 4.43 ft/s

Output Results:

Computed D50 0.26 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

ONSITE Channel Sec-1
USGS Method

Input Parameters:

Average Channel velocity 4.43 ft/s

Output Results:

Computed D50 0.38 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel velocity 4.43 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.25 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-1

Average Channel velocity	4.43 ft/s
Average Flow Depth	4.00 ft
Unit Weight of Stone	165. lbs/cu ft
Cotangent of Side Slope	2.50
Material Angle of Repose	41.00 deg.
Riprap Placement	Channel Bank
Safety Factor	1.1

Output Results:

Computed D50	0.05 ft
--------------	---------

** FHWA Gradation**

Gradation Class	Facing
Layer Thickness	1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-1_btm

Date: 02/12/2009 Time: 15:44

```

*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
* COPYRIGHT (c) 2005                                     *
* WEST CONSULTANTS, INC.                                 *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB:WWW.WESTCONSULTANTS.COM *
*****
    
```

Project: Cholla Ash Onsite
 Description: Onsite Channel SEC-1 bottom

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	4.43 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	1.55 ft
Cotangent of Side Slope	N/A
Safety Factor	1.1
Riprap Placement	Channel Bottom
Rock Type	Angular

Output Results:

Computed D30	0.09 ft
Computed Local Depth Averaged Velocity	4.43 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

ONSITE Channel Sec-1_btm
 W15 2. 5.

ASCE Method

Input Parameters:

Local Velocity 4.43 ft/s
 Cotangent of Side slope N/A
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bottom

Output Results:

Computed D50 0.13 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 4.43 ft/s

Output Results:

Computed D50 0.26 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

ONSITE Channel Sec-1_btm
USGS Method

Input Parameters:

Average Channel Velocity 4.43 ft/s

Output Results:

Computed D50 0.38 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 4.43 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.25 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-1_btm

Average Channel Velocity	4.43 ft/s
Average Flow Depth	4.00 ft
Unit Weight of Stone	165. lbs/cu ft
Cotangent of Side Slope	N/A
Material Angle of Repose	deg.
Riprap Placement	Channel Bottom
Safety Factor	1.1

Output Results:

Computed D50 0.04 ft

** FHWA Gradation**

Gradation Class	Facing
Layer Thickness	1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-2

Date: 02/12/2009 Time: 15:50

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
* COPYRIGHT (c) 2005                                     *
* WEST CONSULTANTS, INC.                                 *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX: 858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB: WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite Channel SEC-2

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	4.69 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	1.48 ft
Cotangent of Side slope	2.50
Safety Factor	1.1
Riprap Placement	Channel Bank
Rock Type	Angular

Output Results:

Computed D30	0.11 ft
Computed Local Depth Averaged velocity	4.69 ft/s
Local velocity/Avg. velocity	1.00
Side slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

ONSITE Channel Sec-2
2.

w15

5.

ASCE Method

Input Parameters:

Local velocity 4.69 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.16 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 4.69 ft/s

Output Results:

Computed D50 0.29 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

ONSITE Channel Sec-2
USGS Method

Input Parameters:

Average Channel Velocity 4.69 ft/s

Output Results:

Computed D50 0.43 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 4.69 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.28 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-2

 Average Channel Velocity 4.69 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.
 Riprap Placement Channel Bank
 Safety Factor 1.1

Output Results:

Computed D50 0.06 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock Weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-2_btm

Date: 02/12/2009 Time: 15:50

```
*****
*                               *
*       RIPRAP DESIGN SYSTEM (RDS)       *
*                               *
*               BY               *
*       WEST Consultants, Inc.           *
*                               *
*                               *
* Version 3.0                          March, 2005 *
*                               *
*                               *
* COPYRIGHT (c) 2005                   *
* WEST CONSULTANTS, INC.               *
* 16870 WEST BERNARDO DRIVE            PH: 858-487-9378 *
* SUITE 340                            FAX:858-487-9448 *
* SAN DIEGO, CA 92127                  WEB:WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite Channel SEC-2_btm

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	4.69 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165.7 lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	1.48 ft
Cotangent of Side Slope	N/A
Safety Factor	1.1
Riprap Placement	Channel Bottom
Rock Type	Angular

Output Results:

Computed D30	0.11 ft
Computed Local Depth Averaged Velocity	4.69 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

ONSITE Channel Sec-2_btm
USGS Method

Input Parameters:

Average Channel Velocity 4.69 ft/s

Output Results:

Computed D50 0.43 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 4.69 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.28 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-2_btm

 Average Channel Velocity 4.69 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope N/A
 Material Angle of Repose deg.
 Riprap Placement Channel Bottom
 Safety Factor 1.1

Output Results:

Computed D50 0.05 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-3
2.

w15

5.

ASCE Method

Input Parameters:

Local velocity 3.63 ft/s
 Cotangent of Side slope 2.50
 Unit weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.09 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 3.63 ft/s

Output Results:

Computed D50 0.17 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

ONSITE Channel Sec-3
USGS Method

Input Parameters:

Average Channel Velocity 3.63 ft/s

Output Results:

Computed D50 0.23 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 3.63 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.17 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-3

Average Channel Velocity	3.63 ft/s
Average Flow Depth	4.00 ft
Unit Weight of Stone	165. lbs/cu ft
Cotangent of Side Slope	2.50
Material Angle of Repose	41.00 deg.
Riprap Placement	Channel Bank
Safety Factor	1.1

Output Results:

Computed D50	0.03 ft
--------------	---------

** FHWA Gradation**

Gradation Class	Facing
Layer Thickness	1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-3_btm
USGS Method

Input Parameters:

Average Channel Velocity 3.63 ft/s

Output Results:

Computed D50 0.23 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 3.63 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.17 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-3_btm

 Average Channel Velocity 3.63 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope N/A
 Material Angle of Repose deg.
 Riprap Placement Channel Bottom
 Safety Factor 1.1

Output Results:

Computed D50 0.02 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-4
2.

w15

5.

ASCE Method

Input Parameters:

Local Velocity 4.63 ft/s
 Cotangent of Side slope 2.50
 Unit weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.15 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 4.63 ft/s

Output Results:

Computed D50 0.29 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

ONSITE Channel Sec-4
USGS Method

Input Parameters:

Average Channel velocity 4.63 ft/s

Output Results:

Computed D50 0.42 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel velocity 4.63 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.27 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-4

 Average Channel Velocity 4.63 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.
 Riprap Placement Channel Bank
 Safety Factor 1.1

Output Results:

Computed D50 0.06 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent smaller by size	Rock size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-4_btm

Date: 02/12/2009 Time: 16:31

```
*****
*                               *
*             RIPRAP DESIGN SYSTEM (RDS)             *
*                               *
*                   BY                               *
*             WEST Consultants, Inc.                 *
*                               *
*                               *
* Version 3.0                                     March, 2005 *
*                               *
*                               *
* COPYRIGHT (c) 2005                               *
* WEST CONSULTANTS, INC.                           *
* 16870 WEST BERNARDO DRIVE                         PH: 858-487-9378 *
* SUITE 340                                         FAX:858-487-9448 *
* SAN DIEGO, CA 92127                             WEB:WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite Channel SEC-4_btm

USACE Method

Input Parameters:

```
-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    4.63 ft/s
Bottom width                N/A
Bend Radius                 N/A
Top Width                   N/A
Unit Weight of Stone        165. lbs/cu ft
Riprap Layer Thickness      1.00
Local Flow Depth           1.49 ft
Cotangent of Side Slope    N/A
Safety Factor               1.1
Riprap Placement           Channel Bottom
Rock Type                   Angular
```

Output Results:

```
-----
Computed D30                 0.10 ft
Computed Local Depth Averaged Velocity 4.63 ft/s
Local Velocity/Avg. Velocity 1.00
Side Slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
```

*** Using Gradations from COE ETL 1110-2-120 ***

```
Specific weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30    0.37 ft
Selected Minimum D90    0.53 ft
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

Input Parameters:

Average Channel Velocity 4.63 ft/s

Output Results:

Computed D50 0.42 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 4.63 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.27 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-4_btm

 Average Channel Velocity 4.63 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope N/A
 Material Angle of Repose deg.
 Riprap Placement Channel Bottom
 Safety Factor 1.1

Output Results:

Computed D50 0.04 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock Weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-5-6-7

Date: 02/12/2009 Time: 16:32

```
*****
*                               *
*       RIPRAP DESIGN SYSTEM (RDS)       *
*                               *
*               BY               *
*       WEST Consultants, Inc.           *
*                               *
*                               *
* Version 3.0                         March, 2005 *
*                               *
*                               *
* COPYRIGHT (c) 2005                 *
* WEST CONSULTANTS, INC.             *
* 16870 WEST BERNARDO DRIVE          PH: 858-487-9378 *
* SUITE 340                          FAX:858-487-9448 *
* SAN DIEGO, CA 92127                WEB:WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
 Description: Onsite Channel SEC-5-6-7

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	3.53 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	0.92 ft
Cotangent of Side Slope	2.50
Safety Factor	1.1
Riprap Placement	Channel Bank
Rock Type	Angular

Output Results:

Computed D30	0.06 ft
Computed Local Depth Averaged Velocity	3.53 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

ONSITE Channel Sec-5-6-7
2.

w15

5.

ASCE Method

Input Parameters:

Local Velocity 3.53 ft/s
 Cotangent of Side slope 2.50
 Unit weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.09 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 3.53 ft/s

Output Results:

Computed D50 0.16 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

ONSITE Channel Sec-5-6-7
USGS Method

Input Parameters:

Average Channel velocity 3.53 ft/s

Output Results:

Computed D50 0.22 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel velocity 3.53 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.16 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-5-6-7

Average Channel Velocity	3.53 ft/s
Average Flow Depth	4.00 ft
Unit weight of Stone	165. lbs/cu ft
Cotangent of Side Slope	2.50
Material Angle of Repose	41.00 deg.
Riprap Placement	Channel Bank
Safety Factor	1.1

Output Results:

Computed D50	0.03 ft
--------------	---------

** FHWA Gradation**

Gradation Class	Facing
Layer Thickness	1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Input Parameters:

Average Channel velocity 3.53 ft/s

Output Results:

Computed D50 0.22 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel velocity 3.53 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.16 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-5-6-7_btm

Average Channel Velocity	3.53 ft/s
Average Flow Depth	4.00 ft
Unit weight of Stone	165. lbs/cu ft
Cotangent of Side Slope	N/A
Material Angle of Repose	deg.
Riprap Placement	Channel Bottom
Safety Factor	1.1

Output Results:

Computed D50	0.02 ft
--------------	---------

** FHWA Gradation**

Gradation Class	Facing
Layer Thickness	1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel DROP Sec-7

Date: 02/12/2009 Time: 16:41

```

*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                                                         *
* Version 3.0                                           March, 2005 *
*                                                         *
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* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                    WEB:WWW.WESTCONSULTANTS.COM *
*****
    
```

Project: Cholla Ash Onsite
 Description: Onsite Channel DROP SEC-7

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	9.44 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	0.39 ft
Cotangent of Side Slope	2.50
Safety Factor	1.1
Riprap Placement	Channel Bank
Rock Type	Angular

Output Results:

Computed D30	0.85 ft
Computed Local Depth Averaged Velocity	9.44 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	2.000 ft
Selected Minimum D30	0.97 ft
Selected Minimum D90	1.41 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	276.	691.
w50	138.	205.

ONSITE Channel DROP Sec-7
43.

W15

102.

ASCE Method

Input Parameters:

Local Velocity	9.44 ft/s
Cotangent of Side slope	2.50
Unit Weight of Stone	165. lbs/cu ft
Riprap Placement	Channel Bank

Output Results:

Computed D50 0.63 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	1.250 ft
Selected Minimum D30	0.61 ft
Selected Minimum D90	0.88 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	67.	169.
W50	34.	50.
W15	11.	25.

USB Method

Input Parameters:

Average Channel Velocity 9.44 ft/s

Output Results:

Computed D50 1.24 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	2.250 ft
Selected Minimum D30	1.10 ft
Selected Minimum D90	1.59 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	394.	984.
W50	197.	291.
W15	62.	146.

ONSITE Channel DROP Sec-7
USGS Method

Input Parameters:

Average Channel Velocity 9.44 ft/s

Output Results:

Computed D50 2.39 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 4.500 ft
Selected Minimum D30 2.19 ft
Selected Minimum D90 3.17 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	3149.	7873.
w50	1575.	2330.
w15	492.	1165.

Isbash Method

Input Parameters:

Average Channel velocity 9.44 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 1.14 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 2.000 ft
Selected Minimum D30 0.97 ft
Selected Minimum D90 1.41 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	276.	691.
w50	138.	205.
w15	43.	102.

HEC-11 Method

Input Parameters:

ONSITE Channel DROP Sec-7

Average Channel Velocity	9.44 ft/s
Average Flow Depth	4.00 ft
Unit Weight of Stone	165. lbs/cu ft
Cotangent of Side Slope	2.50
Material Angle of Repose	41.00 deg.
Riprap Placement	Channel Bank
Safety Factor	1.1

Output Results:

Computed D50	0.50 ft
--------------	---------

** FHWA Gradation**

Gradation Class	Facing
Layer Thickness	1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel DROP Sec-7_btm

Date: 02/12/2009 Time: 16:41

```

*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
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* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB:WWW.WESTCONSULTANTS.COM *
*****
    
```

Project: Cholla Ash Onsite
 Description: Onsite Channel DROP SEC-7_btm

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	9.44 ft/s
Bottom width	N/A
Bend Radius	N/A
Top Width	N/A
Unit Weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	0.39 ft
Cotangent of Side slope	N/A
Safety Factor	1.1
Riprap Placement	Channel Bottom
Rock Type	Angular

Output Results:

Computed D30	0.85 ft
Computed Local Depth Averaged Velocity	9.44 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight	165.0 lbs/cu ft
Layer Thickness	2.000 ft
Selected Minimum D30	0.97 ft
Selected Minimum D90	1.41 ft

Percent Lighter by weight	Stone Weight, lbs	
	Minimum	Maximum
w100	276.	691.
w50	138.	205.

ONSITE Channel DROP Sec-7_btm
USGS Method

Input Parameters:

Average Channel velocity 9.44 ft/s

Output Results:

Computed D50 2.39 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 4.500 ft
Selected Minimum D30 2.19 ft
Selected Minimum D90 3.17 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	3149.	7873.
w50	1575.	2330.
w15	492.	1165.

Isbash Method

Input Parameters:

Average Channel velocity 9.44 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 1.14 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 2.000 ft
Selected Minimum D30 0.97 ft
Selected Minimum D90 1.41 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	276.	691.
w50	138.	205.
w15	43.	102.

HEC-11 Method

Input Parameters:

ONSITE Channel DROP Sec-7_btm

```

-----
Average Channel Velocity          9.44 ft/s
Average Flow Depth                4.00 ft
Unit weight of Stone             165. lbs/cu ft
Cotangent of Side Slope          N/A
Material Angle of Repose         deg.
Riprap Placement                 Channel Bottom
Safety Factor                     1.1
  
```

Output Results:

```

-----
Computed D50                      0.37 ft
  
```

** FHWA Gradation**

```

Gradation Class      Facing
Layer Thickness      1.90 ft
  
```

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

ONSITE Channel Sec-8-9

Date: 02/12/2009 Time: 16:42

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
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* 16870 WEST BERNARDO DRIVE                             PH: 858-487-9378 *
* SUITE 340                                             FAX: 858-487-9448 *
* SAN DIEGO, CA 92127                                 WEB: WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Onsite
Description: Onsite Channel SEC-8-9

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	2.79 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	1.12 ft
Cotangent of side slope	2.50
Safety Factor	1.1
Riprap Placement	Channel Bank
Rock Type	Angular

Output Results:

Computed D30	0.03 ft
Computed Local Depth Averaged Velocity	2.79 ft/s
Local Velocity/Avg. Velocity	1.00
Side slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.

ONSITE Channel Sec-8-9
2.

W15

5.

ASCE Method

Input Parameters:

Local Velocity 2.79 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.05 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

USB Method

Input Parameters:

Average Channel Velocity 2.79 ft/s

Output Results:

Computed D50 0.10 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

ONSITE Channel Sec-8-9
USGS Method

Input Parameters:

Average Channel velocity 2.79 ft/s

Output Results:

Computed D50 0.12 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel velocity 2.79 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.10 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

HEC-11 Method

Input Parameters:

ONSITE Channel Sec-8-9

Average Channel Velocity	2.79 ft/s
Average Flow Depth	4.00 ft
Unit weight of Stone	165. lbs/cu ft
Cotangent of Side Slope	2.50
Material Angle of Repose	41.00 deg.
Riprap Placement	Channel Bank
Safety Factor	1.1

Output Results:

Computed D50	0.01 ft
--------------	---------

** FHWA Gradation**

Gradation Class	Facing
Layer Thickness	1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Date: 02/12/2009 Time: 16:43

```

*****
*                               RIPRAP DESIGN SYSTEM (RDS)                               *
*                               BY                                           *
*                               WEST Consultants, Inc.                         *
*                               *                                               *
*                               *                                               *
* Version 3.0                               March, 2005 *
*                               *                                               *
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* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                               FAX: 858-487-9448 *
* SAN DIEGO, CA 92127                               WEB: WWW.WESTCONSULTANTS.COM *
*****

```

Project: Cholla Ash Onsite
Description: Onsite Channel SEC-8-9_btm

_____ USACE Method _____

Input Parameters:

```

-----
Velocity Type                               Average
Channel Shape                               Trapezoidal
Channel Type                               Straight
Bend Angle (deg)                           N/A
Average Channel Velocity                     2.79 ft/s
Bottom width                                N/A
Bend Radius                                  N/A
Top Width                                    N/A
Unit Weight of Stone                         165. lbs/cu ft
Riprap Layer Thickness                       1.00
Local Flow Depth                             1.12 ft
Cotangent of Side Slope                      N/A
Safety Factor                                1.1
Riprap Placement                            Channel Bottom
Rock Type                                    Angular

```

Output Results:

```

-----
Computed D30                                0.03 ft
Computed Local Depth Averaged Velocity       2.79 ft/s
Local Velocity/Avg. Velocity                 1.00
Side Slope Correction Factor                 1.06
Correction for Layer Thickness                1.00
Correction for Secondary Currents            1.00

```

*** Using Gradations from COE ETL 1110-2-120 ***

```

Specific Weight      165.0 lbs/cu ft
Layer Thickness      0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

```

Percent Lighter by Weight	Stone Weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

ASCE Method

Input Parameters:

Local Velocity	2.79 ft/s
Cotangent of Side slope	N/A
Unit Weight of Stone	165. lbs/cu ft
Riprap Placement	Channel Bottom

Output Results:

Computed D50 0.05 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by Weight	Stone Weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

USB Method

Input Parameters:

Average Channel Velocity	2.79 ft/s
--------------------------	-----------

Output Results:

Computed D50 0.10 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft

Specific Weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by Weight	Stone Weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

HEC-11 Method

Input Parameters:

Average Channel Velocity 2.79 ft/s
 Average Flow Depth 4.00 ft
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope N/A
 Material Angle of Repose deg.
 Riprap Placement Channel Bottom
 Safety Factor 1.1

Output Results:

Computed D50 0.01 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock Weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

**ON-SITE CHANNEL
DROP STRUCTURE CALCULATIONS**

Cholla Ash Monofill
Riprap Basin Sizing and Hydraulic Jump
Onsite Drop Structure SOUTH Channel Into Basin

	A	B	C	D	E	F	G	H	I	J
1	Riprap Basin Sizing									
2										
3	Q=	25 cfs		flow rate for onsite SOUTH channel into basin						
4	Vallow=	0 ft/s		velocity for basin						
5	TW=	10 ft		normal depth for basin						
6	Wo=	10 ft		channel bottom width						
7										
8	From FHWA Hydraulic Design of Energy Dissipators for Culverts and Channels, July 2006									
9	1) Get intial velocity (Vo), depth (yo), and Froude Number (Fr) for brink conditions									
10										
11	From Flowmaster output files:									
12	yo= ye=	0.29 ft		normal depth for drop						
13	Vo=	7.96 ft/s		velocity for drop						
14	Fr=	2.68		Froude number for drop						
15										
16	2) Select trial D50 and obtain hs/ye from Equation 10.1.									
17										
18	Equation 10.1	$\frac{h_s}{y_e} = 0.86 \left(\frac{D_{50}}{y_e} \right)^{-0.55} \left(\frac{V_o}{\sqrt{g y_e}} \right) - C_o$								
19										
20										
21										
22	Get tailwater parameter Co:									
23	Co =	1.4 if TW/ye < 0.75								
24	Co =	136.331 if 0.75 < TW/ye < 1.0								
25	Co =	3 if 1.0 < TW/ye								
26										
27	TW/ye =	34.48276								
28										
29	D50 =	0.11 ft	D50 of riprap							
30										
31	Check to see that hs/D50 >= 2 and that D50/ye >= 0.1.									
32	D50/ye =	0.37931 >= 0.1	OK							
33	g =	32.2 ft/s ²								
34	hs/ye =	0.928133								
35	hs =	0.269159 ft	scour depth							
36	hs/D50 =	2.446896 >= 2.0	OK							
37										
38	3) Size the basin									
39	Ls =	10*hs =	2.69 ft	Dissipator Pool Length						
40	Lsmin =	3Wo =	30 ft							
41										
42	Lb =	15hs =	4.04 ft	Total Pool Length						
43	Lbmin =	4Wo =	40 ft							
44	La =	10 ft	Apron Length							
45	Wb =	10 ft	maintaining channel bottom depth							
46										

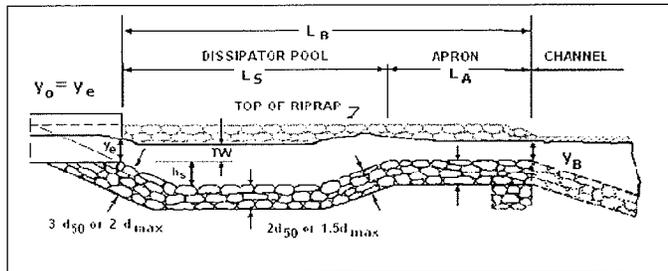


Figure 10.1. Profile of Riprap Basin

Cholla Ash Monofill
Riprap Basin Sizing and Hydraulic Jump
Onsite Drop Structure SOUTH Channel Into Basin

	A	B	C	D	E	F	G	H	I	J
47	Hydraulic Jump									
48	From Drainage Design Manual for Maricopa County, Hydraulics: Hydraulic Structures, 2003									
49	Y1 =	0.29 ft	upstream normal depth for drop							
50	Ydn =	10 ft	downstream normal depth basin							
51	Q =	25 cfs	flow through the drop							
52	g =	32.2 ft/s ²								
53	A1 =	3.14 ft ²	area of flow through the drop							
54	A2 =	10.43 ft ²	area of flow in next section							
55	z =	2.5 ft	sideslope H:1							
56	b =	10 ft	bottom width of channel							
57										
58	2) Calculate sequant height of jump.									
59	Equation 7.2									
60	$Y_2 = \frac{1}{2} Y_1 \left[\left(1 + 8F_{r1}^2 \right)^{\frac{1}{2}} - 1 \right]$									
61										
62										
63	Y2 =	0.96 ft	OK	height of jump						
64										
65	3) Another check on sequant height of jump.									
66	Use Fig. 7-8									
67										
68	Fr1 =	$\frac{V}{\sqrt{gy_m}}$	V =	7.96 ft/s						
69			top width =	11.46 ft						
70			ym =	0.27 ft	= flow area / top width					
71			Fr1 =	2.68						
72	J = Y2 / Y1		t = b/(zy1)							
73	J =	3.1	t =	13.79						
74	Y2 =	0.899 ft	use larger height of jump							
75										
76	1) Calculate depth at beginning location of jump.									
77	Equation 7.3									
78	$\frac{ZY_1^3}{3} + \frac{ZY_1^2}{2} + \frac{Q}{gA_1} = \frac{ZY_2^3}{3} + \frac{bY_2^2}{3} + \frac{Q}{gA_2}$									
79										
80										
81	Leq =	0.373	Req =	0.375	(Plug in values for Y2 until both sides equal)					
82	Yb =	0.29 ft	OK	depth at jump location						
83										
84	4) Calculate length of jump.									
85	Use Fig. 7-9									
86	Lj / y1 =	33								
87	Lj =	9.57 ft	= jump length							
88										
89	Therefore: Min. Length of jump = 30 ft									
90	Min. Length of apron = 10 ft									
91	Total Length of basin = 40 ft									

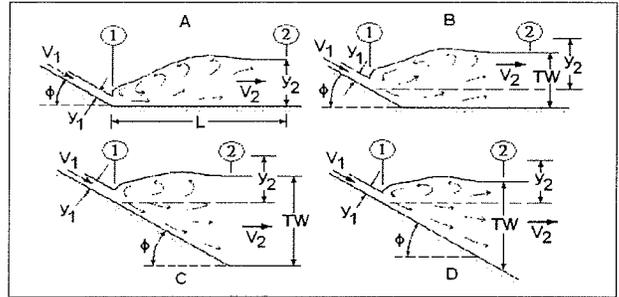


Figure 6.10. Hydraulic Jump Types Sloping Channels (Bradley, 1961)

Cholla Ash Monofill
Riprap Basin Sizing and Hydraulic Jump
Onsite Channel SEC-1 Into Basin

	A	B	C	D	E	F	G	H	I	J
1	Riprap Basin Sizing									
2										
3	Q=	95 cfs	flow rate for onsite channel SEC-1 into basin							
4	Vallow=	0 ft/s	velocity for basin							
5	TW=	10 ft	normal depth for basin							
6	Wo=	10 ft	channel bottom width							
7										
8	From FHWA Hydraulic Design of Energy Dissipators for Culverts and Channels, July 2006									
9	1) Get initial velocity (Vo), depth (yo), and Froude Number (Fr) for brink conditions									
10										
11	From Flowmaster output files:									
12	yo= ye=	0.64 ft	normal depth for drop							
13	Vo=	12.79 ft/s	velocity for drop							
14	Fr=	3.01	Froude number for drop							
15										
16	2) Select trial D50 and obtain hs/ye from Equation 10.1.									
17										
18	Equation 10.1	$\frac{h_s}{y_e} = 0.86 \left(\frac{D_{50}}{y_e} \right)^{-0.55} \left(\frac{V_o}{\sqrt{g y_e}} \right) - C_o$								
19										
20										
21										
22	Get tailwater parameter Co:									
23	Co =	1.4 if TW/ye < 0.75								
24	Co =	60.9 if 0.75 < TW/ye < 1.0								
25	Co =	3 if 1.0 < TW/ye								
26										
27	TW/ye =	15.625								
28										
29	D50 =	0.29 ft	D50 of riprap							
30										
31	Check to see that hs/D50 >= 2 and that D50/ye >= 0.1.									
32	D50/ye =	0.453125 >= 0.1	OK							
33	g =	32.2 ft/s ²								
34	hs/ye =	1.000783								
35	hs =	0.640501 ft	scour depth							
36	hs/D50 =	2.208625 >= 2.0	OK							
37										
38	3) Size the basin									
39	Ls =	10*hs =	6.41 ft	Dissipator Pool Length						
40	Lsmin =	3Wo =	30 ft							
41										
42	Lb =	15hs =	9.61 ft	Total Pool Length						
43	Lbmin =	4Wo =	40 ft							
44	La =	10 ft	Apron Length							
45	Wb =	10 ft	maintaining channel bottom depth							
46										

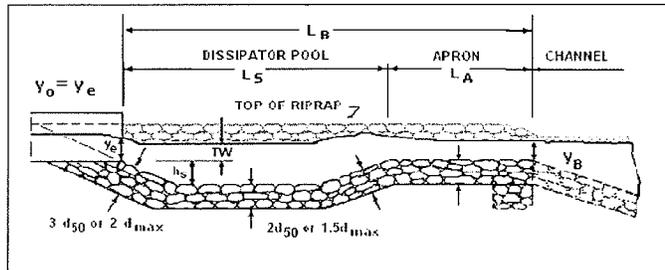


Figure 10.1. Profile of Riprap Basin

Cholla Ash Monofill
Riprap Basin Sizing and Hydraulic Jump
Onsite Channel SEC-1 Into Basin

	A	B	C	D	E	F	G	H	I	J
47	Hydraulic Jump									
48	From Drainage Design Manual for Maricopa County, Hydraulics: Hydraulic Structures, 2003									
49	Y1 =	0.64 ft	upstream normal depth for drop							
50	Ydn =	10 ft	downstream normal depth basin							
51	Q =	95 cfs	flow through the drop							
52	g =	32.2 ft/s ²								
53	A1 =	7.43 ft ²	area of flow through the drop							
54	A2 =	10.43 ft ²	area of flow in next section							
55	z =	2.5 ft	sideslope H:1							
56	b =	10 ft	bottom width of channel							
57										
58	2) Calculate sequant height of jump.									
59	Equation 7.2									
60	$Y_2 = \frac{1}{2} Y_1 \left[\left(1 + 8F_{r1}^2 \right)^{\frac{1}{2}} - 1 \right]$									
61										
62										
63	Y2 =	2.42 ft	OK	height of jump						
64										
65	3) Another check on sequant height of jump.									
66	Use Fig. 7-8									
67										
68	Fr1 =	$\frac{V}{\sqrt{gy_m}}$	V =	12.79 ft/s						
69			top width =	13.20 ft						
70			ym =	0.56 ft	= flow area / top width					
71			Fr1 =	3.00						
72	J = Y2 / Y1		t = b/(zy1)							
73	J =	3.5	t =	6.25						
74	Y2 =	2.24 ft	use larger height of jump							
75										
76	1) Calculate depth at beginning location of jump.									
77	Equation 7.3									
78	$\frac{ZY_1^3}{3} + \frac{ZY_1^2}{2} + \frac{Q}{gA_1} = \frac{ZY_2^3}{3} + \frac{bY_2^2}{3} + \frac{Q}{gA_2}$									
79										
80										
81	Leq =	1.128	Req =	1.143	(Plug in values for Y2 until both sides equal)					
82	Yb =	0.48 ft	OK	depth at jump location						
83										
84	4) Calculate length of jump.									
85	Use Fig. 7-9									
86	Lj / y1 =	55								
87	Lj =	35.2 ft	= jump length							
88										
89	Therefore: Min. Length of jump =		36 ft							
90	Min. Length of apron =		10 ft							
91	Total Length of basin =		46 ft							

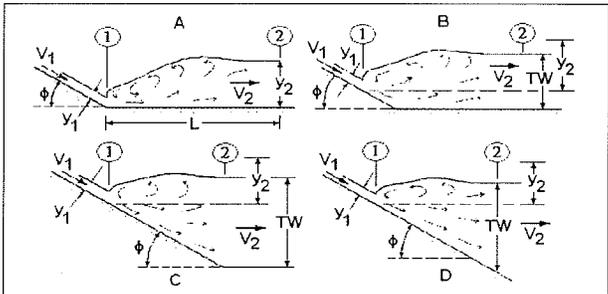


Figure 6.10. Hydraulic Jump Types Sloping Channels (Bradley, 1961)

Cholla Ash Monofill
Riprap Sizing and Hydraulic Jump
Onsite Channel SEC-7 Drop Structure

	A	B	C	D	E	F	G	H	I	J
1	Riprap Basin Sizing									
2										
3	Q=	40 cfs	flow rate for onsite channel SEC-7, just before brink of drop							
4	Vallow=	3.53 ft/s	velocity for onsite channel SEC-6, past drop							
5	TW=	0.92 ft	normal depth for onsite channel SEC-6, past drop							
6	Wo=	10 ft	channel bottom width							
7										
8	From FHWA Hydraulic Design of Energy Dissipators for Culverts and Channels, July 2006									
9	1) Get initial velocity (Vo), depth (yo), and Froude Number (Fr) for brink conditions									
10										
11	From Flowmaster output files:									
12	yo= ye=	0.39 ft	normal depth for drop							
13	Vo=	9.44 ft/s	velocity for drop							
14	Fr=	2.79	Froude number for drop							
15										
16	2) Select trial D50 and obtain hs/ye from Equation 10.1.									
17										
18	Equation 10.1	$\frac{h_s}{y_e} = 0.86 \left(\frac{D_{50}}{y_e} \right)^{-0.55} \left(\frac{V_o}{\sqrt{g y_e}} \right) - C_o$								
19										
20										
21										
22	Get tailwater parameter Co:									
23	Co =	1.4 if TW/ye < 0.75								
24	Co =	7.835897 if 0.75 < TW/ye < 1.0								
25	Co =	3 if 1.0 < TW/ye								
26										
27	TW/ye =	2.358974								
28										
29	D50 =	0.16 ft	D50 of riprap							
30										
31	Check to see that hs/D50 >= 2 and that D50/ye >= 0.1.									
32	D50/ye =	0.410256 >= 0.1	OK							
33	g =	32.2 ft/s ²								
34	hs/ye =	0.916717								
35	hs =	0.35752 ft	scour depth							
36	hs/D50 =	2.234498 >= 2.0	OK							
37										
38	3) Size the basin									
39	Ls =	10*hs =	3.58 ft	Dissipator Pool Length						
40	Lsmin =	3Wo =	30 ft							
41										
42	Lb =	15hs =	5.36 ft	Total Pool Length						
43	Lbmin =	4Wo =	40 ft							
44	La =	10 ft	Apron Length							
45	Wb =	10 ft	maintaining channel bottom depth							
46										
47	4) Assess need for downstream riprap due to TW/ye > 0.74									
48	Using Figure 10.3									
49	Compute equivalent circular diameter, De, for brink area									
50	A =	0.3588	$A = \frac{\pi D_e^2}{4} = y_o W_o$							
51	De =	0.68								
52										
53	Rock size for riprap after energy dissipators Equation 10.6									
54	S =	2.64	specific gravity of rock							
55										
56	L/De	L (ft)	VI/Vo (Fig. 10.3)	VI (ft/s)	D50 (ft)					
57	14.79513	10	0.42	3.9648	0.10					
58	22.19269	15	0.4	3.776	0.09					
59	25.15172	17	0.38	3.5872	0.08					
60	29.59026	20	0.3	2.832	0.05					

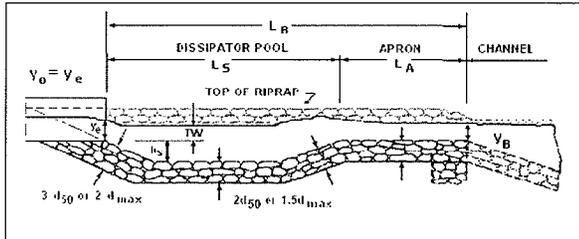


Figure 10.1. Profile of Riprap Basin

Riprap placement required for 17 ft from brink.

Cholla Ash Monofill
Riprap Sizing and Hydraulic Jump
Onsite Channel SEC-7 Drop Structure

	A	B	C	D	E	F	G	H	I	J
61	Hydraulic Jump									
62	From Drainage Design Manual for Maricopa County, Hydraulics: Hydraulic Structures, 2003									
63	Y1 =	0.39 ft	upstream normal depth for drop							
64	Ydn =	0.92 ft	downstream normal depth							
65	Q =	40 cfs	flow through the drop							
66	g =	32.2 ft/s ²								
67	A1 =	4.24 ft ²	area of flow through the drop							
68	A2 =	11.34 ft ²	area of flow in next section							
69	z =	2.5 ft	sideslope H:1							
70	b =	10 ft	bottom width of channel							
71										
72	2) Calculate sequant height of jump.									
73	Equation 7.2									
74	$Y_2 = \frac{1}{2} Y_1 \left[\left(1 + 8F_{r1}^2 \right)^{\frac{1}{2}} - 1 \right]$									
75										
76										
77	Y2 =	1.36 ft	OK	height of jump						
78										
79	3) Another check on sequant height of jump.									
80	Use Fig. 7-8									
81			V =	9.43 ft/s						
82	Fr1 =	$\frac{V}{\sqrt{gy_m}}$	top width =	11.93 ft						
83			ym =	0.36 ft						
84			Fr1 =	2.79						
85										
86	J = Y2 / Y1		t = b/(zy1)							
87	J =	3.4	t =	10.26						
88	Y2 =	1.326 ft	use larger of Y2 for max height of jump							
89										
90	1) Calculate depth at beginning location of jump.									
91	Equation 7.3									
92	$\frac{ZY_1^3}{3} + \frac{ZY_1^2}{2} + \frac{Q}{gA_1} = \frac{ZY_b^3}{3} + \frac{bY_b^2}{3} + \frac{Q}{gA_b}$									
93										
94										
95	Leq =	0.533	Req =	0.528 (Plug in values for Yb until both sides equal)						
96	Yb =	0.34 ft	OK	depth at beginning jump location						
97										
98	4) Calculate length of jump.									
99	Use Fig. 7-9									
100	Lj / y1 =	41								
101	Lj =	15.99 ft	= jump length							
102										
103	Therefore: Min. Length of jump =	30 ft								
104	Min. Length of apron =	10 ft								
105	Total Length of basin =	40 ft								
106	Total Length of required riprap =	17 ft (from brink of drop)								

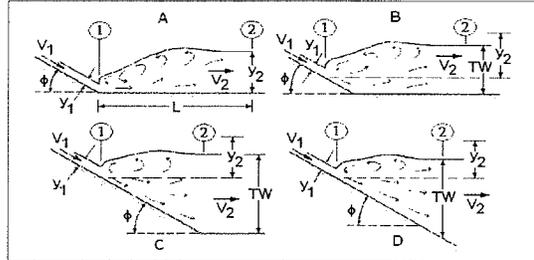


Figure 6.10. Hydraulic Jump Types Sloping Channels (Bradley, 1961)

Cholla Ash Monofill
Sloping Drop Structure
Riprap Sizing and Hydraulic Jump Formulas

	A	B	C	D	E	F	G	H	I
1	Riprap Basin Sizing								
2									
3	Q=	80	cfs	flow rate for section 6, just before brink of drop					
4	Vallow=	4.39	ft/s	velocity for section 5, past drop					
5	TW=	1.36	ft	normal depth for section 5, past drop					
6	Wo=	10	ft	channel bottom w					
7									
8	From FHWA Hydraulic Design of Energy Dissipators for Culverts and Channels, July 2006								
9	1) Get intial velocity (Vo), depth (yo), and Froude Number (Fr) for brink conditions								
10									
11	From Flowmaster output files:								
12	yo= ye=	0.58	ft	normal depth for c					
13	Vo=	12.05	ft/s	velocity for drop					
14	Fr=	2.96		Froude number for drop					
15									
16	2) Select trial D50 and obtain hs/ye from Equation 10.1.								
17									
18	Equation 10.1	$\frac{h_s}{y_e} = 0.86 \left(\frac{D_{50}}{y_e} \right)^{-0.55} \left(\frac{V_o}{\sqrt{g y_e}} \right) - C_o$							
19									
20									
21									
22	Get tailwater parameter Co:								
23	Co =	1.4		if TW/ye < 0.75					
24	Co =	=4*B5/B12-1.6		if 0.75 < TW/ye < 1.0					
25	Co =	3		if 1.0 < TW/ye					
26									
27	TW/ye =	=B5/B12							
28									
29	D50 =	0.25	ft	D50 of riprap					
30									
31	Check to see that hs/D50 >= 2 and that D50/ye >= 0.1.								
32	D50/ye =	=B29/B12	>= 0.1	OK					
33	g =	32.2	ft/s ²						
34	hs/ye =	=0.86*((B32)^(-0.55))*(B14)-B25							
35	hs =	=B34*B12	ft	scour depth					
36	hs/D50 =	=B35/B29	>=2.0	OK					
37									
38	3) Size the basin								
39	Ls =	10*hs =	=10*B35	ft	Dissipator Pool Length				
40	Lsmin =	3Wo =	=3*B6	ft					
41									
42	Lb =	15hs =	=15*B35	ft	Total Pool Length				
43	Lbmin =	4Wo =	=4*B6	ft					
44	La =	=C43-C40	ft	Apron Length					
45	Wb =	10	ft	maintaining channel bottom depth					
46									
47	4) Assess need for downstream riprap due to TW/ye >0.74 (for Drop Structures in channel only)								
48	Using Figure 10.3								
49	Compute equivalent circular diameter, De, for brink area								
50	A =	=B5*B12	$A = \frac{\pi D_e^2}{4} = y_o W_o$						
51	De =	=(B50*4/PI())^0.5							
52									
53	Rock size for riprap aft								
54	S =	=165/62.4	specific gravity of rock	$D_{50} = \frac{0.692}{S-1} \left(\frac{V^2}{2g} \right)$					
55									

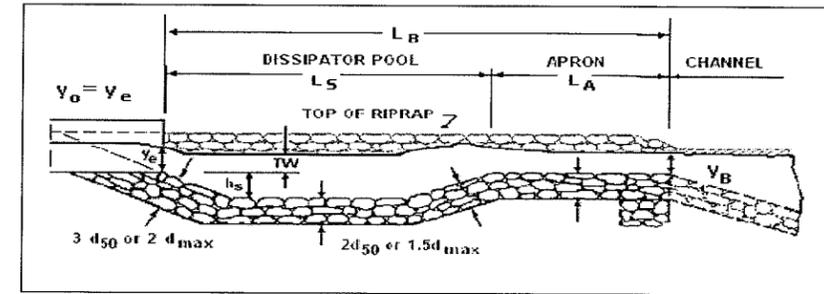


Figure 10.1. Profile of Riprap Basin

Cholla Ash Monofill
Sloping Drop Structure
Riprap Sizing and Hydraulic Jump Formulas

	A	B	C	D	E	F	G	H	I
56	L/De	L (ft)	VI/Vo (Fig. 10.3)	VI (ft/s)	D50 (ft)				
57	=B57/\$B\$51	10	0.42	=C57*\$B\$13	=(0.692/(\$B\$54-1))*((D57^2)/(2*"Drop Off-basin!"\$B\$33))				
58	=B58/\$B\$51	15	0.4	=C58*\$B\$13	=(0.692/(\$B\$54-1))*((D58^2)/(2*"Drop Off-basin!"\$B\$33))				
59	=B59/\$B\$51	17	0.38	=C59*\$B\$13	=(0.692/(\$B\$54-1))*((D59^2)/(2*"Drop Off-basin!"\$B\$33))				
60	=B60/\$B\$51	20	0.3	=C60*\$B\$13	=(0.692/(\$B\$54-1))*((D60^2)/(2*"Drop Off-basin!"\$B\$33))				
61									
62	Hydraulic Jump								
63	From Drainage Design Manual for Maricopa County, Hydraulics: Hydraulic Structures, 2003								
64	Y1 =	0.58	ft	upstream normal depth for drop					
65	Ydn =	1.36	ft	downstream normal depth section 5					
66	Q =	80	cfs	flow through the d					
67	g =	32.2	ft/s ²						
68	A1 =	6.64	ft ²	area of flow through the drop					
69	A2 =	10.43	ft ²	area of flow in next section					
70	z =	2.5	ft	sideslope H:1					
71	b =	10	ft	bottom width of channel					
72									
73	2) Calculate sequant height of jump.								
74	Equation 7.2								
75	$Y_2 = \frac{1}{2} Y_1 \left[(1 + 8F_{r1}^2)^{\frac{1}{2}} - 1 \right]$								
76									
77									
78	Y2 =	=0.5*B64*(((1+8*(D85^2))^0.5)-1)	ft	OK	height				
79									
80	3) Another check on sequant height of jump.								
81	Use Fig. 7-8								
82									
83	Fr1 =	$\frac{V}{\sqrt{gy_m}}$	V =	=B66/B68	ft/s				
84			top width =	12.9	ft				
85			ym =	=B68/D83	ft	= flow area / top width			
86			Fr1 =	=D82/SQRT(B67*D84)					
87	J = Y2 / Y1			t = b/(zy1)					
88	J =	3.5		t =	=B71/(B70*B64)				
89	Y2 =	=B88*B64	ft	use larger	height of jump				
90									
91	1) Calculate depth at beginning location of jump.								
92	Equation 7.3								
93	$\frac{ZY_1^3}{3} + \frac{ZY_1^2}{2} + \frac{Q}{gA_1} = \frac{ZY_2^3}{3} + \frac{bY_2^2}{3} + \frac{Q}{gA_2}$								
94									
95									
96	Leq =	=(B70*(B64^3)/3)+(B70*(B64^2)/2)+(B66/(B67*B68))	Req =	=(B70*B97^3/3)+(B71*B97^2/3)+(B66/(B67*B69))	(Plug in values for Y2 until both sides equal)				
97	Yb =	0.44	ft	OK	depth at jump location				
98									
99	4) Calculate length of jump.								
100	Use Fig. 7-9								
101	Lj / y1 =	55							
102	Lj =	=B101*B64	ft	= jump length					
103									
104	Therefore: Min. Length of jump = 32 ft								
105	Min. Length of apron = 10 ft								
106	Total Length of basin = =D105+D104 ft								

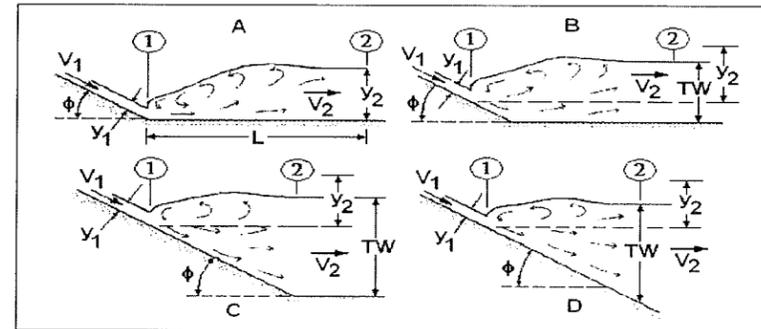


Figure 6.10. Hydraulic Jump Types Sloping Channels (Bradley, 1961)

ON-SITE STORAGE VOLUME CALCULATION

**Cholla Ash Monofill
On-Site Basin Calculation**

Depth	Storage Area (sq ft)	Storage Area (ac)	Volume *(1) (cu ft)	Volume (ac-ft)	Total Volume (ac ft)
0	18,225	0.42	0	0	0
9	35,721	0.82	238,383	5.473	5.5
12	45,369	1.04	121,347	2.786	8.3

Notes: The volume was calculated using the conic equation ($V = h/3 \times (a1 + a2 + (a1 \times a2)^{(1/2)})$)

OFF-SITE HYDRAULIC CALCULATIONS

**OFF-SITE CHANNEL
NORMAL DEPTH CALCULATIONS**

Worksheet for DROP Channel Section 1 (25 cfs-5:1)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.20000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	25.00 ft ³ /s

Results

Normal Depth	0.29 ft
Flow Area	3.14 ft ²
Wetted Perimeter	11.58 ft
Top Width	11.46 ft
Critical Depth	0.55 ft
Critical Slope	0.02311 ft/ft
Velocity	7.96 ft/s
Velocity Head	0.98 ft
Specific Energy	1.28 ft
Froude Number	2.68
Flow Type	Supercritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.29 ft
Critical Depth	0.55 ft
Channel Slope	0.20000 ft/ft
Critical Slope	0.02311 ft/ft

Worksheet for Channel Section 1 (30 cfs - 0.885% Slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.00885 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	25.00 ft ³ /s

Results

Normal Depth	0.73 ft
Flow Area	8.62 ft ²
Wetted Perimeter	13.93 ft
Top Width	13.65 ft
Critical Depth	0.55 ft
Critical Slope	0.02311 ft/ft
Velocity	2.90 ft/s
Velocity Head	0.13 ft
Specific Energy	0.86 ft
Froude Number	0.64
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.73 ft
Critical Depth	0.55 ft
Channel Slope	0.00885 ft/ft
Critical Slope	0.02311 ft/ft

Worksheet for Channel Section 2 (80 cfs - 0.104% Slope)

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01040 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	80.00 ft ³ /s

Results

Normal Depth	1.35 ft
Flow Area	17.97 ft ²
Wetted Perimeter	17.24 ft
Top Width	16.73 ft
Critical Depth	1.14 ft
Critical Slope	0.01896 ft/ft
Velocity	4.45 ft/s
Velocity Head	0.31 ft
Specific Energy	1.65 ft
Froude Number	0.76
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.35 ft
Critical Depth	1.14 ft
Channel Slope	0.01040 ft/ft
Critical Slope	0.01896 ft/ft

Worksheet for Channel Section 3 (140 cfs - 0.5% Slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.00500 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	140.00 ft ³ /s

Results

Normal Depth	2.22 ft
Flow Area	34.50 ft ²
Wetted Perimeter	21.95 ft
Top Width	21.10 ft
Critical Depth	1.59 ft
Critical Slope	0.01739 ft/ft
Velocity	4.06 ft/s
Velocity Head	0.26 ft
Specific Energy	2.48 ft
Froude Number	0.56
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	2.22 ft
Critical Depth	1.59 ft
Channel Slope	0.00500 ft/ft
Critical Slope	0.01739 ft/ft

Worksheet for Channel Section 4 (140 cfs - 1.0% Slope)

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	150.00 ft ³ /s

Results

Normal Depth	1.91 ft
Flow Area	28.32 ft ²
Wetted Perimeter	20.31 ft
Top Width	19.57 ft
Critical Depth	1.65 ft
Critical Slope	0.01721 ft/ft
Velocity	5.30 ft/s
Velocity Head	0.44 ft
Specific Energy	2.35 ft
Froude Number	0.78
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.91 ft
Critical Depth	1.65 ft
Channel Slope	0.01000 ft/ft
Critical Slope	0.01721 ft/ft

Worksheet for Channel Section 5 (80 cfs - 1.0% Slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	80.00 ft ³ /s

Results

Normal Depth	1.36 ft
Flow Area	18.22 ft ²
Wetted Perimeter	17.32 ft
Top Width	16.80 ft
Critical Depth	1.14 ft
Critical Slope	0.01896 ft/ft
Velocity	4.39 ft/s
Velocity Head	0.30 ft
Specific Energy	1.66 ft
Froude Number	0.74
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.36 ft
Critical Depth	1.14 ft
Channel Slope	0.01000 ft/ft
Critical Slope	0.01896 ft/ft

Worksheet for DROP STRUCTURE Channel Sec-6 (80 cfs-5:1)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.20000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	80.00 ft ³ /s

Results

Normal Depth	0.58 ft
Flow Area	6.64 ft ²
Wetted Perimeter	13.12 ft
Top Width	12.90 ft
Critical Depth	1.14 ft
Critical Slope	0.01896 ft/ft
Velocity	12.05 ft/s
Velocity Head	2.26 ft
Specific Energy	2.84 ft
Froude Number	2.96
Flow Type	Supercritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.58 ft
Critical Depth	1.14 ft
Channel Slope	0.20000 ft/ft
Critical Slope	0.01896 ft/ft

Worksheet for Channel Section 6 (80 cfs - 1.0% Slope)

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.01000 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	80.00 ft ³ /s

Results

Normal Depth	1.36 ft
Flow Area	18.22 ft ²
Wetted Perimeter	17.32 ft
Top Width	16.80 ft
Critical Depth	1.14 ft
Critical Slope	0.01896 ft/ft
Velocity	4.39 ft/s
Velocity Head	0.30 ft
Specific Energy	1.66 ft
Froude Number	0.74
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.36 ft
Critical Depth	1.14 ft
Channel Slope	0.01000 ft/ft
Critical Slope	0.01896 ft/ft

Worksheet for Channel Section 7 (25 cfs - 0.5% Slope)

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.035
Channel Slope	0.00500 ft/ft
Left Side Slope	2.50 ft/ft (H:V)
Right Side Slope	2.50 ft/ft (H:V)
Bottom Width	10.00 ft
Discharge	25.00 ft ³ /s

Results

Normal Depth	0.86 ft
Flow Area	10.43 ft ²
Wetted Perimeter	14.62 ft
Top Width	14.29 ft
Critical Depth	0.55 ft
Critical Slope	0.02311 ft/ft
Velocity	2.40 ft/s
Velocity Head	0.09 ft
Specific Energy	0.95 ft
Froude Number	0.49
Flow Type	Subcritical

GVF Input Data

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.00 ft
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.86 ft
Critical Depth	0.55 ft
Channel Slope	0.00500 ft/ft
Critical Slope	0.02311 ft/ft

**OFF-SITE CHANNEL
RIP-RAP SIZING CALCULATIONS**

Cholla Ash Monofill
Riprap Size Comparison
Offsite Channel Drop Structures

Cholla Ash Monofill Drainage Channel Riprap Rock Size (feet)											
Offsite Channel	Method							Min	Max	Chosen Rock Size (D50)	Layer Thickness (ft)
	USACE (D30)	ASCE	USBR	USGS	ISBASH	HEC-11	Maricopa Cty				
Section 1 (bank)	0.04	0.06	0.11	0.13	0.11	0.02	0.04	0.02	0.13	0.33	1.00
Section 1 (bottom)	0.04	0.06	0.11	0.13	0.11	0.02		0.02	0.13	0.33	1.00
Section 2 (bank)	0.10	0.14	0.26	0.38	0.25	0.05	0.10	0.05	0.38	0.33	1.00
Section 2 (bottom)	0.10	0.14	0.26	0.38	0.25	0.05		0.05	0.38	0.33	1.00
Section 3 (bank)	0.06	0.10	0.20	0.27	0.19	0.03	0.06	0.03	0.27	0.33	1.00
Section 3 (bottom)	0.06	0.10	0.20	0.27	0.19	0.03		0.03	0.27	0.33	1.00
Section 4 (bank)	0.14	0.20	0.38	0.59	0.36	0.09	0.14	0.09	0.59	0.50	1.00
Section 4 (bottom)	0.14	0.20	0.38	0.59	0.36	0.09		0.09	0.59	0.50	1.00
Section 5 (bank)	0.09	0.14	0.26	0.37	0.25	0.05	0.10	0.05	0.37	0.33	1.00
Section 5 (bottom)	0.09	0.04	0.26	0.37	0.25	0.05		0.04	0.37	0.33	1.00
Section 6 (bank)	0.09	0.14	0.26	0.37	0.25	0.05	0.10	0.05	0.37	0.33	1.00
Section 6 (bottom)	0.09	0.04	0.26	0.37	0.25	0.05		0.04	0.37	0.33	1.00
Section 7 (bank)	0.02	0.04	0.07	0.08	0.07	0.01	0.02	0.01	0.08	0.33	1.00
Section 7 (bottom)	0.02	0.04	0.07	0.08	0.07	0.01		0.01	0.08	0.33	1.00
Drop Struc Sec-6-5:1 (bnk)	1.42	1.03	2.06	4.34	1.86	1.03	3.07	1.03	4.34	1.00*	2.00
Drop Struc Sec-6-5:1 (btm)	1.42	0.95	2.06	4.34	1.86	0.77		0.77	4.34	1.00*	2.00
Drop Struc Sec-1-basin (bnk)	0.60	0.45	0.88	1.58	0.81	0.30	1.33	0.30	1.58	1.00*	2.00
Drop Struc Sec-1-basin (btm)	0.60	0.42	0.88	1.58	0.81	0.22		0.22	1.58	1.00*	2.00

*NOTE: All drop structures and basins will consist of grouted riprap.

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash Offsite Channel section 1 Drop Structure										
5	Design Flood Freque 100 -yr										
6	Location/Station: 1+50 to 7+50										
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ =	Median diameter of the riprap materials, ft									
21	V _a =	Average velocity in the main channel, ft/s									
22	d _{avg} =	Average depth of flow in the main channel, ft									
23	K ₁ =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	φ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	7.96	ft/s								
32	d _{avg} =	0.29	ft								
33	D ₅₀ =	16	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence.										
38	K ₁ =	0.82									
39	d ₅₀ =	1.25	ft								
40	d ₅₀ (inch) =	16	inch <----D50 = 16 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d₅₀) = 16 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash Offsite Channel section 1										
5	Design Flood Freque 100 -yr										
6	Location/Station: 1+50 to 7+50										
7											
8											
9	<u>Relevant Equations</u>										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ =	Median diameter of the riprap materials, ft									
21	V _a =	Average velocity in the main channel, ft/s									
22	d _{avg} =	Average depth of flow in the main channel, ft									
23	K ₁ =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	φ =	Riprap material's angle of repose, degree									
26											
27											
28	<u>Input Parameters</u>										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	2.90	ft/s								
32	d _{avg} =	0.73	ft								
33	D ₅₀ =	6	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	0.04	ft								
40	d ₅₀ (inch) =	1	<----D50 = 6 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 6 inch										

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name:	Cholla Ash Offsite Channel section 2									
5	Design Flood Freque	100 -yr									
6	Location/Station:	8+00 to 11+00									
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d_{50} =	Median diameter of the riprap materials, ft									
21	V_a =	Average velocity in the main channel, ft/s									
22	d_{avg} =	Average depth of flow in the main channel, ft									
23	K_1 =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	ϕ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V_a =	4.45	ft/s								
32	d_{avg} =	1.35	ft								
33	D_{50} =	6	inch Assume a D_{50} and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	ϕ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K_1 =	0.82									
39	d_{50} =	0.10	ft								
40	d_{50} (inch) =	2	<---- D_{50} = 6 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d_{50}) = 6 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash Offsite Channel section 3										
5	Design Flood Freque 100 -yr										
6	Location/Station: 11+50 to 13+75										
7											
8											
9	<u>Relevant Equations</u>										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ = Median diameter of the riprap materials, ft										
21	V _a = Average velocity in the main channel, ft/s										
22	d _{avg} = Average depth of flow in the main channel, ft										
23	K ₁ = Bank angle correction factor										
24	θ = Bank angle with the horizontal, degree										
25	φ = Riprap material's angle of repose, degree										
26											
27											
28	<u>Input Parameters</u>										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	3.85	ft/s								
32	d _{avg} =	1.85	ft								
33	D ₅₀ =	6	inch	Assume a D ₅₀ and then calculate if it is stable.							
34	θ =	21.80	degree	[2.5:1 (H:V)]							
35	φ =	41.0	degree	From Figure 6.14 of the Manual for rounded riprap - attached.							
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	0.06	ft								
40	d ₅₀ (inch) =	1	inch	<----D50 = 6 inches is stable.							
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 6 inch										

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name:	Cholla Ash Offsite Channel section 4									
5	Design Flood Freque	100 -yr									
6	Location/Station:	13+75 to 23+50									
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where.										
20	d_{50} =	Median diameter of the riprap materials, ft									
21	V_a =	Average velocity in the main channel, ft/s									
22	d_{avg} =	Average depth of flow in the main channel, ft									
23	K_1 =	Bank angle correction factor									
24	θ =	Bank angle with the horizontal, degree									
25	ϕ =	Riprap material's angle of repose, degree									
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V_a =	5.30	ft/s								
32	d_{avg} =	1.91	ft								
33	D_{50} =	6	inch Assume a D_{50} and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	ϕ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K_1 =	0.82									
39	d_{50} =	0.14	ft								
40	d_{50} (inch) =	2	<---- D_{50} = 6 inches is stable.								
41											
42											
43	Therefore, proposed design riprap size (d_{50}) = 6 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash Offsite Channel section 5										
5	Design Flood Freque 100 -yr										
6	Location/Station: 23+50 to 32+50										
7											
8											
9	<u>Relevant Equations</u>										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ = Median diameter of the riprap materials, ft										
21	V _a = Average velocity in the main channel, ft/s										
22	d _{avg} = Average depth of flow in the main channel, ft										
23	K ₁ = Bank angle correction factor										
24	θ = Bank angle with the horizontal, degree										
25	φ = Riprap material's angle of repose, degree										
26											
27											
28	<u>Input Parameters</u>										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	4.39	ft/s								
32	d _{avg} =	1.36	ft								
33	D ₅₀ =	6	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	0.10 ft									
40	d ₅₀ (inch) =	2 inch		<----D50 = 6 inches is stable.							
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 6 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

1	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash Offsite Channel section 6										
5	Design Flood Freque 100 -yr										
6	Location/Station: 33+00 to 35+00										
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ = Median diameter of the riprap materials, ft										
21	V _a = Average velocity in the main channel, ft/s										
22	d _{avg} = Average depth of flow in the main channel, ft										
23	K ₁ = Bank angle correction factor										
24	θ = Bank angle with the horizontal, degree										
25	φ = Riprap material's angle of repose, degree										
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	4.39 ft/s									
32	d _{avg} =	1.36 ft									
33	D ₅₀ =	6 inch Assume a D ₅₀ and then calculate if it is stable.									
34	θ =	21.80 degree [2.5:1 (H:V)]									
35	φ =	41.0 degree From Figure 6.14 of the Manual for rounded riprap - attached.									
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	0.10 ft									
40	d ₅₀ (inch) =	2 inch <----D50 = 6 inches is stable.									
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 6 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K
1	Calculation of Riprap Size for Channel Lining										
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)										
3											
4	Channel Name: Cholla Ash Offsite Drop Structure section 6										
5	Design Flood Freque 100 -yr										
6	Location/Station: 32+50 to 33+00										
7											
8											
9	Relevant Equations										
10											
11											
12	$d_{50} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$										
13											
14											
15											
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$										
17											
18											
19	Where,										
20	d ₅₀ = Median diameter of the riprap materials, ft										
21	V _a = Average velocity in the main channel, ft/s										
22	d _{avg} = Average depth of flow in the main channel, ft										
23	K ₁ = Bank angle correction factor										
24	θ = Bank angle with the horizontal, degree										
25	φ = Riprap material's angle of repose, degree										
26											
27											
28	Input Parameters										
29	(Based on output from FlowMaster and based on the Manual)										
30											
31	V _a =	12.05	ft/s								
32	d _{avg} =	0.58	ft								
33	D ₅₀ =	37	inch Assume a D ₅₀ and then calculate if it is stable.								
34	θ =	21.80	degree [2.5:1 (H:V)]								
35	φ =	41.0	degree From Figure 6.14 of the Manual for rounded riprap - attached.								
36											
37	Hence,										
38	K ₁ =	0.82									
39	d ₅₀ =	3.07 ft									
40	d ₅₀ (inch) =	37 inch <----D50 = 37 inches is stable.									
41											
42											
43	Therefore, proposed design riprap size (d ₅₀) = 37 inch										

Cholla Ash Monofill
Riprap Calculation

By: _____
Checked: _____

	A	B	C	D	E	F	G	H	I	J	K		
1	Calculation of Riprap Size for Channel Lining												
2	Calculations are based on Drainage Design Manual for Maricopa County (Manual)												
3													
4	Channel Name: Cholla Ash Offsite Channel section 7												
5	Design Flood Freque 100 -yr												
6	Location/Station: 35+00 to 38+50												
7													
8													
9	Relevant Equations												
10													
11													
12	$d_{s0} = \frac{0.001 V_a^3}{d_{avg}^{0.5} K_1^{1.5}}$												
13													
14													
15													
16	$K_1 = \left[1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{0.5}$												
17													
18													
19	Where,												
20	d ₅₀ = Median diameter of the riprap materials, ft												
21	V _a = Average velocity in the main channel, ft/s												
22	d _{avg} = Average depth of flow in the main channel, ft												
23	K ₁ = Bank angle correction factor												
24	θ = Bank angle with the horizontal, degree												
25	φ = Riprap material's angle of repose, degree												
26													
27													
28	Input Parameters												
29	(Based on output from FlowMaster and based on the Manual)												
30													
31	V _a = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">2.40</td><td>ft/s</td></tr></table>											2.40	ft/s
2.40	ft/s												
32	d _{avg} = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">0.86</td><td>ft</td></tr></table>											0.86	ft
0.86	ft												
33	D ₅₀ = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">6</td><td>inch</td></tr></table> Assume a D ₅₀ and then calculate if it is stable.											6	inch
6	inch												
34	θ = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">21.80</td><td>degree</td></tr></table> [2.5:1 (H:V)]											21.80	degree
21.80	degree												
35	φ = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">41.0</td><td>degree</td></tr></table> From Figure 6.14 of the Manual for rounded riprap - attached.											41.0	degree
41.0	degree												
36													
37	Hence,												
38	K ₁ = 0.82												
39	d ₅₀ = 0.02 ft												
40	d ₅₀ (inch) = 1 inch <----D50 = 6 inches is stable.												
41													
42													
43	Therefore, proposed design riprap size (d ₅₀) = <table border="1" style="display: inline-table; vertical-align: middle;"><tr><td style="width: 50px;">6</td><td>inch</td></tr></table>											6	inch
6	inch												

Drop Structure Offsite Sec-1-basin(0.2%slope)

Date: 02/12/2009 Time: 10:02

```

*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
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* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX: 858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB: WWW.WESTCONSULTANTS.COM *
*****
    
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 1 Drop

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	7.96 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	0.29 ft
Cotangent of Side Slope	2.50
Safety Factor	1.1
Riprap Placement	Channel Bank
Rock Type	Angular

Output Results:

Computed D30	0.60 ft
Computed Local Depth Averaged velocity	7.96 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	1.250 ft
Selected Minimum D30	0.61 ft
Selected Minimum D90	0.88 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	67.	169.
W50	34.	50.

Drop Structure offsite sec-1-basin(0.2% slope)

w15

11.

25.

ASCE Method

Input Parameters:

Local velocity 7.96 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.45 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 7.96 ft/s

Output Results:

Computed D50 0.88 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 1.500 ft
 Selected Minimum D30 0.73 ft
 Selected Minimum D90 1.06 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	117.	292.
w50	58.	86.
w15	18.	43.

Drop Structure Offsite Sec-1-basin(0.2% slope)
USGS Method

Input Parameters:

Average Channel Velocity 7.96 ft/s

Output Results:

Computed D50 1.58 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 2.750 ft
Selected Minimum D30 1.34 ft
Selected Minimum D90 1.94 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	719.	1797.
w50	359.	532.
w15	112.	266.

Isbash Method

Input Parameters:

Average Channel Velocity 7.96 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.81 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 1.500 ft
Selected Minimum D30 0.73 ft
Selected Minimum D90 1.06 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	117.	292.
w50	58.	86.
w15	18.	43.

Cal B & SP Method

Input Parameters:

Drop Structure Offsite Sec-1-basin(0.2%slope)

 Average Channel Velocity 7.96 ft/s
 Velocity Affecting Bank 10.61 ft/s
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

 Output Results:

Computed w 40.71 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than Rock Size (Ton)

0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

 Input Parameters:

Average Channel Velocity 7.96 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.

Drop Structure Offsite Sec-1-basin(0.2% slope)
 Riprap Placement Channel Bank
 Safety Factor 1.1

Output Results:

Computed D50 0.30 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Drop Structure Offsite Sec-1-basin_btm(0.2% slope)
 W15 11. 25.

ASCE Method

Input Parameters:

Local Velocity 7.96 ft/s
 Cotangent of Side slope N/A
 Unit weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bottom

Output Results:

Computed D50 0.42 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 7.96 ft/s

Output Results:

Computed D50 0.88 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 1.500 ft
 Selected Minimum D30 0.73 ft
 Selected Minimum D90 1.06 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	117.	292.
W50	58.	86.
W15	18.	43.

Drop structure Offsite Sec-1-basin_btm(0.2%slope)
 _____ USGS Method _____

Input Parameters:

Average Channel velocity 7.96 ft/s

Output Results:

Computed D50 1.58 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 2.750 ft
 Selected Minimum D30 1.34 ft
 Selected Minimum D90 1.94 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	719.	1797.
w50	359.	532.
w15	112.	266.

_____ Isbash Method _____

Input Parameters:

Average Channel velocity 7.96 ft/s
 Unit weight of Stone 165. lbs/cu ft
 Turbulence Level High

Output Results:

Computed D50 0.81 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 1.500 ft
 Selected Minimum D30 0.73 ft
 Selected Minimum D90 1.06 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	117.	292.
w50	58.	86.
w15	18.	43.

_____ Cal B & SP Method _____

Input Parameters:

Drop Structure Offsite Sec-1-basin_btm(0.2%slope)

Average Channel Velocity 7.96 ft/s
Velocity Affecting Bank 10.61 ft/s
Unit weight of Stone 165. lbs/cu ft
Cotangent of side slope 2.50
Flow Type Impinging

Output Results:

Computed W 40.71 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
Layer Thickness 3.40 ft

Percent Larger than Rock Size (Ton)

0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 7.96 ft/s
Average Flow Depth 4.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side Slope N/A
Material Angle of Repose deg.

Drop Structure Offsite Sec-1-basin_btm(0.2%slope)
 Riprap Placement Channel Bottom
 Safety Factor 1.1

Output Results:

Computed D50 0.22 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Offsite Channel Sec-1

Date: 02/06/2009 Time: 16:25

```
*****
*                               *
*             RIPRAP DESIGN SYSTEM (RDS)             *
*                               *
*                   BY                               *
*             WEST Consultants, Inc.                 *
*                               *
*                               *
* Version 3.0                                       March, 2005 *
*                               *
*                               *
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* 16870 WEST BERNARDO DRIVE                         PH: 858-487-9378 *
* SUITE 340                                         FAX: 858-487-9448 *
* SAN DIEGO, CA 92127                             WEB: WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 1

_____ USACE Method _____

Input Parameters:

```
-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    2.90 ft/s
Bottom width                N/A
Bend Radius                 N/A
Top width                   N/A
Unit weight of Stone       165. lbs/cu ft
Riprap Layer Thickness     1.00
Local Flow Depth           0.73 ft
Cotangent of Side Slope   2.50
Safety Factor              1.1
Riprap Placement           Channel Bank
Rock Type                   Angular
```

Output Results:

```
-----
Computed D30                0.04 ft
Computed Local Depth Averaged Velocity 2.90 ft/s
Local Velocity/Avg. Velocity 1.00
Side Slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
```

*** Using Gradations from COE ETL 1110-2-120 ***

```
Specific weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.

Offsite Channel Sec-1
2.

w15

5.

ASCE Method

Input Parameters:

Local Velocity 2.90 ft/s
Cotangent of Side slope 2.50
Unit weight of Stone 165. lbs/cu ft
Riprap Placement Channel Bank

Output Results:

Computed D50 0.06 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 2.90 ft/s

Output Results:

Computed D50 0.11 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Offsite Channel Sec-1
USGS Method

Input Parameters:

Average Channel Velocity 2.90 ft/s

Output Results:

Computed D50 0.13 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 2.90 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.11 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

offsite Channel Sec-1

Average Channel Velocity 2.90 ft/s
Velocity Affecting Bank 3.87 ft/s
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Flow Type Impinging

Output Results:

Computed W 0.10 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
Layer Thickness 3.40 ft

Percent Larger than Rock Size (Ton)

0 - 5 1.00
50 - 100 0.50
95 - 100 0.25

(2) Inner Layer:

Gradation Class None
Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel velocity 2.90 ft/s
Average Flow Depth 3.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side Slope 2.50
Material Angle of Repose 41.00 deg.

offsite channel sec-1

Riprap Placement
Safety Factor

Channel Bank
1.1

Output Results:

Computed D50

0.02 ft

** FHWA Gradation**

Gradation Class
Layer Thickness

Facing
1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Date: 02/08/2009 Time: 12:42

```

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*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
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* SUITE 340                                               FAX: 858-487-9448 *
* SAN DIEGO, CA 92127   WEB:WWW.WESTCONSULTANTS.COM     *
*****

```

Project: Cholla Ash Offsite
 Description: Offsite Channel - 10 ft bottom width *Sect. 1 - btm*

_____ USACE Method _____

Input Parameters:

```

-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    2.90 ft/s
Bottom width                 N/A
Bend Radius                  N/A
Top Width                    N/A
Unit Weight of Stone        165. lbs/cu ft
Riprap Layer Thickness      1.00
Local Flow Depth            0.73 ft
Cotangent of Side Slope     2.50
Safety Factor                1.1
Riprap Placement            Channel Bank
Rock Type                    Angular

```

Output Results:

```

-----
Computed D30                  0.04 ft
Computed Local Depth Averaged Velocity 2.90 ft/s
Local Velocity/Avg. Velocity  1.00
Side Slope Correction Factor   1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00

```

*** Using Gradations from COE ETL 1110-2-120 ***

```

Specific Weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

```

Percent Lighter by Weight	Stone Weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

ASCE Method

Input Parameters:

Local Velocity	2.90 ft/s
Cotangent of Side slope	2.50
Unit Weight of Stone	165. lbs/cu ft
Riprap Placement	Channel Bank

Output Results:

Computed D50 0.06 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by Weight	Stone Weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

USB Method

Input Parameters:

Average Channel Velocity	2.90 ft/s
--------------------------	-----------

Output Results:

Computed D50 0.11 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft

Specific Weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by Weight	Stone Weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

Cal B & SP Method

Input Parameters:

 Average Channel Velocity 2.90 ft/s
 Velocity Affecting Bank 3.87 ft/s
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

 Computed W 0.10 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1

Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

HEC-11 Method

Input Parameters:

Average Channel Velocity 2.90 ft/s
Average Flow Depth 3.00 ft
Unit Weight of Stone 165. lbs/cu ft
Cotangent of Side Slope 2.50
Material Angle of Repose 41.00 deg.
Riprap Placement Channel Bank
Safety Factor 1.1

Output Results:

Computed D50 0.02 ft

** FHWA Gradation**

Gradation Class Facing
Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock Weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Offsite Channel Sec-2

Date: 02/06/2009 Time: 16:26

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
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* WEST CONSULTANTS, INC.                                 *
* 16870 WEST BERNARDO DRIVE                             PH: 858-487-9378 *
* SUITE 340                                             FAX: 858-487-9448 *
* SAN DIEGO, CA 92127                                  WEB: WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 2

USACE Method

Input Parameters:

```
-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    4.45 ft/s
Bottom width                N/A
Bend Radius                 N/A
Top width                   N/A
Unit weight of Stone        165. lbs/cu ft
Riprap Layer Thickness      1.00
Local Flow Depth           1.35 ft
Cotangent of Side Slope    2.50
Safety Factor               1.1
Riprap Placement           Channel Bank
Rock Type                   Angular
```

Output Results:

```
-----
Computed D30                0.10 ft
Computed Local Depth Averaged velocity 4.45 ft/s
Local Velocity/Avg. velocity 1.00
Side Slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
```

*** Using Gradations from COE ETL 1110-2-120 ***

```
Specific weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30    0.37 ft
Selected Minimum D90    0.53 ft
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

offsite Channel sec-2

w15 2. 5.

ASCE Method

Input Parameters:

Local Velocity 4.45 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.14 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 4.45 ft/s

Output Results:

Computed D50 0.26 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Offsite Channel Sec-2
USGS Method

Input Parameters:

Average Channel Velocity 4.45 ft/s

Output Results:

Computed D50 0.38 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 4.45 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.25 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

Cal B & SP Method

Input Parameters:

offsite channel sec-2

 Average Channel Velocity 4.45 ft/s
 Velocity Affecting Bank 5.93 ft/s
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

Computed W 1.24 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 4.45 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.

Offsite Channel Sec-2

Riprap Placement
Safety Factor

Channel Bank
1.1

Output Results:

Computed D50

0.05 ft

** FHWA Gradation**

Gradation Class
Layer Thickness

Facing
1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

offsite channel sec-2_btm

Date: 02/06/2009 Time: 17:01

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
* COPYRIGHT (c) 2005                                     *
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* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127   WEB:WWW.WESTCONSULTANTS.COM     *
*****
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 2 btm

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	4.45 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	1.35 ft
Cotangent of Side slope	2.50
Safety Factor	1.1
Riprap Placement	Channel Bank
Rock Type	Angular

Output Results:

Computed D30	0.10 ft
Computed Local Depth Averaged Velocity	4.45 ft/s
Local Velocity/Avg. Velocity	1.00
Side slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.

Offsite Channel Sec-2_btm

w15 2. 5.

ASCE Method

Input Parameters:

Local velocity 4.45 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.14 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USB Method

Input Parameters:

Average Channel velocity 4.45 ft/s

Output Results:

Computed D50 0.26 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Offsite Channel Sec-2_btm
USGS Method

Input Parameters:

Average Channel Velocity 4.45 ft/s

Output Results:

Computed D50 0.38 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 4.45 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.25 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

Offsite Channel Sec-2_btm

 Average Channel Velocity 4.45 ft/s
 Velocity Affecting Bank 5.93 ft/s
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

Computed W 1.24 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel velocity 4.45 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.

Offsite Channel Sec-2_btm
 Riprap Placement Channel Bank
 Safety Factor 1.1

Output Results:

Computed D50 0.05 ft

** FHWA Gradation**

Gradation Class Facing
 Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Offsite Channel Sec-3

Date: 02/06/2009 Time: 16:27

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                      *
*                *                                          *
* Version 3.0                                             March, 2005 *
*                *                                          *
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* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB:WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 3

USACE Method

Input Parameters:

```
-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    3.85 ft/s
Bottom width                 N/A
Bend Radius                  N/A
Top width                    N/A
Unit weight of Stone        165. lbs/cu ft
Riprap Layer Thickness      1.00
Local Flow Depth            1.85 ft
Cotangent of Side Slope    2.50
Safety Factor                1.1
Riprap Placement            Channel Bank
Rock Type                    Angular
```

Output Results:

```
-----
Computed D30                0.06 ft
Computed Local Depth Averaged Velocity 3.85 ft/s
Local Velocity/Avg. Velocity 1.00
Side Slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
```

*** Using Gradations from COE ETL 1110-2-120 ***

```
Specific weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft
```

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.

Offsite Channel Sec-3
2.

w15

5.

ASCE Method

Input Parameters:

Local Velocity 3.85 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.10 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 3.85 ft/s

Output Results:

Computed D50 0.20 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Offsite Channel Sec-3
USGS Method

Input Parameters:

Average Channel Velocity 3.85 ft/s

Output Results:

Computed D50 0.27 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 3.85 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.19 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

offsite Channel Sec-3

 Average Channel Velocity 3.85 ft/s
 Velocity Affecting Bank 5.13 ft/s
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

Computed W 0.52 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 3.85 ft/s
 Average Flow Depth 4.00 ft
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.

Offsite Channel Sec-3

Riprap Placement
Safety Factor

Channel Bank
1.1

Output Results:

Computed D50

0.03 ft

** FHWA Gradation**

Gradation Class
Layer Thickness

Facing
1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

offsite channel Sec-3_btm

Date: 02/06/2009 Time: 17:00

```

*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                                                         *
* Version 3.0                                           March, 2005 *
*                                                         *
* COPYRIGHT (c) 2005                                     *
* WEST CONSULTANTS, INC.                                 *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB:WWW.WESTCONSULTANTS.COM *
*****
    
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 3 btm

USACE Method

Input Parameters:

```

-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    3.85 ft/s
Bottom width                N/A
Bend Radius                 N/A
Top width                   N/A
Unit weight of Stone       165. lbs/cu ft
Riprap Layer Thickness     1.00
Local Flow Depth           1.85 ft
Cotangent of Side Slope   2.50
Safety Factor              1.1
Riprap Placement          Channel Bank
Rock Type                 Angular
    
```

Output Results:

```

-----
Computed D30                0.06 ft
Computed Local Depth Averaged Velocity 3.85 ft/s
Local Velocity/Avg. Velocity 1.00
Side Slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
    
```

*** Using Gradations from COE ETL 1110-2-120 ***

```

Specific weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft
    
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.

Offsite Channel Sec-3_btm
USGS Method

Input Parameters:

Average Channel Velocity 3.85 ft/s

Output Results:

Computed D50 0.27 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 3.85 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.19 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

Offsite Channel Sec-3_btm

 Average Channel Velocity 3.85 ft/s
 Velocity Affecting Bank 5.13 ft/s
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

Computed W 0.52 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 3.85 ft/s
 Average Flow Depth 4.00 ft
 Unit Weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.

Offsite Channel Sec-4

Date: 02/06/2009 Time: 16:31

```
*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                    *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
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* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                WEB:WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 4

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	5.30 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	1.91 ft
Cotangent of Side slope	2.50
Safety Factor	1.1
Riprap Placement	Channel Bank
Rock Type	Angular

Output Results:

Computed D30	0.14 ft
Computed Local Depth Averaged Velocity	5.30 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.

Offsite Channel Sec-4
2.

w15

5.

ASCE Method

Input Parameters:

Local Velocity 5.30 ft/s
Cotangent of Side slope 2.50
Unit weight of Stone 165. lbs/cu ft
Riprap Placement Channel Bank

Output Results:

Computed D50 0.20 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel velocity 5.30 ft/s

Output Results:

Computed D50 0.38 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Offsite Channel Sec-4
USGS Method

Input Parameters:

Average Channel Velocity 5.30 ft/s

Output Results:

Computed D50 0.59 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight 165.0 lbs/cu ft
Layer Thickness 1.000 ft
Selected Minimum D30 0.49 ft
Selected Minimum D90 0.70 ft

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
W100	35.	86.
W50	17.	26.
W15	5.	13.

Isbash Method

Input Parameters:

Average Channel Velocity 5.30 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.36 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.
W15	2.	5.

Cal B & SP Method

Input Parameters:

offsite Channel Sec-4

Average Channel Velocity 5.30 ft/s
Velocity Affecting Bank 7.07 ft/s
Unit Weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Flow Type Impinging

Output Results:

Computed w 3.55 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
Layer Thickness 3.40 ft

Percent Larger than Rock Size (Ton)

0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 5.30 ft/s
Average Flow Depth 4.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Material Angle of Repose 41.00 deg.

Offsite Channel Sec-4

Riprap Placement
Safety Factor

Channel Bank
1.1

Output Results:

Computed D50 0.09 ft

** FHWA Gradation**

Gradation Class Facing
Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

offsite channel sec-4_btm

Date: 02/06/2009 Time: 16:59

```

*****
*                               *
*       RIPRAP DESIGN SYSTEM (RDS)       *
*                               *
*                               *
*                               *
*                               *
*       Version 3.0                               *
*                               *
*                               *
*       March, 2005                               *
*                               *
*       COPYRIGHT (c) 2005                               *
*       WEST CONSULTANTS, INC.                               *
*       16870 WEST BERNARDO DRIVE                               *
*       SUITE 340                               *
*       SAN DIEGO, CA 92127                               *
*                               *
*       PH: 858-487-9378                               *
*       FAX:858-487-9448                               *
*       WEB:WWW.WESTCONSULTANTS.COM                               *
*****
    
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 4 btm

USACE Method

Input Parameters:

```

-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel velocity    5.30 ft/s
Bottom width                N/A
Bend Radius                 N/A
Top width                   N/A
Unit weight of Stone       165. lbs/cu ft
Riprap Layer Thickness     1.00
Local Flow Depth           1.91 ft
Cotangent of Side Slope    2.50
Safety Factor               1.1
Riprap Placement           Channel Bank
Rock Type                   Angular
    
```

Output Results:

```

-----
Computed D30                0.14 ft
Computed Local Depth Averaged Velocity 5.30 ft/s
Local Velocity/Avg. Velocity 1.00
Side slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
    
```

*** Using Gradations from COE ETL 1110-2-120 ***

```

Specific weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft
    
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.

Offsite Channel Sec-4_btm
USGS Method

Input Parameters:

Average Channel Velocity 5.30 ft/s

Output Results:

Computed D50 0.59 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 1.000 ft
Selected Minimum D30 0.49 ft
Selected Minimum D90 0.70 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	35.	86.
w50	17.	26.
w15	5.	13.

Isbash Method

Input Parameters:

Average Channel Velocity 5.30 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.36 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

Offsite Channel Sec-4_btm

Average Channel Velocity 5.30 ft/s
Velocity Affecting Bank 7.07 ft/s
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Flow Type Impinging

Output Results:

Computed w 3.55 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel velocity 5.30 ft/s
Average Flow Depth 4.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Material Angle of Repose 41.00 deg.

Riprap Placement
Safety Factor

Offsite Channel Sec-4_btm
Channel Bank
1.1

Output Results:

Computed D50 0.09 ft

** FHWA Gradation**

Gradation Class Facing
Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Offsite Channel Sec-5&6
2.

w15

5.

ASCE Method

Input Parameters:

Local Velocity 4.39 ft/s
 Cotangent of Side slope 2.50
 Unit weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.14 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USBR Method

Input Parameters:

Average Channel Velocity 4.39 ft/s

Output Results:

Computed D50 0.26 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Offsite Channel Sec-5&6
USGS Method

Input Parameters:

Average Channel Velocity 4.39 ft/s

Output Results:

Computed D50 0.37 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel velocity 4.39 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.25 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

offsite channel sec-5&6

Average Channel Velocity 4.39 ft/s
Velocity Affecting Bank 5.85 ft/s
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Flow Type Impinging

Output Results:

Computed W 1.15 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel velocity 4.39 ft/s
Average Flow Depth 4.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side Slope 2.50
Material Angle of Repose 41.00 deg.

Offsite Channel Sec-5&6

Riprap Placement
Safety Factor

Channel Bank
1.1

Output Results:

Computed D50

0.05 ft

** FHWA Gradation**

Gradation Class
Layer Thickness

Facing
1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Offsite Channel Sec-5&6_btm
USGS Method

Input Parameters:

Average Channel Velocity 4.39 ft/s

Output Results:

Computed D50 0.37 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel Velocity 4.39 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.25 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

Offsite Channel Sec-5&6_btm

 Average Channel Velocity 4.39 ft/s
 Velocity Affecting Bank 5.85 ft/s
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

Computed w 1.15 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 4.39 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.

Drop Structure Offsite Channel Sec-6(0.2% slope)

Date: 02/09/2009 Time: 17:30

```

*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                        *
*                WEST Consultants, Inc.                    *
*                *                                        *
* Version 3.0                                           March, 2005 *
*                *                                        *
* COPYRIGHT (c) 2005                                    *
* WEST CONSULTANTS, INC.                                *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB:WWW.WESTCONSULTANTS.COM *
*****
    
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 6 DROP

USACE Method

Input Parameters:

```

-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    12.05 ft/s
Bottom width                N/A
Bend Radius                 N/A
Top width                   N/A
Unit weight of Stone       165. lbs/cu ft
Riprap Layer Thickness     1.00
Local Flow Depth           0.58 ft
Cotangent of Side slope    2.50
Safety Factor               1.1
Riprap Placement           Channel Bank
Rock Type                   Angular
    
```

Output Results:

```

-----
Computed D30                1.42 ft
Computed Local Depth Averaged Velocity 12.05 ft/s
Local Velocity/Avg. Velocity 1.00
Side slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
    
```

*** Using Gradations from COE ETL 1110-2-120 ***

```

Specific weight    165.0 lbs/cu ft
Layer Thickness    3.000 ft
Selected Minimum D30 1.46 ft
Selected Minimum D90 2.11 ft
    
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
W100	933.	2333.
W50	467.	690.

Drop Structure Offsite channel sec-6(0.2% slope)

w15

146.

345.

ASCE Method

Input Parameters:

Local velocity 12.05 ft/s
 Cotangent of Side slope 2.50
 Unit weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 1.03 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 1.750 ft
 Selected Minimum D30 0.85 ft
 Selected Minimum D90 1.23 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	185.	463.
w50	93.	137.
w15	29.	69.

USBR Method

Input Parameters:

Average Channel velocity 12.05 ft/s

Output Results:

Computed D50 2.06 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 3.500 ft
 Selected Minimum D30 1.70 ft
 Selected Minimum D90 2.47 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	1482.	3704.
w50	741.	1096.
w15	232.	548.

Drop Structure Offsite Channel Sec-6(0.2% slope)
USGS Method

Input Parameters:

Average Channel Velocity 12.05 ft/s

Output Results:

Computed D50 4.34 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Warning: The required stone size is greater than the largest USACE stone gradation.

Isbash Method

Input Parameters:

Average Channel Velocity 12.05 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 1.86 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 3.500 ft
Selected Minimum D30 1.70 ft
Selected Minimum D90 2.47 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	1482.	3704.
w50	741.	1096.
w15	232.	548.

Cal B & SP Method

Input Parameters:

Average Channel Velocity 12.05 ft/s
Velocity Affecting Bank 16.07 ft/s
Unit weight of Stone 165. lbs/cu ft
Cotangent of side slope 2.50
Flow Type Impinging

Drop Structure Offsite Channel Sec-6(0.2%slope)

Output Results:

Computed w 489.90 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

_____ HEC-11 Method _____

Input Parameters:

Average Channel Velocity 12.05 ft/s
Average Flow Depth 4.00 ft
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side Slope 2.50
Material Angle of Repose 41.00 deg.
Riprap Placement Channel Bank
Safety Factor 1.1

Output Results:

Computed D50 1.03 ft

Drop Structure Offsite Channel Sec-6(0.2% slope)

** FHWA Gradation**

Gradation Class
Layer Thickness

Light
2.60 ft

Percent Smaller by Size	Rock Size, ft	Rock Weight, lbs
D100	1.80	500.
D50	1.30	200.
D10	0.40	5.

Drop Structure Offsite Channel Sec-6_btm(0.2%slope)

Date: 02/09/2009 Time: 17:31

```
*****
*                               *
*             RIPRAP DESIGN SYSTEM (RDS)             *
*                               *
*                   BY                               *
*             WEST Consultants, Inc.                 *
*                               *
*                               *
* Version 3.0                               March, 2005 *
*                               *
*                               *
* COPYRIGHT (c) 2005                               *
* WEST CONSULTANTS, INC.                           *
* 16870 WEST BERNARDO DRIVE                         PH: 858-487-9378 *
* SUITE 340                                         FAX: 858-487-9448 *
* SAN DIEGO, CA 92127                             WEB: WWW.WESTCONSULTANTS.COM *
*****
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - SECTION 6 DROP btm

_____ USACE Method _____

Input Parameters:

```
-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel Velocity    12.05 ft/s
Bottom width                N/A
Bend Radius                 N/A
Top width                   N/A
Unit weight of Stone        165. lbs/cu ft
Riprap Layer Thickness      1.00
Local Flow Depth            0.58 ft
Cotangent of Side Slope    N/A
Safety Factor               1.1
Riprap Placement           Channel Bottom
Rock Type                   Angular
```

Output Results:

```
-----
Computed D30                1.42 ft
Computed Local Depth Averaged Velocity 12.05 ft/s
Local Velocity/Avg. Velocity 1.00
Side Slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
```

*** Using Gradations from COE ETL 1110-2-120 ***

```
Specific weight    165.0 lbs/cu ft
Layer Thickness    3.000 ft
Selected Minimum D30 1.46 ft
Selected Minimum D90 2.11 ft
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	933.	2333.
w50	467.	690.

Drop Structure Offsite Channel Sec-6_btm(0.2% slope)

W15

146.

345.

ASCE Method

Input Parameters:

Local Velocity 12.05 ft/s
 Cotangent of Side slope N/A
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bottom

Output Results:

Computed D50 0.95 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight 165.0 lbs/cu ft
 Layer Thickness 1.750 ft
 Selected Minimum D30 0.85 ft
 Selected Minimum D90 1.23 ft

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
W100	185.	463.
W50	93.	137.
W15	29.	69.

USBR Method

Input Parameters:

Average Channel Velocity 12.05 ft/s

Output Results:

Computed D50 2.06 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight 165.0 lbs/cu ft
 Layer Thickness 3.500 ft
 Selected Minimum D30 1.70 ft
 Selected Minimum D90 2.47 ft

Percent Lighter by Weight	Stone weight, lbs	
	Minimum	Maximum
W100	1482.	3704.
W50	741.	1096.
W15	232.	548.

Drop Structure Offsite Channel Sec-6_btm(0.2% slope)
USGS Method

Input Parameters:

Average Channel Velocity 12.05 ft/s

Output Results:

Computed D50 4.34 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Warning: The required stone size is greater than the largest
USACE stone gradation.

Isbash Method

Input Parameters:

Average Channel Velocity 12.05 ft/s
Unit Weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 1.86 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 3.500 ft
Selected Minimum D30 1.70 ft
Selected Minimum D90 2.47 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	1482.	3704.
w50	741.	1096.
w15	232.	548.

Cal B & SP Method

Input Parameters:

Average Channel Velocity 12.05 ft/s
Velocity Affecting Bank 16.07 ft/s
Unit weight of Stone 165. lbs/cu ft
Cotangent of Side slope 2.50
Flow Type Impinging

Drop Structure Offsite Channel Sec-6_btm(0.2%slope)

Output Results:

Computed w 489.90 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

_____ HEC-11 Method _____

Input Parameters:

Average Channel velocity 12.05 ft/s
 Average Flow Depth 4.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope N/A
 Material Angle of Repose deg.
 Riprap Placement Channel Bottom
 Safety Factor 1.1

Output Results:

Computed D50 0.77 ft

Drop structure offsite channel sec-6_btm(0.2% slope)

** FHWA Gradation**

Gradation Class
Layer Thickness

Facing
1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock Weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Offsite Channel Sec-7

Date: 02/06/2009 Time: 16:32

```

*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                      *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
* COPYRIGHT (c) 2005                                     *
* WEST CONSULTANTS, INC.                                 *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127   WEB:WWW.WESTCONSULTANTS.COM      *
*****
    
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 7

_____ USACE Method _____

Input Parameters:

```

-----
Velocity Type                Average
Channel Shape                Trapezoidal
Channel Type                 Straight
Bend Angle (deg)            N/A
Average Channel velocity     2.40 ft/s
Bottom width                 N/A
Bend Radius                  N/A
Top width                    N/A
Unit weight of Stone        165. lbs/cu ft
Riprap Layer Thickness      1.00
Local Flow Depth            0.86 ft
Cotangent of Side slope     2.50
Safety Factor                1.1
Riprap Placement            Channel Bank
Rock Type                    Angular
    
```

Output Results:

```

-----
Computed D30                0.02 ft
Computed Local Depth Averaged velocity 2.40 ft/s
Local Velocity/Avg. velocity 1.00
Side slope Correction Factor 1.06
Correction for Layer Thickness 1.00
Correction for Secondary Currents 1.00
    
```

*** Using Gradations from COE ETL 1110-2-120 ***

```

Specific weight    165.0 lbs/cu ft
Layer Thickness    0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft
    
```

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.

offsite Channel Sec-7

w15

2.

5.

ASCE Method

Input Parameters:

Local velocity 2.40 ft/s
 Cotangent of Side slope 2.50
 Unit Weight of Stone 165. lbs/cu ft
 Riprap Placement Channel Bank

Output Results:

Computed D50 0.04 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

USB Method

Input Parameters:

Average Channel velocity 2.40 ft/s

Output Results:

Computed D50 0.07 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
 Layer Thickness 0.750 ft
 Selected Minimum D30 0.37 ft
 Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Offsite Channel Sec-7
USGS Method

Input Parameters:

Average Channel velocity 2.40 ft/s

Output Results:

Computed D50 0.08 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel velocity 2.40 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.07 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

Offsite Channel Sec-7

 Average Channel Velocity 2.40 ft/s
 Velocity Affecting Bank 3.20 ft/s
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side slope 2.50
 Flow Type Impinging

Output Results:

Computed W 0.03 lbs

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class 1/2 Ton
 Layer Thickness 3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class None
 Layer Thickness 0.00 ft

(3) Backing:

Backing Class No. 1
 Layer Thickness 1.8 ft

(4) Fabric:

Fabric Type B

Total Thickness (1)+(2)+(3)+(4): 5.2 ft

----- HEC-11 Method -----

Input Parameters:

Average Channel Velocity 2.40 ft/s
 Average Flow Depth 3.00 ft
 Unit weight of Stone 165. lbs/cu ft
 Cotangent of Side Slope 2.50
 Material Angle of Repose 41.00 deg.

Riprap Placement
Safety Factor

Offsite Channel Sec-7
Channel Bank
1.1

Output Results:

Computed D50 0.01 ft

** FHWA Gradation**

Gradation Class Facing
Layer Thickness 1.90 ft

Percent Smaller by Size	Rock Size, ft	Rock weight, lbs
D100	1.30	200.
D50	0.95	75.
D10	0.40	5.

Offsite Channel Sec-7_btm

Date: 02/06/2009 Time: 16:58

```

*****
*                RIPRAP DESIGN SYSTEM (RDS)                *
*                BY                                          *
*                WEST Consultants, Inc.                     *
*                *                                          *
*                *                                          *
* Version 3.0                                           March, 2005 *
*                *                                          *
*                *                                          *
* COPYRIGHT (c) 2005                                     *
* WEST CONSULTANTS, INC.                                 *
* 16870 WEST BERNARDO DRIVE                               PH: 858-487-9378 *
* SUITE 340                                               FAX:858-487-9448 *
* SAN DIEGO, CA 92127                                   WEB:WWW.WESTCONSULTANTS.COM *
*****
    
```

Project: Cholla Ash Offsite
 Description: Offsite Channel - Section 7 btm

USACE Method

Input Parameters:

Velocity Type	Average
Channel Shape	Trapezoidal
Channel Type	Straight
Bend Angle (deg)	N/A
Average Channel Velocity	2.40 ft/s
Bottom width	N/A
Bend Radius	N/A
Top width	N/A
Unit Weight of Stone	165. lbs/cu ft
Riprap Layer Thickness	1.00
Local Flow Depth	0.86 ft
Cotangent of Side Slope	N/A
Safety Factor	1.1
Riprap Placement	Channel Bottom
Rock Type	Angular

Output Results:

Computed D30	0.02 ft
Computed Local Depth Averaged Velocity	2.40 ft/s
Local Velocity/Avg. Velocity	1.00
Side Slope Correction Factor	1.06
Correction for Layer Thickness	1.00
Correction for Secondary Currents	1.00

*** Using Gradations from COE ETL 1110-2-120 ***

Specific Weight	165.0 lbs/cu ft
Layer Thickness	0.750 ft
Selected Minimum D30	0.37 ft
Selected Minimum D90	0.53 ft

Percent Lighter by Weight	stone weight, lbs	
	Minimum	Maximum
W100	15.	36.
W50	7.	11.

Offsite Channel Sec-7_btm
USGS Method

Input Parameters:

Average Channel velocity 2.40 ft/s

Output Results:

Computed D50 0.08 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Isbash Method

Input Parameters:

Average Channel velocity 2.40 ft/s
Unit weight of Stone 165. lbs/cu ft
Turbulence Level High

Output Results:

Computed D50 0.07 ft

*** Using Gradations from COE ETL 1110-2-120 ***

Specific weight 165.0 lbs/cu ft
Layer Thickness 0.750 ft
Selected Minimum D30 0.37 ft
Selected Minimum D90 0.53 ft

Percent Lighter by weight	Stone weight, lbs	
	Minimum	Maximum
w100	15.	36.
w50	7.	11.
w15	2.	5.

Cal B & SP Method

Input Parameters:

 offsite channel sec-7_btm

Average Channel Velocity	2.40 ft/s
Velocity Affecting Bank	3.20 ft/s
Unit Weight of Stone	165. lbs/cu ft
Cotangent of Side slope	2.50
Flow Type	Impinging

 Output Results:

Computed W	0.03 lbs
------------	----------

** CalTrans A Gradation **

(1) Outside Layer:

Gradation Class	1/2 Ton
Layer Thickness	3.40 ft

Percent Larger than	Rock Size (Ton)
0 - 5	1.00
50 - 100	0.50
95 - 100	0.25

(2) Inner Layer:

Gradation Class	None
Layer Thickness	0.00 ft

(3) Backing:

Backing Class No.	1
Layer Thickness	1.8 ft

(4) Fabric:

Fabric Type	B
Total Thickness (1)+(2)+(3)+(4):	5.2 ft

----- HEC-11 Method -----

 Input Parameters:

Average Channel velocity	2.40 ft/s
Average Flow Depth	3.00 ft
Unit Weight of Stone	165. lbs/cu ft
Cotangent of Side Slope	N/A
Material Angle of Repose	deg.

**OFF-SITE CHANNEL
DROP STRUCTURE CALCULATIONS**

Cholla Ash Monofill
Riprap Basin Sizing and Hydraulic Jump
Offsite Drop Structure Channel Section 1 Into Basin

	A	B	C	D	E	F	G	H	I	J
1	Riprap Basin Sizing									
2										
3	Q=	25 cfs	flow rate for section 1 just before brink of drop							
4	Vallow=	0.001 ft/s	velocity for basin, past drop							
5	TW=	10 ft	normal depth for basin, past drop							
6	Wo=	10 ft	channel bottom width							
7										
8	From FHWA Hydraulic Design of Energy Dissipators for Culverts and Channels, July 2006									
9	1) Get intial velocity (Vo), depth (yo), and Froude Number (Fr) for brink conditions									
10										
11	From Flowmaster output files:									
12	yo= ye=	0.29 ft	normal depth for drop							
13	Vo=	7.96 ft/s	velocity for drop							
14	Fr=	2.68	Froude number for drop							
15										
16	2) Select trial D50 and obtain hs/ye from Equation 10.1.									
17										
18	Equation 10.1	$\frac{h_s}{y_e} = 0.86 \left(\frac{D_{50}}{y_e} \right)^{-0.55} \left(\frac{V_o}{\sqrt{g y_e}} \right) - C_o$								
19										
20										
21										
22	Get tailwater parameter Co:									
23	Co =	1.4 if TW/ye < 0.75								
24	Co =	136.331 if 0.75 < TW/ye < 1.0								
25	Co =	3 if 1.0 < TW/ye								
26										
27	TW/ye =	34.48276								
28										
29	D50 =	0.11 ft	D50 of riprap							
30										
31	Check to see that hs/D50 >= 2 and that D50/ye >= 0.1.									
32	D50/ye =	0.37931 >= 0.1	OK							
33	g =	32.2 ft/s ²								
34	hs/ye =	0.928133								
35	hs =	0.269159 ft	scour depth							
36	hs/D50 =	2.446896 >= 2.0	OK							
37										
38	3) Size the basin									
39	Ls =	10*hs =	2.69 ft	Dissipator Pool Length						
40	Lsmin =	3Wo =	30 ft							
41										
42	Lb =	15hs =	4.04 ft	Total Pool Length						
43	Lbmin =	4Wo =	40 ft							
44	La =	10 ft	Apron Length							
45	Wb =	10 ft	maintaining channel bottom depth							
46										

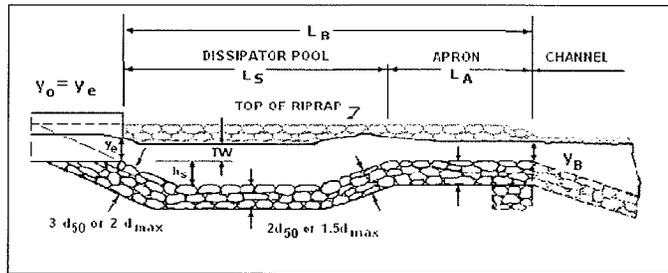


Figure 10.1. Profile of Riprap Basin

Cholla Ash Monofill
Riprap Basin Sizing and Hydraulic Jump
Offsite Drop Structure Channel Section 1 Into Basin

	A	B	C	D	E	F	G	H	I	J
47	Hydraulic Jump									
48	From Drainage Design Manual for Maricopa County, Hydraulics: Hydraulic Structures, 2003									
49	Y1 =	0.29 ft	upstream normal depth for drop							
50	Ydn =	10 ft	downstream normal depth basin							
51	Q =	25 cfs	flow through the drop							
52	g =	32.2 ft/s ²								
53	A1 =	3.14 ft ²	area of flow through the drop							
54	A2 =	10.43 ft ²	area of flow in next section							
55	z =	2.5 ft	sideslope H:1							
56	b =	10 ft	bottom width of channel							
57										
58	2) Calculate sequant height of jump.									
59	Equation 7.2									
60	$Y_2 = \frac{1}{2} Y_1 \left[\left(1 + 8F_{r1}^2 \right)^{\frac{1}{2}} - 1 \right]$									
61										
62										
63	Y2 =	0.96 ft	OK	height of jump						
64										
65	3) Another check on sequant height of jump.									
66	Use Fig. 7-8									
67										
68	Fr1 =	$\frac{V}{\sqrt{gy_m}}$	V =	7.96 ft/s						
69			top width =	11.46 ft						
70			ym =	0.27 ft	= flow area / top width					
71			Fr1 =	2.68						
72	J = Y2 / Y1		t = b/(zy1)							
73	J =	3.1	t =	13.79						
74	Y2 =	0.899 ft	use larger height of jump							
75										
76	1) Calculate depth at beginning location of jump.									
77	Equation 7.3									
78	$\frac{ZY_1^3}{3} + \frac{ZY_1^2}{2} + \frac{Q}{gA_1} = \frac{ZY_2^3}{3} + \frac{bY_2^2}{3} + \frac{Q}{gA_2}$									
79										
80										
81	Leq =	0.373	Req =	0.375	(Plug in values for Y2 until both sides equal)					
82	Yb =	0.29 ft	OK	depth at jump location						
83										
84	4) Calculate length of jump.									
85	Use Fig. 7-9									
86	Lj / y1 =	33								
87	Lj =	9.57 ft	= jump length							
88										
89	Therefore: Min. Length of jump =		30 ft							
90	Min. Length of apron =		10 ft							
91	Total Length of basin =		40 ft							

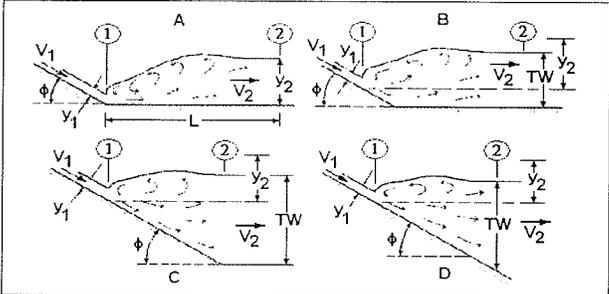


Figure 6.10. Hydraulic Jump Types Sloping Channels (Bradley, 1961)

Cholla Ash Monofill
Riprap Basin Sizing and Hydraulic Jump
Offsite Drop Structure Channel Section 6

	A	B	C	D	E	F	G	H	I	J
1	Riprap Basin Sizing									
2										
3	Q=	80 cfs	flow rate for section 6, just before brink of drop							
4	Vallow=	4.39 ft/s	velocity for section 5, past drop							
5	TW=	1.36 ft	normal depth for section 5, past drop							
6	Wo=	10 ft	channel bottom width							
7										
8	From FHWA Hydraulic Design of Energy Dissipators for Culverts and Channels, July 2006									
9	1) Get initial velocity (Vo), depth (yo), and Froude Number (Fr) for brink conditions									
10										
11	From Flowmaster output files:									
12	yo= ye=	0.58 ft	normal depth for drop							
13	Vo=	12.05 ft/s	velocity for drop							
14	Fr=	2.96	Froude number for drop							
15										
16	2) Select trial D50 and obtain hs/ye from Equation 10.1.									
17										
18	Equation 10.1	$\frac{h_s}{y_e} = 0.86 \left(\frac{D_{50}}{y_e} \right)^{-0.55} \left(\frac{V_o}{\sqrt{g y_e}} \right) - C_o$								
19										
20										
21										
22	Get tailwater parameter Co:									
23	Co =	1.4 if TW/ye < 0.75								
24	Co =	7.77931 if 0.75 < TW/ye < 1.0								
25	Co =	3 if 1.0 < TW/ye								
26										
27	TW/ye =	2.344828								
28										
29	D50 =	0.25 ft	D50 of riprap							
30										
31	Check to see that hs/D50 >= 2 and that D50/ye >= 0.1.									
32	D50/ye =	0.431034 >= 0.1	OK							
33	g =	32.2 ft/s ²								
34	hs/ye =	1.043976								
35	hs =	0.605506 ft	scour depth							
36	hs/D50 =	2.422024 >= 2.0	OK							
37										
38	3) Size the basin									
39	Ls =	10*hs =	6.06 ft	Dissipator Pool Length						
40	Lsmin =	3Wo =	30 ft							
41										
42	Lb =	15hs =	9.08 ft	Total Pool Length						
43	Lbmin =	4Wo =	40 ft							
44	La =	10 ft	Apron Length							
45	Wb =	10 ft	maintaining channel bottom depth							
46										
47	4) Assess need for downstream riprap due to TW/ye > 0.74									
48	Using Figure 10.3									
49	Compute equivalent circular diameter, De, for brink area									
50	A =	5.8	$A = \frac{\pi D_e^2}{4} = y_o W_o$							
51	De =	2.72								
52										
53	Rock size for riprap after energy dissipators - Equation 10.6									
54	S =	2.64	specific gravity of rock							
55										
56	L/De	L (ft)	VI/Vo (Fig. 10.3)	VI (ft/s)	D50 (ft)					
57	3.679857	10	0.95	11.4475	0.86					
58	7.359714	20	0.75	9.0375	0.53					
59	11.03957	30	0.5	6.025	0.24					
60	14.71943	40	0.4	4.82	0.15					
61	17.29533	47	0.36	4.338	0.12					

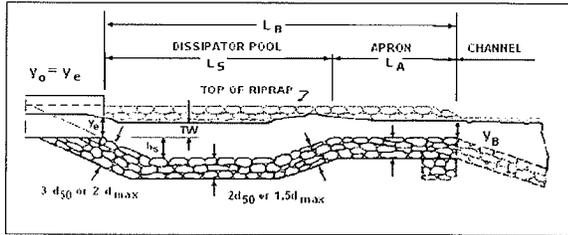


Figure 10.1. Profile of Riprap Basin

Riprap placement required for 47 ft from brink.

Cholla Ash Monofill
Riprap Basin Sizing and Hydraulic Jump
Offsite Drop Structure Channel Section 6

	A	B	C	D	E	F	G	H	I	J
62	Hydraulic Jump									
63	From Drainage Design Manual for Maricopa County, Hydraulics: Hydraulic Structures, 2003									
64	Y1 =	0.58 ft	upstream normal depth for drop							
65	Ydn =	1.36 ft	downstream normal depth							
66	Q =	80 cfs	flow through the drop							
67	g =	32.2 ft/s ²								
68	A1 =	6.64 ft ²	area of flow through the drop							
69	A2 =	18.22 ft ²	area of flow in next section							
70	z =	2.5 ft	sideslope H:1							
71	b =	10 ft	bottom width of channel							
72										
73	2) Calculate sequant height of jump.									
74	Equation 7.2									
75	$Y_2 = \frac{1}{2} Y_1 \left[\left(1 + 8F_{r1}^2 \right)^{1/2} - 1 \right]$									
76										
77										
78	Y2 =	2.15 ft	OK	height of jump						
79										
80	3) Another check on sequant height of jump.									
81	Use Fig. 7-8									
82			V =	12.05 ft/s						
83	Fr1 =	$\frac{V}{\sqrt{g y_m}}$	top width =	12.90 ft						
84			ym =	0.51 ft						
85			Fr1 =	2.96						
86										
87	J = Y2 / Y1		t = b/(zy1)							
88	J =	3.5	t =	6.90						
89	Y2 =	2.03 ft	use larger height of jump							
90										
91	1) Calculate depth at beginning location of jump.									
92	Equation 7.3									
93	$\frac{ZY_1^3}{3} + \frac{ZY_1^2}{2} + \frac{Q}{gA_1} = \frac{ZY_2^3}{3} + \frac{bY_2^2}{3} + \frac{Q}{gA_2}$									
94										
95										
96	Leq =	0.957	Req =	0.959 (Plug in values for Y2 until both sides equal)						
97	Yb =	0.47 ft	OK	depth at jump location						
98										
99	4) Calculate length of jump.									
100	Use Fig. 7-9									
101	Lj / y1 =	55								
102	Lj =	31.9 ft	= jump length							
103										
104	Therefore: Min. Length of jump =	32 ft								
105	Min. Length of apron =	10 ft								
106	Total Length of basin =	42 ft								
107	Total Length of required riprap =	47 ft (from brink of drop)								

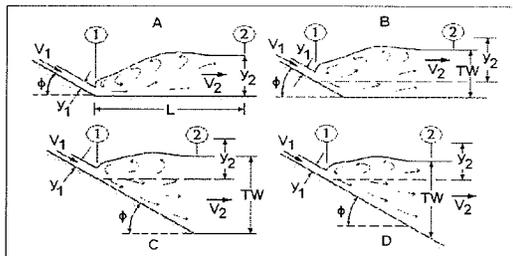


Figure 6.10. Hydraulic Jump Types Sloping Channels (Bradley, 1961)

Cholla Ash Monofill
Sloping Drop Structure
Riprap Sizing and Hydraulic Jump Formulas

	A	B	C	D	E	F	G	H	I
1	Riprap Basin Sizing								
2									
3	Q=	80		cfs	flow rate for section 6, just before brink of drop				
4	Vallow=	4.39		ft/s	velocity for section 5, past drop				
5	TW=	1.36		ft	normal depth for section 5, past drop				
6	Wo=	10		ft	channel bottom w				
7									
8	From FHWA Hydraulic Design of Energy Dissipators for Culverts and Channels, July 2006								
9	1) Get intial velocity (Vo), depth (yo), and Froude Number (Fr) for brink conditions								
10									
11	From Flowmaster output files:								
12	yo= ye=	0.58		ft	normal depth for c				
13	Vo=	12.05		ft/s	velocity for drop				
14	Fr=	2.96			Froude number for drop				
15									
16	2) Select trial D50 and obtain hs/ye from Equation 10.1.								
17									
18	Equation 10.1	$\frac{h_s}{y_e} = 0.86 \left(\frac{D_{50}}{y_e} \right)^{-0.55} \left(\frac{V_o}{\sqrt{g y_e}} \right) - C_o$							
19									
20									
21									
22	Get tailwater parameter Co:								
23	Co =	1.4			if TW/ye < 0.75				
24	Co =	=4*B5/B12-1.6			if 0.75 < TW/ye < 1.0				
25	Co =	3			if 1.0 < TW/ye				
26									
27	TW/ye =	=B5/B12							
28									
29	D50 =	0.25		ft	D50 of riprap				
30									
31	Check to see that hs/D50 >= 2 and that D50/ye >= 0.1.								
32	D50/ye =	=B29/B12		>= 0.1	OK				
33	g =	32.2		ft/s ²					
34	hs/ye =	=0.86*((B32)^(-0.55))*(B14)-B25							
35	hs =	=B34*B12		ft	scour depth				
36	hs/D50 =	=B35/B29		>=2.0	OK				
37									
38	3) Size the basin								
39	Ls =	10*hs =		=10*B35	ft	Dissipator Pool Length			
40	Lsmin =	3Wo =		=3*B6	ft				
41									
42	Lb =	15hs =		=15*B35	ft	Total Pool Length			
43	Lbmin =	4Wo =		=4*B6	ft				
44	La =	=C43-C40		ft	Apron Length				
45	Wb =	10		ft	maintaining channel bottom depth				
46									
47	4) Assess need for downstream riprap due to TW/ye >0.74 (for Drop Structures in channel only)								
48	Using Figure 10.3								
49	Compute equivalent circular diameter, De, for brink area								
50	A =	=B5*B12		$A = \frac{\pi D_e^2}{4} = y_o W_o$					
51	De =	=(B50*4/PI())^0.5							
52									
53	Rock size for riprap aft								
54	S =	=165/62.4		specific gravity of rock	$D_{50} = \frac{0.692}{S-1} \left(\frac{V^2}{2g} \right)$				
55									

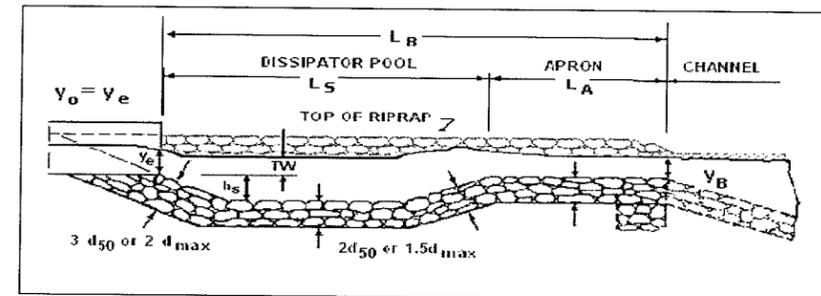


Figure 10.1. Profile of Riprap Basin

Cholla Ash Monofill
Sloping Drop Structure
Riprap Sizing and Hydraulic Jump Formulas

	A	B	C	D	E	F	G	H	I
56	L/De		L (ft)	VI/Vo (Fig. 10.3)	VI (ft/s)	D50 (ft)			
57	=B57/\$B\$51	10		0.42	=C57*\$B\$13				
58	=B58/\$B\$51	15		0.4	=C58*\$B\$13				
59	=B59/\$B\$51	17		0.38	=C59*\$B\$13				
60	=B60/\$B\$51	20		0.3	=C60*\$B\$13				
61									
62	Hydraulic Jump								
63	From Drainage Design Manual for Maricopa County, Hydraulics: Hydraulic Structures, 2003								
64	Y1 =	0.58	ft	upstream normal depth for drop					
65	Ydn =	1.36	ft	downstream normal depth section 5					
66	Q =	80	cfs	flow through the d					
67	g =	32.2	ft/s ²						
68	A1 =	6.64	ft ²	area of flow through the drop					
69	A2 =	10.43	ft ²	area of flow in next section					
70	z =	2.5	ft	sideslope H:1					
71	b =	10	ft	bottom width of channel					
72									
73	2) Calculate sequant height of jump.								
74	Equation 7.2								
75	$Y_2 = \frac{1}{2} Y_1 \left[(1 + 8F_{r1}^2)^{\frac{1}{2}} - 1 \right]$								
76									
77									
78	Y2 =	=0.5*B64*(((1+8*(D85^2))^0.5)-1)	ft	OK	height				
79									
80	3) Another check on sequant height of jump.								
81	Use Fig. 7-8								
82									
83	Fr1 =	$\frac{V}{\sqrt{gy_m}}$		V =	=B66/B68	ft/s			
84				top width =	12.9	ft			
85				ym =	=B68/D83	ft	= flow area / top width		
86				Fr1 =	=D82/SQRT(B67*D84)				
87	J = Y2 / Y1			t = b/(zy1)					
88	J =	3.5		t =	=B71/(B70*B64)				
89	Y2 =	=B88*B64	ft	use larger	height of jump				
90									
91	1) Calculate depth at beginning location of jump.								
92	Equation 7.3								
93	$\frac{ZY_1^3}{3} + \frac{ZY_1^2}{2} + \frac{Q}{gA_1} = \frac{ZY_2^3}{3} + \frac{bY_2^2}{3} + \frac{Q}{gA_2}$								
94									
95									
96	Leq =	=(B70*(B64^3)/3)+(B70*(B64^2)/2)+(B66/(B67*B68))		Req =	=(B70*B97^3/3)+(B71*B97^2/3)+(B66/(B67*B69))				
97	Yb =	0.44	ft	OK	depth at jump location			(Plug in values for Y2 until both sides equal)	
98									
99	4) Calculate length of jump.								
100	Use Fig. 7-9								
101	Lj / y1 =	55							
102	Lj =	=B101*B64	ft	= jump length					
103									
104									
105									
106									

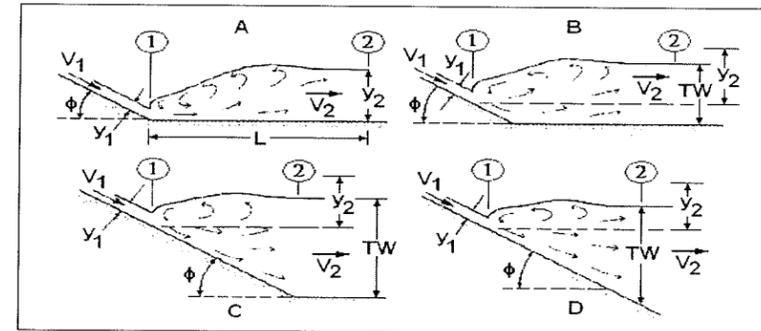


Figure 6.10. Hydraulic Jump Types Sloping Channels (Bradley, 1961)

Riprap placement required for 17 ft from brink.

Therefore: Min. Length of jump = **32** ft
 Min. Length of apron = **10** ft
 Total Length of basin = **=D105+D104** ft

**OFF-SITE CHANNEL
CULVERT AND WEIR CALCULATIONS**

CALCULATION COVER SHEET

Client: Arizona Public Service Project Name: Cholla Ash Monofill

Project/Calculation Number: 23445548

Title: Weir Equation Calculation

Total Number of Pages (including cover sheet): 6

Total Number of Computer Runs: _____

Prepared by: Michelle C. West, EIT Date: 2/12/09

Checked by: _____ Date: _____

Description and Purpose:

The purpose of this calculation is to estimate the height of the weirs for the two culverts in the offsite drainage channel.

The inputs were calculated in Culvertmaster and are attached.

Design Basis/References/Assumptions

The weirs are designed as broad crested and have a weir coefficient (C) of 2.7.

The weirs are 20 feet in length.

Remarks/Conclusions/Results:

See attached printouts.

Calculation Approved by: _____

Project Manager/Date

Revision No.:

Description of Revision:

Approved by:

Project Manager/Date

Job Cholla Ash Mono fill

 Project No. 23445548

 Sheet of

 Description Weir Calculation - Offsite

 Computed by MCW

 Date 2-12-09

 Checked by

 Date

Reference

Weir 1 (North Weir) elevation - 5081 ft

$$\text{Depth} = 1.07 \text{ ft} = H$$

$$\text{Length} = 20 \text{ ft} = L$$

$$\text{Weir Coefficient (Broad Crested Weir)} = 2.7 = C$$

$$Q = C \times L \times (H)^{3/2} = 2.7 (20) (1.07)^{3/2}$$

$$Q = 59.8 \text{ cfs}$$

$$Q \approx 60 \text{ cfs}$$

Weir 2 (South Weir) elevation - 5076.03 ft

$$\text{Depth} = 0.96 \text{ ft} = H$$

$$\text{Length} = 20 \text{ ft} = L$$

$$\text{Weir Coeff.} = 2.7 = C$$

$$Q = 2.7 (20) (0.96)^{3/2} = 50.8 \text{ cfs}$$

$$Q \approx 51 \text{ cfs}$$

- Inputs from Culvert master

Culvert Designer/Analyzer Report South Culvert (Sta. 7+50 to 8+00)

Analysis Component			
Storm Event	Design	Discharge	25.00 cfs

Peak Discharge Method: User-Specified			
Design Discharge	25.00 cfs	Check Discharge	25.00 cfs

Tailwater Conditions: Constant Tailwater	
Tailwater Elevation	N/A ft

Name	Description	Discharge	HW Elev.	Velocity
Culvert-1	1-30 inch Circular	25.00 cfs	5,076.99 ft	7.01 ft/s
Weir	Not Considered	N/A	N/A	N/A

Culvert Designer/Analyzer Report South Culvert (Sta. 7+50 to 8+00)

Component: Culvert-1

Culvert Summary			
Computed Headwater Elev.	5,076.99 ft	Discharge	25.00 cfs
Inlet Control HW Elev.	5,076.47 ft	Tailwater Elevation	N/A ft
Outlet Control HW Elev.	5,076.99 ft	Control Type	Outlet Control
Headwater Depth/Height	1.27		

Grades			
Upstream Invert	5,073.80 ft	Downstream Invert	5,073.70 ft
Length	50.00 ft	Constructed Slope	0.002000 ft/ft

Hydraulic Profile			
Profile	Composite M2 Pressure Profile	Depth, Downstream	1.70 ft
Slope Type	Mild	Normal Depth	N/A ft
Flow Regime	Subcritical	Critical Depth	1.70 ft
Velocity Downstream	7.01 ft/s	Critical Slope	0.019391 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	2.50 ft
Section Size	30 inch	Rise	2.50 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	5,076.99 ft	Upstream Velocity Head	0.40 ft
Ke	0.50	Entrance Loss	0.20 ft

Inlet Control Properties			
Inlet Control HW Elev.	5,076.47 ft	Flow Control	Unsubmerged
Inlet Type	Headwall	Area Full	4.9 ft ²
K	0.00780	HDS 5 Chart	2
M	2.00000	HDS 5 Scale	1
C	0.03790	Equation Form	1
Y	0.69000		

Culvert Designer/Analyzer Report North Culvert (Sta. 11+00 to 11+50)

5

Analysis Component			
Storm Event	Design	Discharge	80.00 cfs
Peak Discharge Method: User-Specified			
Design Discharge	80.00 cfs	Check Discharge	80.00 cfs
Tailwater Conditions: Constant Tailwater			
Tailwater Elevation	N/A ft		

Name	Description	Discharge	HW Elev.	Velocity
Culvert-1	2-30 inch Circular	80.00 cfs	5,082.07 ft	8.98 ft/s
Weir	Not Considered	N/A	N/A	N/A

Culvert Designer/Analyzer Report North Culvert (Sta. 11+00 to 11+50)

Component: Culvert-1

Culvert Summary			
Computed Headwater Elev.	5,082.07 ft	Discharge	80.00 cfs
Inlet Control HW Elev.	5,081.82 ft	Tailwater Elevation	N/A ft
Outlet Control HW Elev.	5,082.07 ft	Control Type	Outlet Control
Headwater Depth/Height	1.63		

Grades			
Upstream Invert	5,078.00 ft	Downstream Invert	5,076.93 ft
Length	50.00 ft	Constructed Slope	0.021400 ft/ft

Hydraulic Profile			
Profile	CompositeM2PressureProfile	Depth, Downstream	2.13 ft
Slope Type	Mild	Normal Depth	N/A ft
Flow Regime	Subcritical	Critical Depth	2.13 ft
Velocity Downstream	8.98 ft/s	Critical Slope	0.030441 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	2.50 ft
Section Size	30 inch	Rise	2.50 ft
Number Sections	2		

Outlet Control Properties			
Outlet Control HW Elev.	5,082.07 ft	Upstream Velocity Head	1.03 ft
Ke	0.20	Entrance Loss	0.21 ft

Inlet Control Properties			
Inlet Control HW Elev.	5,081.82 ft	Flow Control	N/A
Inlet Type	Beveled ring, 45° (1:1) bevels	Area Full	9.8 ft²
K	0.00180	HDS 5 Chart	3
M	2.50000	HDS 5 Scale	A
C	0.03000	Equation Form	1
Y	0.74000		

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